

#### **OHIO VALLEY ELECTRIC CORPORATION**

3932 U. S. Route 23 P. O. Box 468 Piketon, Ohio 45661 740-289-7200

WRITER'S DIRECT DIAL NO: 740-289-7259

June 29, 2023

#### **Delivered Electronically**

Mr. Brian Rockensuess Commissioner Indiana Department of Environmental Management 100 N. Senate Avenue Mail Code 50-01 Indianapolis, IN 46204-2251

Re: Indiana-Kentucky Electric Corporation- Clifty Creek Station Revision to the Landfill Runoff Collection Pond Closure Plan

Dear Mr. Rockensuess:

As required by 40 CFR 257.106(i)(4), Indiana-Kentucky Electric Corporation is providing notification to the Commissioner of the Indiana Department of Environmental Management (IDEM) that the Closure Plan for Clifty Creek Station's Landfill Runoff Collection Pond has been revised. The newly revised plan will be placed in the facility's operation record as well as the publicly accessible internet site, which can be viewed at <a href="http://www.ovec.com/CCRCompliance.php">http://www.ovec.com/CCRCompliance.php</a>

If you have any questions, or require any additional information, please call me at (740) 289-7259, or you can contact Tim Fulk at (740) 897-7768.

Sincerely,

Jeremy Galloway

**Environmental Specialist** 

JDG: tlf



#### Stantec Consulting Services Inc. 9200 Shelbyville Road, Suite 800, Louisville KY 40222-5136

June 27, 2023

Project/File: 175531036

Mr. Gabriel Coriell
Indiana-Kentucky Electric Corporation
3932 U.S. Route 23
P.O. Box 468
Piketon, Ohio 45661

Reference: Closure Plan

**Clifty Creek Station** 

Landfill Runoff Collection Pond Madison, Jefferson County, Indiana

Dear Mr. Coriell,

The attached closure plan for Clifty Creek Station's Landfill Runoff Collection Pond (LRCP) was prepared by Stantec Consulting Services Inc. (Stantec) for the Indiana-Kentucky Electric Corporation (IKEC).

The initial closure and post-closure plans for the LRCP were posted on October 11, 2016 as part of the U.S. Environmental Protection Agency (EPA) final coal combustion residuals (CCR) rule demonstrations. The plans were conceptual and subject to the completion of all necessary environmental reviews. Though conceptual, they demonstrated compliance with the requirements set forth in 40 CFR 257.102(b) and 257.104(d).

IKEC, Stantec, and the Indiana Department of Environmental Management (IDEM) met in Indianapolis on December 9, 2019 to discuss the requirements for Indiana CCR surface impoundment closures. IKEC submitted the closure plan for the LRCP to IDEM on June 17, 2021. No comment has been received from IDEM on this proposed closure plan to date.

The plan was prepared in accordance with the accepted practice of engineering and accurate information at the date of its submittal to meet the requirements described in 40 CFR 257.102(b). Changes to the closure plan may be required. Revised plans will be posted at that time.

June 27, 2023 Mr. Gabriel Coriell Page 2 of 2

Reference: Closure Plan

Clifty Creek Station Landfill Runoff Collection Pond Madison, Jefferson County, Indiana

Regards,

STANTEC CONSULTING SERVICES INC.

**Matt Vaughan** 

Principal \*Licensed in KY, IN

Phone: (502) 212-5088 matt.vaughan@stantec.com

stantec.com

Attachment: Stantec Consulting Services Inc. (2021). Closure Plan. Clifty Creek Station. Landfill Runoff Collection Pond. Madison, Jefferson County,

Indiana. Prepared for Indiana-Kentucky Electric Corporation. June 16.

# **ATTACHMENT**

Closure Plan. Clifty Creek Station. Landfill Runoff Collection Pond.

Stantec Consulting Services Inc. (2021)



# OHIO VALLEY ELECTRIC CORPORATION INDIANA- KENTUCKY ELECTRIC CORPORATION

3932 U. S. Route 23 P.O. Box 468 Piketon, Ohio 45661 740-289-7200

WRITER'S DIRECT DIAL NO: (740) 897-7768

June 17, 2021

Ms. Kate Garvey
Office of Land Quality
Indiana Department of Environmental Management
Solid Waste Permits Section
100 N. Senate Avenue
MC 65-45 IGCN 1101
Indianapolis, IN 46204-2205

Dear Ms. Garvey:

Re: Indiana-Kentucky Electric Corporation
Clifty Creek Station Landfill Runoff Collection Pond Closure Plan

In accordance with 329 IAC 10-3-1(9), the Indiana-Kentucky Electric Corporation (IKEC) is submitting for agency review the accompanying Closure Plan for Clifty Creek Station's Landfill Runoff Collection Pond (LRCP). IKEC is committed to being a good steward of the environment and to satisfying our environmental compliance obligations. We recognize the importance of maintaining a close partnership with IDEM in this endeavor, and appreciate the opportunity to submit this plan for agency review. Note that while IKEC desires to work closely with IDEM, IKEC is not waiving any of the positions it identified in its May 28, 2021 letter to Steven Thill or those identified in its June 1, 2021 Petition for Review of the Approval of the Partial Closure Plan for IKEC's WBSP.

If you have any questions or comments please contact me at (740) 897-7768.

Sincerely,

Tim Full

Tim Fulk Engineer II

TLF:gsc

Attachments





## Closure Plan

Clifty Creek Station
Landfill Runoff Collection Pond
Madison, Jefferson County,
Indiana

Indiana-Kentucky Electric Corporation 3932 U.S. Route 23 Piketon, Ohio 45661

June 16, 2021

# Closure Plan Landfill Runoff Collection Pond

## Clifty Creek Station Madison, Jefferson County, Indiana

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# Closure Plan Landfill Runoff Collection Pond

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#### 1. Objective

Indiana-Kentucky Electric Corporation (IKEC) is submitting this Closure Plan for the Clifty Creek Station's Landfill Runoff Collection Pond (LRCP) to the Indiana Department of Environmental Management (IDEM) Office of Water Quality (OWQ) with copies to the Office of Land Quality (OLQ), Waste Section. IKEC requests OWQ coordinate its review and comments with OLQ in a timely manner that facilitates adherence to the proposed schedule to close the LRCP at the Clifty Creek Station.

The LRCP is an inactive coal combustion residuals (CCR)surface impoundment. It currently receives stormwater runoff from more than 500 acres. Contact water from the adjacent landfill and stormwater run-on from the adjacent hillsides flow into the LRCP. Discharge from the LRCP is managed under the site's National Pollution Discharge Elimination System (NPDES) permit at the southwest corner of the LRCP. The applicable NPDES Permit No. is IN0001759. IKEC is preparing to cap and close the LRCP following the requirements in 329 Indiana Administrative Code (IAC) 10-9-1(9)(b) and (c) which incorporates portions of the United States Environmental Protection Agency (EPA) Final CCR Rule by reference.

The closure design includes two distinct phases of construction. Phase 1 involves the operational drawdown of the existing water surface elevation within the LRCP to a reduced footprint using the existing discharge structure and construction of a stormwater run-on diversion ditch around the perimeter of the LRCP to divert run-on from the surrounding hillsides directly to a new outfall to the Ohio River. These two steps in Phase 1 will remove water from the LRCP and promote drying of the CCR material in preparation for closure.

Phase 2 involves dewatering the CCR, grading the CCR to subgrade elevations, the installation of the final cover system, and modifications to the existing outfall structure.

In association with the closure of the LRCP, minor modifications will be necessary at the landfill to manage stormwater runoff and leachate flows. This will require the construction of two interim sediment basins and two leachate collection ponds, and appurtenances. Proposed modifications to the existing IDEM landfill permit will be submitted under a separate permit application. For the purposes of this Closure Plan, it is assumed these landfill improvements will be completed prior to commencing LRCP closure construction.

Appendix A is a list of acronyms and abbreviations.

#### 2. Description of the CCR Unit

The Clifty Creek Station is located on the north bank of the Ohio River west of Madison, Indiana. It consists of six coal-fired electric generating units, each nominally rated at 217megawatts. The station began producing electricity in 1955 to support the Department of Energy's (DOE's) Portsmouth Gaseous Diffusion Plant located near Piketon, Ohio. The LRCP is located southwest of the station, adjacent to the CCR landfill. Figure 1shows the location of the LRCP and other key facilities at the site.

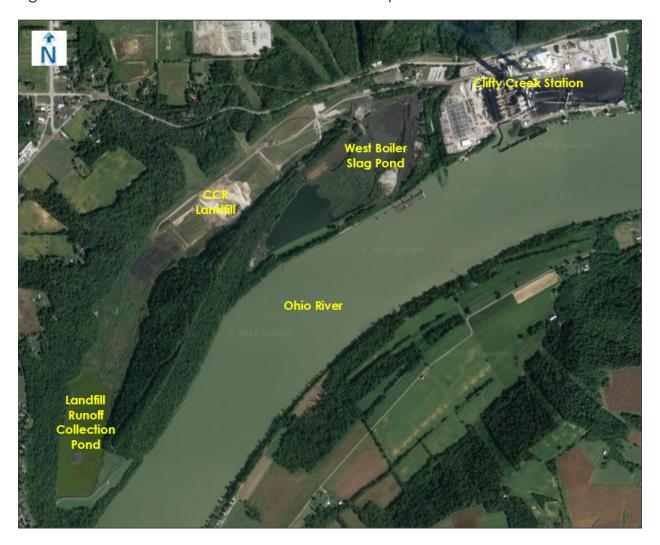


Figure 1- Aerial View of Clifty Creek Station

#### 2.1. Impoundment Structure

The LRCP is surrounded by steep hillsides to the east and west, the CCR landfill to the north, and a constructed embankment to the south. The total closure area is approximately 57 acres, 38 of which is currently surface water. The current water surface elevation is approximately 486 feet. The crest of the south embankment is at approximate elevation 505 feet. FEMA (2015) Flood Insurance Study No. 18077CV000A

shows that the flood stages of the Ohio River at the LRCP are 463 feet and 468 feet for the 1 percent and 0.2 percent annual chance of flooding.

According to As-built drawings 16-3170-1, 16-3171-4, and 16-3171A-4, the crest of the dam is 26 feet wide. The upstream slopes are 2.5H:1V (horizontal slope: vertical slope). The downstream slopes are 2.5H:1V above elevation 478 feet and 3H:1V below this elevation. The exterior LRCP slopes are grass covered. As-built drawings are included in Appendix B.

#### 2.2. Primary Spillway

Flows from the LRCP are currently permitted to be discharged through Outfall 001 to the Ohio River under NPDES Permit No. IN0001759 effective May 1, 2018, which is administered by IDEM. The LRCP's primary spillway consists of an intake structure in the pond connected by riser structures to a concrete pipe which runs under the LRCP dam and eventually to the Ohio River. The CCR pond intake structure consists of an inclined 6-feet x 3-feet reinforced concrete box culvert with intermittent riser box structures containing grated inlets located at every 11 feet in elevation. Currently, the two uppermost riser box structures are above the pond level and the lower riser structure is the active outlet for the pond. The inclined box is connected to a 400-feet long, 72-inch diameter concrete pipe that is located under the dam where outflow discharges into the Ohio River.

#### 2.3. LRCP Location

The Clifty Creek Station is in Jefferson County, Madison Township, Indiana in Township 3N, Range 10E, Section 5. A legal description of the facility boundary is included in Appendix C. Located south of Indiana State Route 56 and northwest of a bedrock geologic feature called the Devil's Backbone, the CCR Landfill is being constructed in two phases from northeast to southwest, down the valley. Water from the landfill and stormwater run-on from the adjacent hillsides flows southwest to the LRCP. Discharge from the LRCP is managed under the site's NPDES permit at the southwest corner of the site.

In Appendix D, Figures 2 and 3 reflect a half-mile offset from the LRCP's waste boundary with regional water wells identified in the Indiana Department of Natural Resources (IDNR), Division of Water Well Record Database (IDNR, 2019). Figure 2 shows a plan view of the LRCP overlain on a November 2017 ESRI aerial. Figure 3 reflects the same data shown on a portion of the 7½-minute USGS topographic quadrangle map for Madison West (2019).

Four wells are shown within the half-mile offset. Available IDNR well information is provided in Appendix D. All four wells are owned by nearby Hanover College. Table 1 details the four wells within the half-mile offset from the LRCP.

Table 1 - Water Wells Within a Half-Mile Offset

Well Record Reference No.	Туре	Depth (feet)	Completion Date	Location	Status
				Madison–West, Jefferson County,	Inactive
219384	Unconsolidated	68	06/30/1967	IN	
				Madison–West, Jefferson County,	Inactive
219399	Unconsolidated	69	10/11/1948	IN	
				Madison–West, Jefferson County,	Inactive
312919	Unknown			IN	
				Madison–West, Jefferson County,	Inactive
312920	Unconsolidated	80	01/01/1940	IN	

#### 2.4. Available Geotechnical Data

Geotechnical data is available from six field explorations at the LRCP. A plan view of the borings and logs are provided in Appendix E. Appendix F contains a slope stability analysis for the LRCP closure.

#### American Gas & Electric Service Corp. (1953)

The 1956 as-built design drawings include geotechnical profiles within the LRCP dam. The drawings show the embankment dimensions and generalized original ground topography.

#### American Gas & Electric Service Corp. (1985)

An exploration program was carried out in 1984 to obtain geotechnical properties of the LRCP dam to evaluate the feasibility of raising it by 30 feet to elevation 535 feet. Thirty-seven borings and nine CPT soundings were advanced along the dam crest, downstream toe and pond of the LRCP facility. Results from the exploration indicate that the dam was constructed in compacted layers of silty and sandy clays. A 30 to 40-ft thick seam of gray clay was encountered underlying the main dam and the bedrock underlying foundation soils was described as Indiana Limestone with layers of calcareous shale.

#### Stantec (2016)

Stantec performed two geotechnical field explorations to support the safety factor demonstration under the CCR Rule. Four borings were advanced along the crest and the downstream toe of the LRCP Dam in 2009/2010 and one borehole was advanced in 2015 to confirm field conditions. Laboratory testing was performed to confirm field classifications (natural moisture content, hydrometer analyses, Atterberg limits), estimate shear strength (consolidated-undrained triaxial compression testing), and permeability. Results from the explorations indicate that the dam was constructed of lean and silty clay with sand. Silt with sand and silty sand soils were encountered in the foundation.

#### AGES (2016)

Applied Geology and Environmental Science (AGES), Inc. was contracted by IKEC to identify upgrades in the groundwater monitoring program of the LRCP necessary for compliance with the CCR Rule. In 2015, three monitoring wells were installed using a sonic drill rig. Excerpts from AGES (2018) are included in Appendix E. This includes a well summary table, a generalized geologic cross section, groundwater flow maps for four sampling periods, and sample/well construction logs.

#### Stantec (2021)

Stantec conducted a field exploration in 2021 to support design and construction of the LRCP closure. Six geotechnical borings were advanced to a depth ranging from 15 to 40 feet below existing grade. The logs described the material as boiler slag (coarse sand); lean and silty clay, silty sand, gravel with sand, or shale bedrock. Laboratory testing of samples collected during this exploration was ongoing at the time of this report. A plan view and boring logs for this exploration are included in Appendix E.

#### 3. Regulatory Overview

#### 3.1. Regulatory Framework for Design

The United States EPA Final CCR Rule defines the criteria for conducting the closure of CCR units under 40 CFR 257.102.

Per the IDEM CCR Fact Sheet, Indiana coal ash surface impoundments that are subject to an NPDES permit are not regulated under IDEM's solid waste program (IDEM 2021). Once the NPDES permit is terminated, the final disposal of solid waste in the surface impoundment is subject to the closure requirements under 329 Indiana Administrative Code (IAC) 10-9-1(9)(b) and (c), which incorporates portions of the CCR Rule by reference.

This submittal is an amendment to a written closure plan (40 CFR 257.102(b)(3)) describing closure in place (40 CFR 257.102(d)) for the LRCP.

Below is a general summary of how the LRCP will be closed. The permit-level drawings are included in Appendix G. Ditch sizing calculations associated with the permit-level design are included in Appendix H.

#### 3.2. Description of Closure Plan - 257.102(b)(1)(i)

# [A narrative description of how the CCR unit will be closed in accordance with this section.]

The intent is to consolidate within the LRCP where possible, grade stored CCR, and close the facility in place. The closure will consist of dewatering the unit through an NPDES-permitted outfall, grading, and construction of an engineered cap.

IDEM's OLQ has requested that the LRCP be closed in accordance with Type I restricted waste site (RWS) standards. Under 329 IAC 10-30-2, final cover must have:

- A maximum projected erosion rate of five tons per acre per year
- A final compacted cover of six inches of topsoil plus a minimum depth of compacted clay of 30 inches:
- Slopes not less than two percent nor greater than 33 percent.

Appendix I includes the final cover soil loss calculations. The final cover consists of a 30-inch cover material layer and six inches of earthen material capable of growing and sustaining native vegetative growth. A geosynthetic membrane liner and geocomposite drainage layer is included below the cover material. The capped surface will be graded to promote surface water runoff, and then seeded and mulched to promote growth of the vegetative cover.

#### 3.3. Closure in Place - 257.102(b)(1)(iii)

[If closure of the CCR unit will be accomplished by leaving the CCR in place, a description of the final cover system, designed in accordance with paragraph (d) of this section, and the methods and procedures to be used to install the final cover.]

Prior to installing the final cover system, the CCR unit will be drained of free water and the material within the unit will be stabilized and graded to provide a stable and suitable subgrade upon which to construct the cap. All water will be managed in the remaining open portion of the pond and discharged through the existing NPDES outfall. The final closure system will consist of a flexible geomembrane liner (FML), that will have a permeability that is less than or equal to the permeability of the natural subsoils, and is no greater than 1x10-5 cm/sec. FML will be installed directly over the graded CCR material followed by a layer of geocomposite drainage media, 2.5-foot thick cover soil, and 6-inches of earthen material capable of growing and sustaining native vegetative growth. The capped surface will be graded to promote surface water runoff, and then seeded and mulched to promote growth of the vegetative cover.

Piezometers will be installed to monitor water levels within the closed footprint. Wellpoints and sump pumps may be added as needed to drawdown the near surface water to support construction of the proposed final cover system. All pumped water will be returned to the remaining open portion of the pond to be discharged through the existing NPDES outfall.

Stormwater drainage improvements will be implemented during the final closure activities with minor grading of existing channels and construction of new channels to improve drainage of the closed pond. The final cover slope will be a minimum of two percent (2%) and will convey surface water to an NPDES-permitted outfall. Permanent stormwater ditch slopes may vary and will be sized to adequately convey anticipated design storm events.

#### 3.4. Closure Performance Standards - 257.102(d)(1)

#### 3.4.1. Section 257.102(d)(1)(i),(ii),(iii)

[(i)Control, minimize or eliminate, to the maximum extent feasible, post-closure infiltration of liquids into the waste and releases of CCR, leachate, or contaminated run-off to the ground or surface waters or to the atmosphere; (ii) Preclude the probability of future impoundment of water, sediment, or slurry; (iii) Include measures that provide for major slope stability to prevent the sloughing or movement of the final cover system during the closure and post-closure care period]

Post-closure infiltration of liquids into the waste will be controlled through the design of the site grading plan, construction of an engineered cap system, and establishment of a stormwater management system in accordance with engineering practices. The intent of such a plan is to limit the infiltration of precipitation, cover, control, and prevent the releases of CCRs, and promote positive drainage. CCR materials will be placed and compacted in a manner to minimize settling and subsidence that could affect the integrity of the final cover system prior to cap placement.

Installation and quality control testing of the geosynthetics will be performed as specified by the manufacturer.

Stability analyses were performed as part of the EPA Final CCR Rule's design criteria demonstrations (Stantec, 2016). Stability analyses can be found in Appendix F. Additional analyses have been performed to support the proposed closure configuration and improvements to the LRCP dam.

#### 3.4.2. Section 257.102(d)(1)(iv)

[Minimize the need for further maintenance of the CCR unit.]

The impoundment will be vegetated to prevent erosion. Maintenance of the final cover system will include regularly scheduled inspections to monitor post-closure conditions and preventative maintenance.

#### 3.4.3. Section 257.102(d)(1)(v)

[Be completed in the shortest amount of time consistent with recognized and generally accepted good engineering practices.]

The impoundment will be closed in a time frame consistent with recognized and generally accepted good engineering practices. Refer to the schedule below for key milestone dates.

#### 3.5. Draining and Stabilizing the Surface Impoundment

[The owner or operator of a CCR surface impoundment or any lateral expansion of a CCR surface impoundment must meet the requirements of paragraph (d)(2)(i) and (ii) of this section prior to installing the final cover system required under paragraph (d)(3) of this section.]

#### 3.5.1. Section 257.102(d)(2)(i)

[Free liquids must be eliminated by removing liquid wastes or solidifying the remaining wastes and waste residue.]

Free liquid will be removed as part of the final closure of the CCR unit and discharged in a manner consistent with the facility's NPDES permit.

#### 3.5.2. Section 257.102(d)(2)(ii)

[Remaining waste must be stabilized sufficient to support the final cover system.]

The remaining wastes that constitute the subgrade of the final cover system will be stabilized by removal of free liquids and providing bridging material as necessary.

#### 3.6. Final Cover System - 257.102(d)(3)

[If a CCR unit is closed by leaving the CCR in place, the owner or operator must install a final cover system that is designed to minimize infiltration and erosion, and at a minimum, meets the requirements of paragraph (d)(3)(i) of this section, or the requirements of the alternative final cover system specified in paragraph (d)(3)(ii) of this section.

The final cover system must be designed and constructed to meet the criteria in paragraphs (d)(3)(i)(A) through (D) of this section. The design of the final cover system must be included in the written closure plan.]

The final closure system details are included above in Sections 3.2 and 3.3

#### 3.7. Estimate of Maximum CCR Volume - 257.102(b)(1)(iv)

[An estimate of the maximum inventory of CCR ever on-site over the active life of the CCR unit.]

The LRCP is an inactive facility that no longer receives CCR material. The estimated maximum amount of CCR to ever be on-site for the LRCP is approximately 1,900 acrefeet.

#### 3.8. Estimate of Largest Area of CCR Requiring Cover - 257.102(b)(1)(v)

[An estimate of the largest area of CCR unit ever requiring a final cover.]

The area of the proposed final cover system is approximately 57 acres.

#### 3.9. Closure Schedule - 257.102(b)(1)(vi)

[A schedule for collecting all activities necessary to satisfy the closure criteria in the section, including an estimate of the year in which all closure activities for the CCR unit will be completed. The schedule should provide sufficient information to describe the steps that will be taken to close the CCR unit, including identification of major milestones such as coordinating with and obtaining necessary approvals and permits from other agencies, the dewatering and stabilization of the CCR surface impoundment closure, or installation of the final cover system, and the estimated timeframes to complete each step or phase of the CCR unit closure.]

LRCP closure activities will begin upon approval of this permit application. The intent is to complete construction of the cap.

USEPA (2020) has defined a closure schedule process for existing CCR surface impoundments that are considered "unlined" under the CCR Rule. Table 2 provides an approximate closure schedule to meet the required regulation.

Task	Completion Date				
Phase 1 construction	End of 2023				
Phase 2 construction	Within 5 years of cessation of flows or when				
	technically feasible				

Table 2 - Proposed Closure Schedule

#### 4. General Considerations

General considerations for the LRCP closure are presented in the following sections. Subsequent to final closure, IKEC will address environmental concerns and permit obligations that are regulated by other IDEM divisions during the closure process. All demonstrations reflecting the LRCP's compliance with the EPA Final CCR Rule in terms of location restrictions, design criteria, operating criteria, and groundwater monitoring are available on IKEC's public website, www.ovec.com/CCRClifty.php.

The closure design includes two distinct phases of construction. Phase 1 involves the operational drawdown of the existing water surface elevation within the LRCP to a reduced footprint using the existing discharge structure, and construction of a stormwater run-on diversion ditch around the perimeter of the LRCP to divert run-on from the surrounding hillsides directly to a new outfall to the Ohio River. These two steps in Phase 1 will remove water from the LRCP and promote drying of the CCR material in preparation for closure.

Phase 2 involves dewatering the CCR, grading the CCR to subgrade elevations, the installation of the final cover system, and modifications to the existing outfall structure.

In association with the closure of the LRCP, minor modifications will be necessary at the landfill to manage stormwater runoff and leachate flows. This will require the construction of two sediment basins and two leachate collection ponds, and appurtenances. Proposed minor modifications to the existing IDEM landfill permit will be submitted under a separate permit application. For the purposes of this Closure Plan, it is assumed these landfill improvements will be completed prior to commencing LRCP closure construction.

#### 5. Closure Plan Scope of Work

The closure design includes two distinct phases of construction. The following general tasks are anticipated as part of the closure process.

#### 5.1. Phase 1 – Drawdown and Ditch Diversion

Specific activities associated with Phase 1 of the LRCP closure plan include:

- Operational drawdown of the existing water surface elevation within the LRCP to a reduced footprint using the existing discharge structure.
- Construction of a stormwater run-on diversion ditch around the perimeter of the LRCP to divert landfill runoff and run-on from the surrounding hillsides directly to a new outfall to the Ohio River.
- Notification of completion of Phase 1 closure to the IDEM OWQ and OLQ.
- Once Phase 1 construction is complete, the only inflows to the LRCP will be direct precipitation and stormwater run-on. All CCR waste streams will cease no later than 2023.

#### 5.2. Phase 2 – Dewatering and Cover System

Specific activities associated with Phase 2 of the LRCP closure plan include:

- Continued dewatering of the CCR in the LRCP's reduced footprint.
- Re-grading the CCR to design subgrade elevations.
- Construct piezometers to monitor water levels.
- Construct the engineered cap system.
- Establish the access road and vegetation on the site.
- Notification of completion of Phase 2 final closure to the IDEM OWQ and OLQ.

#### 5.3. Cap System

An engineered cap system will be constructed over the LRCP closure area. Appendix G provides the proposed permit drawings, including the cap area and details.

The engineered cap will consist of the following materials, listed in order of construction (from bottom to top):

- 40-mil geomembrane
- Geocomposite drainage layer
- 36 inches of cover soil, of which the top 6 inches are capable of supporting vegetation

The final cap system design will accommodate settling and subsidence so to preserve the cap system's integrity.

#### 5.4. Stormwater Construction Permit

Since more than one acre will be disturbed during the pond closure activities, a Stormwater Notice of Intent (NOI) to discharge stormwater associated with construction activities will be submitted to IDEM OWQ. A Stormwater Pollution Prevention Plan (SWPPP) will be prepared and submitted as required by the NOI along with applicable permit fees. A Notice of Termination (NOT) to terminate the stormwater construction permit will be submitted upon completion of the pond closure.

#### 5.5. Construction Quality Assurance

Construction quality assurance (CQA) plan activities will be finalized as part of the detailed design and prior to construction of each phase of the LRCP closure. Construction observations will be conducted and recorded to document the closure and CQA testing. CQA activities are included in the project Quality Management Plan (QMP). Sections of the QMP will include:

- A. Purpose and Scope
- B. Responsibility and Authority
- C. Quality Control Activities
- D. Quality Assurance Activities
- E. Material Testing
- F. Project Documentation

Appendix J includes a draft QMP. This is proposed as the basis for the final plan to maintain consistency on the site.

#### 5.6. Closure Documentation

Upon completion of approved closure construction activities, a closure report will be prepared by an independent professional engineer registered in the State of Indiana to document the completed construction activities. The closure report will be submitted to IDEM OLQ and OWQ. The letter report will document the source of fill material, amount of fill material used, details regarding cap construction, and final cap elevations.

#### 6. Post-Closure Plan

Post-closure care will be performed in accordance with the Post-closure Plan included as Appendix K.

Estimated Closure and Post-Closure costs are provided in Appendix L.

#### 7. References

- American Electric Power Service Corporation. (2016). History of Construction. CFR 257.73(c)(1). West Boiler Slag Pond. Clifty Creek Plant. Madison, Indiana. October. Prepared for Indiana-Kentucky Electric Corporation. GERS-16-142. Columbus, Ohio.
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# **APPENDIX A**

Acronyms and Abbreviations

# **Acronyms and Abbreviations**

AEPSC American Electric Power Service Corporation

AGES Applied Geology and Environmental Science, Inc.

CCR Coal Combustion Residuals

CFR Code of Federal Regulations

cm/sec centimeters per second

CQA Construction Quality Assurance

DOE Department of Energy

ELG Effluent Limitations, Guidelines

FML flexible membrane liner

H:V horizontal slope : vertical slope

IAC Indiana Administrative Code

IDEM Indiana Department of Environmental Management

IDNR Indiana Department of Natural Resources

IKEC Indiana-Kentucky Electric Corporation

LRCP Landfill Runoff Collection Pond

mW megawatts

No. number

NOI Notice of Intent

NOT Notice of Termination

NPDES National Pollutant Discharge Elimination System

OLQ Office of Land Quality

OWQ Office of Water Quality

QMP Quality Management Plan

SWPPP Stormwater Pollution Prevention Plan

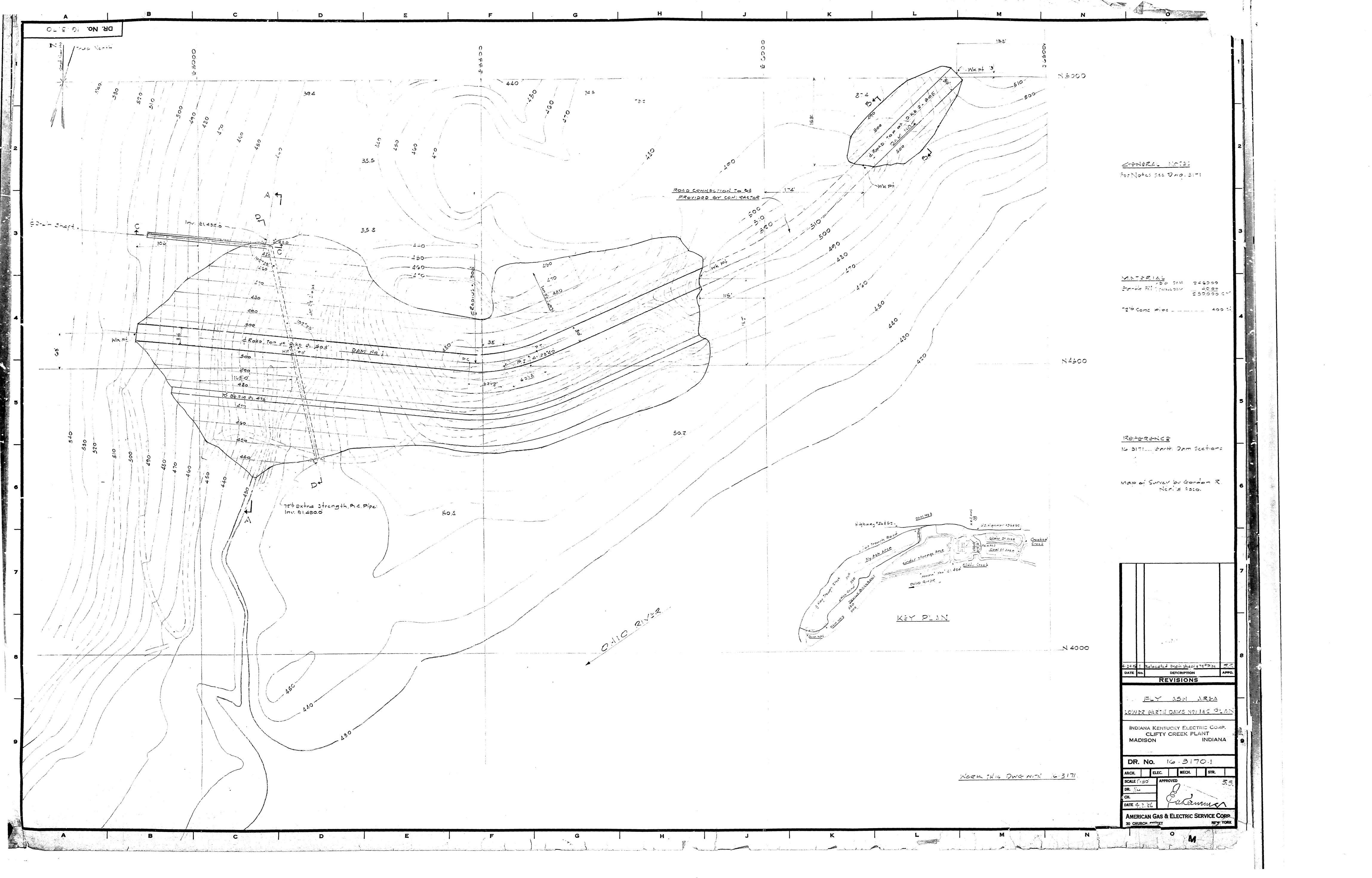
UMA uppermost aquifer

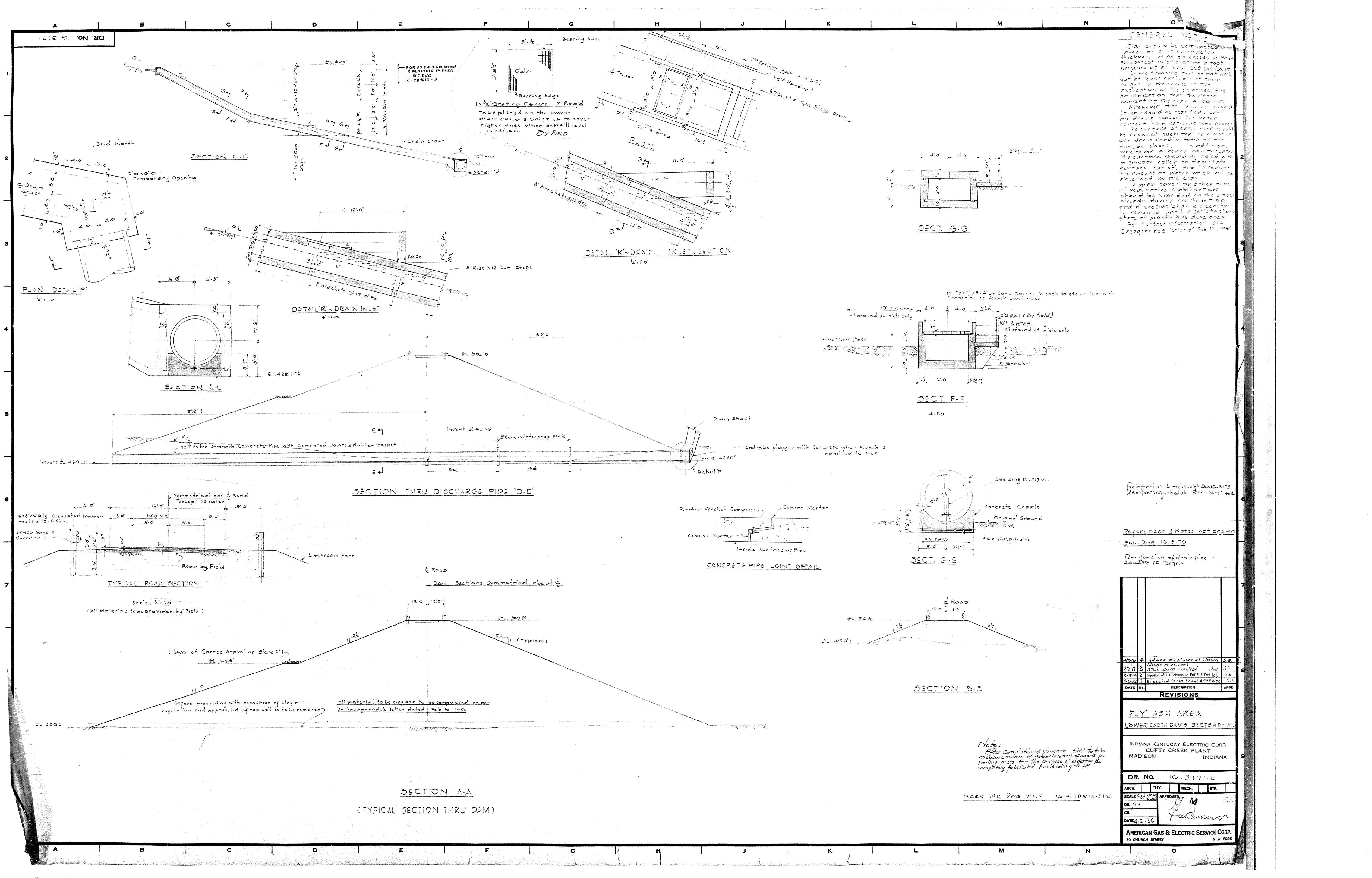
USEPA United States Environmental Protection Agency

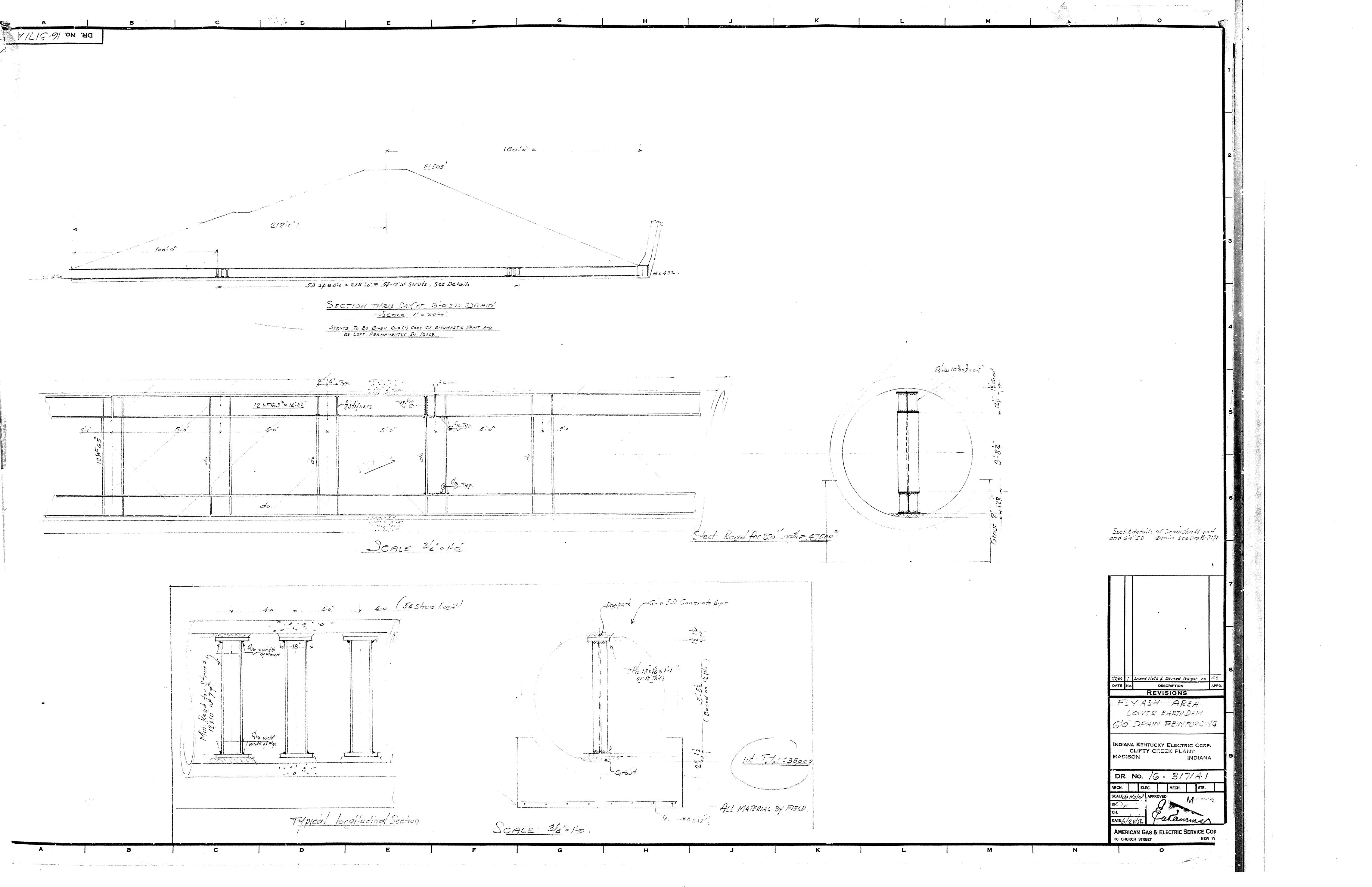
WBSP West Boiler Slag Pond

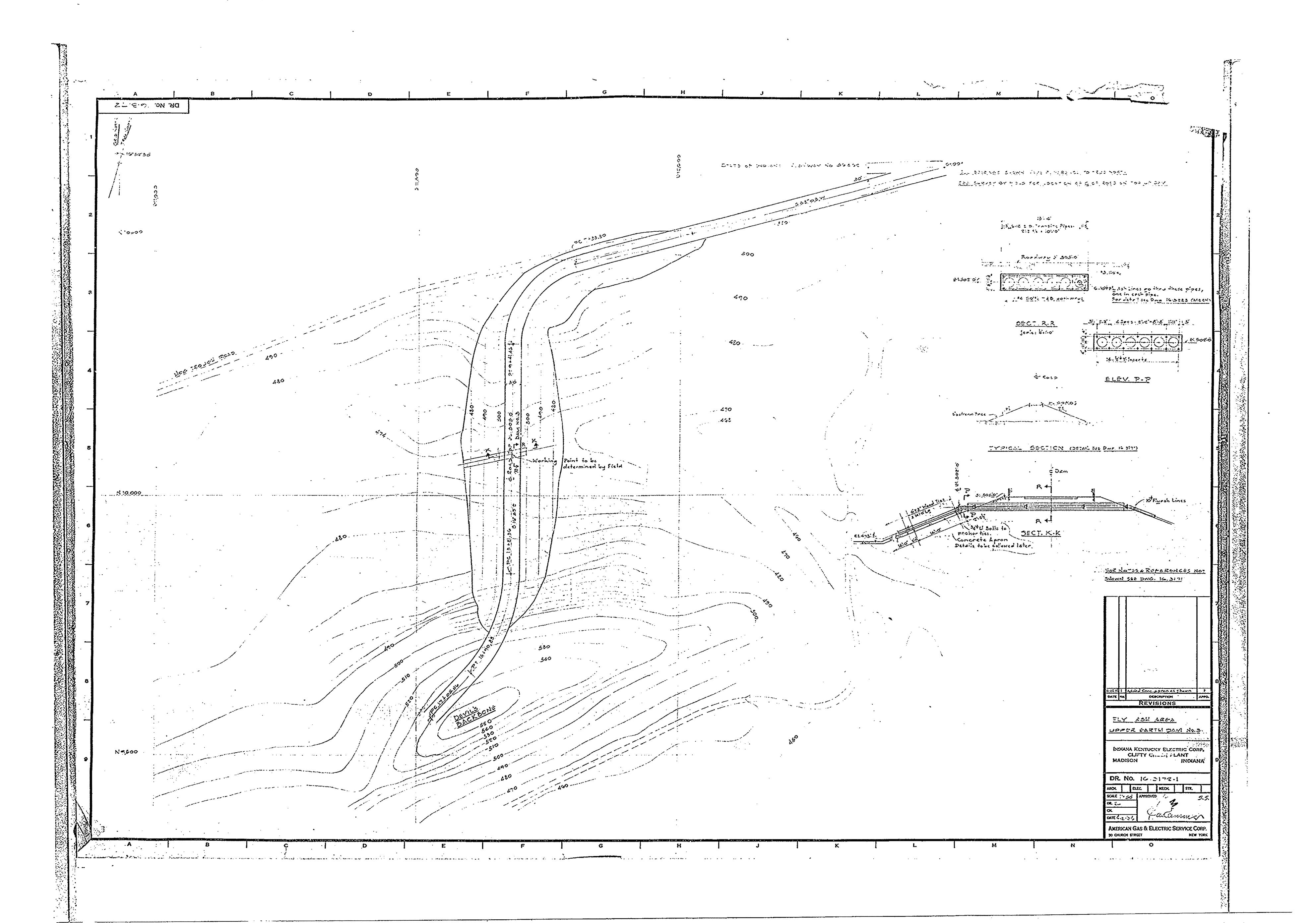
# **APPENDIX B**

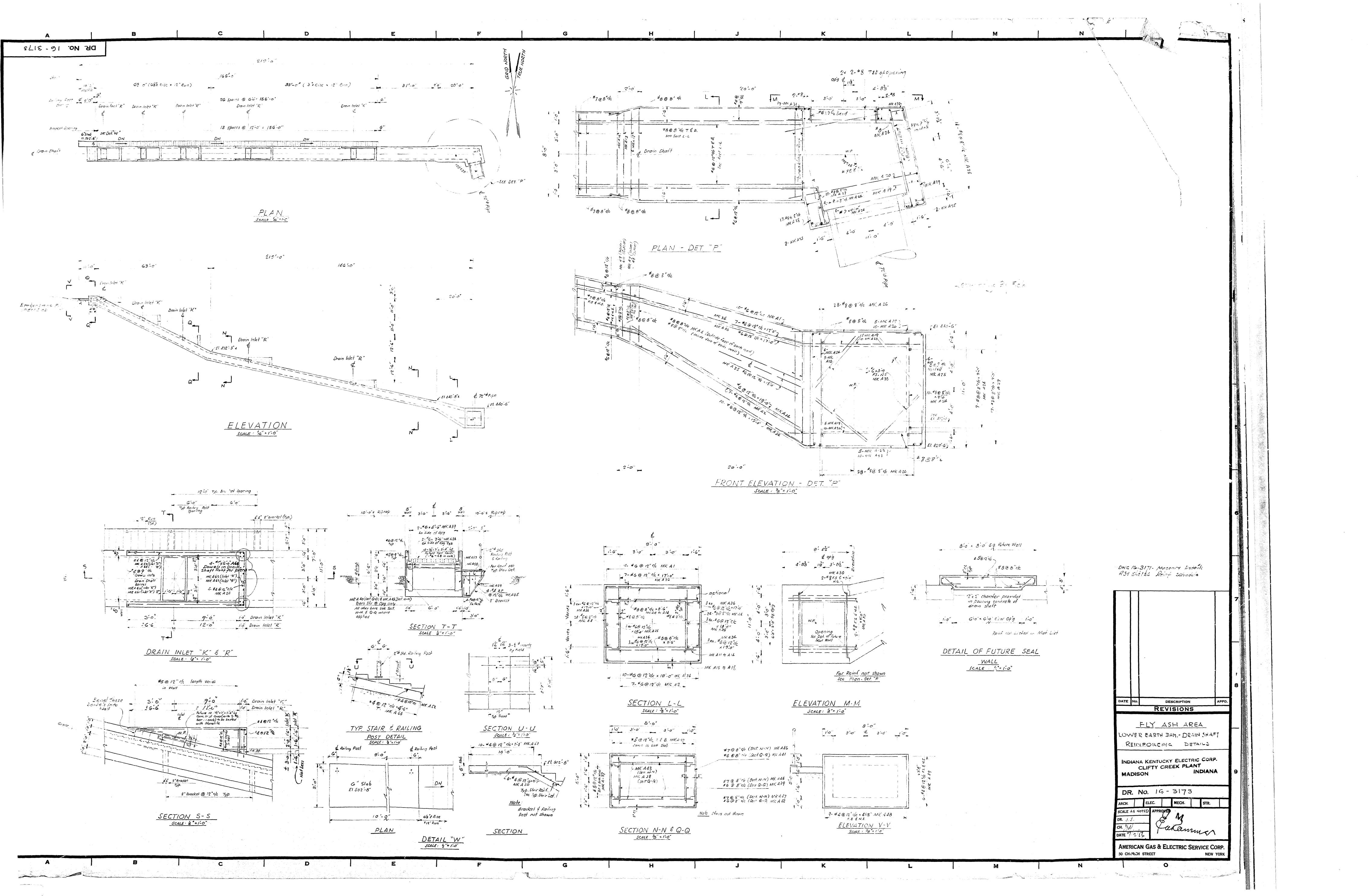
As-Built Design Drawings





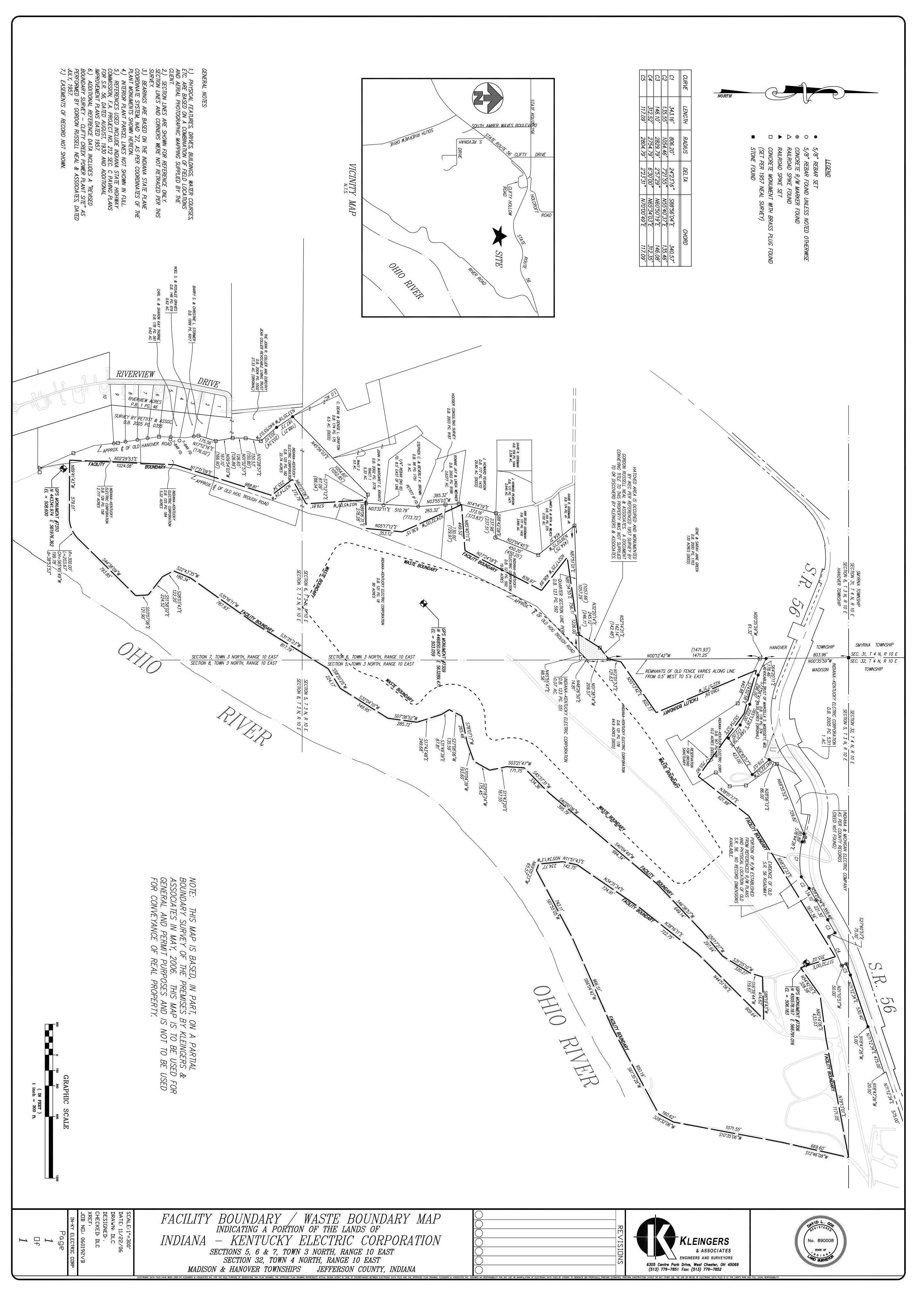






# **APPENDIX C**

Boundary Survey





November 22, 2006

# LEGAL DESCRIPTION INDIANA – KENTUCKY ELECTRIC CORPORATION FACILITY BOUNARY 357.74 ACRES

Situated in Section 5, Town 3 North, Range 10 East, Madison Township and in Sections 6 and 7, Town 3 North, Range 10 East, Hanover Township, Jefferson County, Indiana and being part of the lands conveyed to Indiana-Kentucky Electric Corporation and being more particularly described as follows:

Commencing at a stone found at the northwest corner of said Section 5 and the northeast corner of said Section 6;

Thence along the westerly line of Section 5 and the easterly line of Section 6, also being the line between Madison and Hanover Townships, S00°35'59"E a distance of 803.96 feet;

Thence continuing along said section line, S00°12'42"E a distance of 1471.25 feet to a concrete monument found at the true Point of Beginning;

Thence along the lines or through the lands of the Indiana-Kentucky Electric Corporation the following sixty five (65) courses:

- 1) N12°01'53"E a distance of 121.63 feet;
- 2) N55°13'42"E a distance of 602.73 feet;
- 3) N22°57'31"W a distance of 1060.09 feet;
- 4) S74°02'58"E a distance of 440.98 feet;
- 5) S53°54'53"E a distance of 755.90 feet:
- 6) N38°01'17"E a distance of 621.98 feet:
- 7) N58°22'33"E a distance of 1611.56 feet;
- 8) S17°32'00"E a distance of 355.02 feet;
- 9) N54°42'01"E a distance of 328.58 feet;
- 10) N82°14'06"E a distance of 433.03 feet;
- 11) N79°13'01"E a distance of 1171.05 feet;
- 12) S12°46'08"W a distance of 669.60 feet;
- 13) S10°35'06"W a distance of 1071.55 feet;
- 14) S28°32'06"W a distance of 160.62 feet;
- 15) S61°35'28"W a distance of 950.19 feet;
- 16) S66°04'42"W a distance of 966.51 feet;
- 17) S61°55'05"W a distance of 742.11 feet:
- 18) N69°05'23"W a distance of 65.57 feet;
- 19) N05°34'13"W a distance of 234.77 feet;

- 20) N11°51'43"E a distance of 142.75 feet;
- 21) N34°31'34"E a distance of 734.91 feet;
- 22) N38°16'11"E a distance of 722.75 feet;
- 23) N44°51'56"E a distance of 909.64 feet;
- 24) S88°19'43"W a distance of 414.62 feet;
- 25) S16°39'44"W a distance of 119.61 feet;
- 26) S31°25'10"W a distance of 320.27 feet;
- 27) S50°17'27"W a distance of 297.64 feet;
- 28) S46°38'57"W a distance of 649.14 feet;
- 29) S40°04'49"W a distance of 994.34 feet:
- 30) S40°00'06"W a distance of 395.79 feet;
- 31) S45°37'31"W a distance of 334.36 feet;
- 32) S03°21'47"W a distance of 171.75 feet;
- 33) S11°43'20"E a distance of 167.55 feet:
- 34) S29°18'24"W a distance of 175.45 feet;
- 35) \$70°04'39"W a distance of 193.60 feet;
- 36) S78°07'17"W a distance of 265.48 feet;
- 37) S27°08'06"W a distance of 135.59 feet;
- 38) \$31°49'39"E a distance of 87.81 feet;
- 39) S17°43'48"E a distance of 249.68 feet;
- 40) S07°38'52"W a distance of 285.72 feet;
- 41) S32°04'10"W a distance of 549.95 feet;
- 42) S58°20'25"W a distance of 224.17 feet;
- 43) S31°35'23"W a distance of 817.79 feet;
- 44) S35°16'11"W a distance of 787.93 feet;
- 45) S22°43'35"W a distance of 180.34 feet;
- 46) \$28°03'43"E a distance of 122.20 feet;
- 47) S51°58'02"E a distance of 224.52 feet;
- 48) \$03°07'06"E a distance of 121.90 feet;
- 49) S44°38'09"W a distance of 793.85 feet;
- 50) Along a curve to the right, an arc distance of 203.67 feet, said curve having a central angle of 38°53'53", a radius of 300.00 feet, and a chord bearing S65°45'49"W for 199.78 feet;
- 51) N89°41'43"W a distance of 579.01 feet;
- 52) N02°29'53"E a distance of 1024.08 feet;
- 53) N17°31'09"E a distance of 988.81 feet;
- 54) N37°14'24"W a distance of 255.34 feet;
- 55) N60°25'06"E a distance of 272.79 feet;
- 56) N03°45'09"W a distance of 579.81 feet;
- 57) N89°06'31"E a distance of 278.28 feet;
- 58) N05°17'12"E a distance of 353.12 feet;
- 59) N24°32'05"W a distance of 636.15 feet;
- 60) N85°40'11"E a distance of 449.52 feet; 61) N27°24'18"E a distance of 839.97 feet;
- 01)1127 2110 E a distance of 039.97 feet,
- 62) N52°17'25"W a distance of 406.99 feet;
- 63) N81°34'55"E a distance of 756.11 feet;
- 64) N32°20'14"E a distance of 245.12 feet;

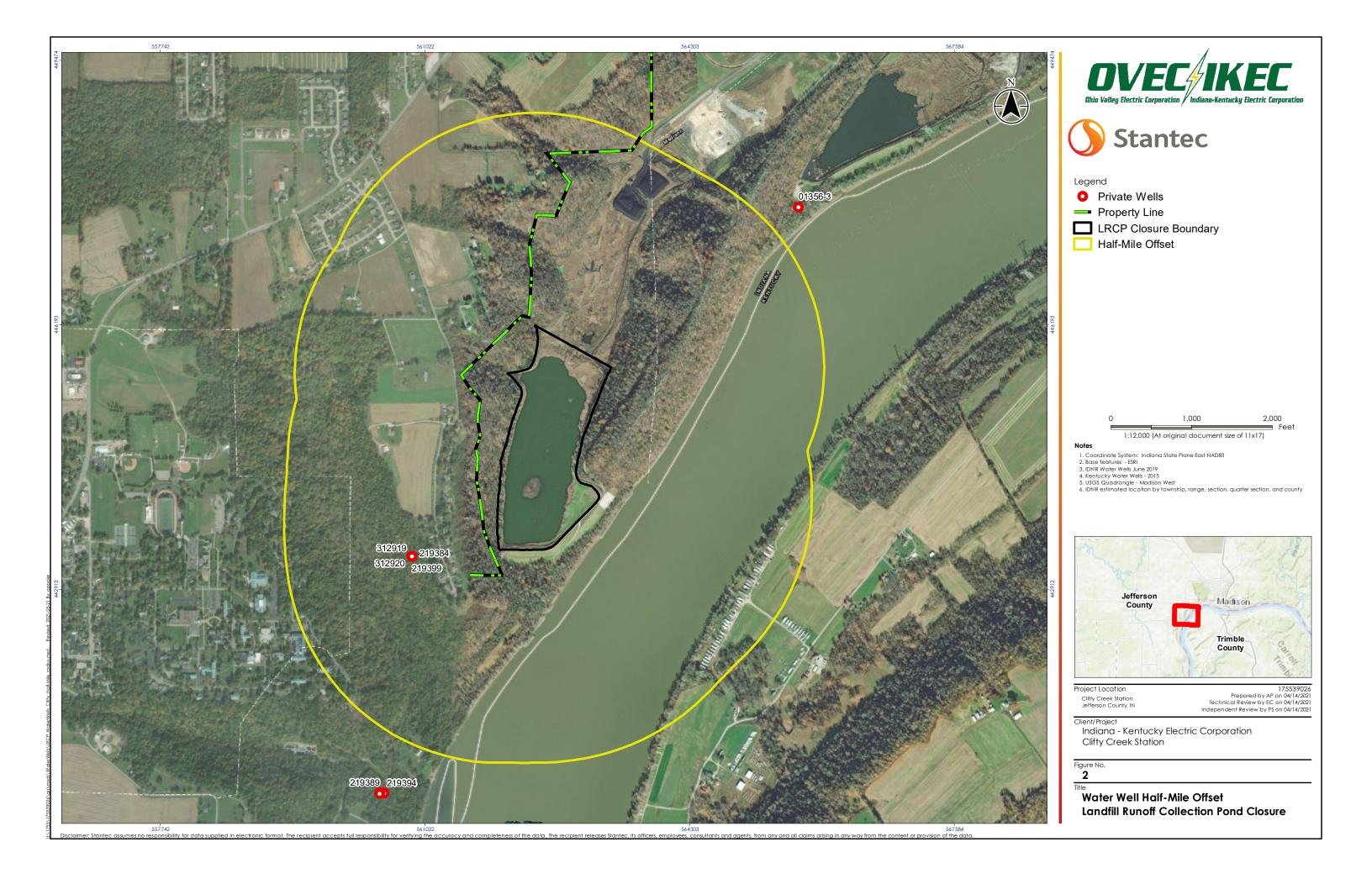
65) N52°14'21"E a distance of 142.14 feet to the Point of Beginning, containing 357.74 acres, more or less.

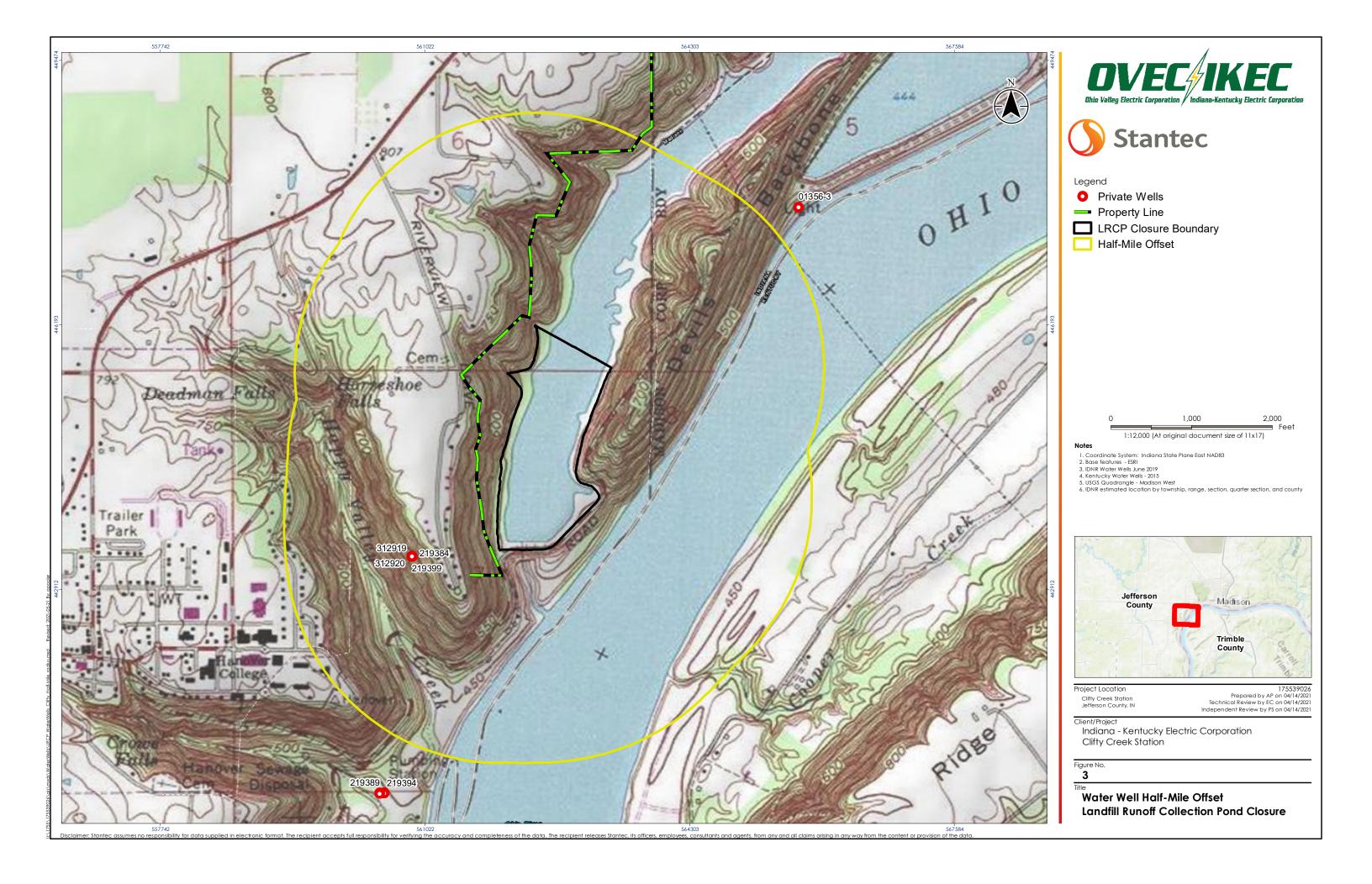
Bearings are based on the Indiana State Plane Coordinate System, NAD '27 as per the coordinates of the plant monuments provided by others.

This description was prepared by Kleingers & Associates, Inc., under the direction of David L. Cox, Indiana Licensed Surveyor No. 890008 and is based on a partial survey of the subject property performed in April and May, 2006. This description is to be used for permit purposes only and is not for the conveyance of real property.

# **APPENDIX D**

Location Figures





#### **Record of Water Well Indiana Department of Natural Resources**

Reference **Driving Direction to Well Date Completed** Number

219384 2100W, 300N OF SE CORNER 6/30/1967

Owner-Name **Address Telephone** Contractor HANOVER COLLEGE Not available Not available Owner Driller LAYNE NORTHERN **INDIANAPOLIS** Not available

Operator LESTER HOYT License Not available

**Construction Details** 

Well Use: Not available **Drilling Method:** Not available Pump Type: Not available

Pump Setting Depth: Not **Depth:** 68.0 Water Quality: Not available available

Length: Not available Material: Not available Diameter: 12.0 Casing Screen **Length: 15.0** Material: Not available Diameter: 12.0

Slot Size: 50

**Well Capacity** 

Type of Test: Not available Test Rate: 720.0 gpm Bail Test Rate: Not available Test

> Drawdown: 13.0 ft. Static Water Level: 30.0 ft. Bailer Drawdown: Not available

Grouting Material: Not available **Depth:** From (not available) To (not available) Information

> Installation Method: Not available Number of Bags Used: Not available

Well Sealing Material: Not available Depth: From (not available) To (not available) **Abandonment** 

Installation Method: Not available Number of Bags Used: Not available

**Administrative County: JEFFERSON** Township: 3N

> Section: SE of the SE of the SW of Section 7 Range: 10E

Topo Map: MADISON WEST, IN-KY Grant: Not available

Field Located By: Not available Field Located On: Not available

Courthouse Location By: Not available Courthouse Location On: Not available Location Accepted w/o Verification By: Not Location Accepted w/o Verification On: Not

available available

Subdivision Name: Not available Lot Number: Not available Ft W of EL: Not available Ft N of SL: Not available Ft E of WL: Not available Ft S of NL: Not available

Ground Elevation: Not available Depth of Bedrock: Not available Bedrock Elevation: Not available Aquifer Elevation: Not available **UTM Easting:** Not available **UTM Northing:** Not available

Well Log

Тор	Bottom	Formation
0.0	30.0	TOPSOIL & SOFT RED CLAY
30.0	33.0	GRITTY YEL CLAY
33.0	55.0	C. SAND AND GRAVEL W/ BOULDERS
55.0	60.0	M TO F SAND W/ BOULDERS
60.0	68.0	BLUE GREEN SHALE

Comments **CHEMICAL ANALYSIS** 

## Record of Water Well Indiana Department of Natural Resources

Reference Driving Direction to Well Date Completed

219399 SW 1/4 OF SE 1/4 10/11/1948

Owner-ContractorNameAddressTelephoneOwnerHANOVER COLLEGEHANOVERNot availableDrillerA D COOKNot availableNot available

Operator Not available License Not available

**Construction Details** 

Well Use: Not available Drilling Method: Not available Pump Type: Not available

Depth: 69.0 Pump Setting Depth: Not available Water Quality: Not available

Casing Length: Not available Material: Not available Diameter: Not available Screen Material: Not available Diameter: Not available

Slot Size: Not available

Well Capacity
Test

Type of Test: Not available

Test Rate: 250.0 gpm

Bail Test Rate: Not available

Drawdown: Not available Static Water Level: Not Bailer Drawdown: Not available

available

Grouting Information Material: Not available Depth: From (not available) To (not available)

Installation Method: Not available Number of Bags Used: Not available

Well Abandonment Sealing Material: Not available Depth: From (not available) To (not available)

**Installation Method:** Not available **Number of Bags Used:** Not available

Administrative County: JEFFERSON Township: 3N

Range: 10E Section: SW of the SE of Section 7

**Topo Map:** MADISON WEST, IN-KY **Grant:** Not available

Field Located By: Not available Field Located On: Not available

Courthouse Location By: Not available

Location Accepted w/o Verification By: Not

Courthouse Location On: Not available

Location Accepted w/o Verification On: Not

available available

Subdivision Name:Not availableFt W of EL:Not availableFt E of WL:Not availableFt E of WL:Not availableFt S of NL:Not available

Ground Elevation: Not available

Bedrock Elevation: Not available

UTM Easting: Not available

UTM Northing: Not available

UTM Northing: Not available

Well Log

Тор	Bottom	Formation
0.0	11.0	CLAY
11.0	33.0	LIME
33.0	57.0	S&G
57.0	60.5	FINE SAND
60.5	64.0	CLAY
64.0	69.0	CRS GRAV

**Comments** 

None

## Record of Water Well Indiana Department of Natural Resources

Reference Driving Direction to Well Date Completed

312919 Not available Not available

Owner-ContractorNameAddressTelephoneOwnerHANOVER COLLEGEHANOVERNot availableDrillerA.D. COOKNot availableNot available

Operator Not available License Not available

**Construction Details** 

Well Use: Not available Drilling Method: Not available Pump Type: Not available

Depth: Not available

Pump Setting Depth: Not available

Water Quality: Not available

Casing Length: Not available Material: Not available Diameter: Not available Screen Length: Not available Material: Not available Diameter: Not available

Slot Size: Not available

Well Capacity
Type of Test: Not available
Test Rate: Not available
Bail Test Rate: Not available

Drawdown: Not available

Static Water Level: Not

Bailer Drawdown: Not available

available

Grouting Information Material: Not available Depth: From (not available) To (not available)

Installation Method: Not available Number of Bags Used: Not available

Well
Abandonment
Sealing Material: Not available
Depth: From (not available) To (not available)

Installation Method: Not available Number of Bags Used: Not available

Administrative County: JEFFERSON Township: 3N

Range: 10E Section: SW of the SE of Section 7

Topo Map: MADISON WEST, IN-KY Grant: Not available

Field Located By: Not available Field Located On: Not available

Courthouse Location By: Not available Courthouse Location On: Not available

Location Accepted w/o Verification By: VCH
Subdivision Name: Not available

Location Accepted w/o Verification On: 6/1/1966
Lot Number: Not available

Ft W of EL: Not available
Ft E of WL: Not available
Ft S of NL: Not available
Ft S of NL: Not available

Ground Elevation: Not available

Bedrock Elevation: Not available

UTM Easting: Not available

UTM Northing: Not available

UTM Northing: Not available

Well Log Comments ABANDONED Test

## Record of Water Well Indiana Department of Natural Resources

Reference Driving Direction to Well Date Completed

312920 Not available 1/1/1940

Owner-ContractorNameAddressTelephoneOwnerHANOVER COLLEGEHANOVERNot availableDrillerA.D. COOKNot availableNot available

Operator Not available License Not available

**Construction Details** 

Well Use: Not available Drilling Method: Not available Pump Type: Not available

Depth: 80.0 Pump Setting Depth: Not available Water Quality: Not available

Casing Length: Not available Material: Not available Diameter: Not available Screen Length: 20.0 Material: Not available Diameter: Not available

Slot Size: Not available

Well Capacity

Type of Test: Not available

Test Rate: Not available

Bail Test Rate: Not available

Drawdown: Not available Static Water Level: Not Bailer Drawdown: Not available

available available

Grouting Information Material: Not available Depth: From (not available) To (not available)

Installation Method: Not available Number of Bags Used: Not available

Well Abandonment Sealing Material: Not available Depth: From (not available) To (not available)

Installation Method: Not available Number of Bags Used: Not available

Administrative County: JEFFERSON Township: 3N

Range: 10E Section: SW of the SE of Section 7

Topo Map: MADISON WEST, IN-KY
Field Located By: Not available
Field Located On: Not available

Courthouse Location By: Not available

Location Accepted w/o Verification By: Not

Courthouse Location On: Not available

Location Accepted w/o Verification On: Not

available available

Subdivision Name:Not availableFt W of EL:Not availableFt N of SL:Not available

Ft E of WL: Not available

Ground Elevation: Not available

Depth of Bedrock: Not available

April of Bedrock: Not available

April of Bedrock: Not available

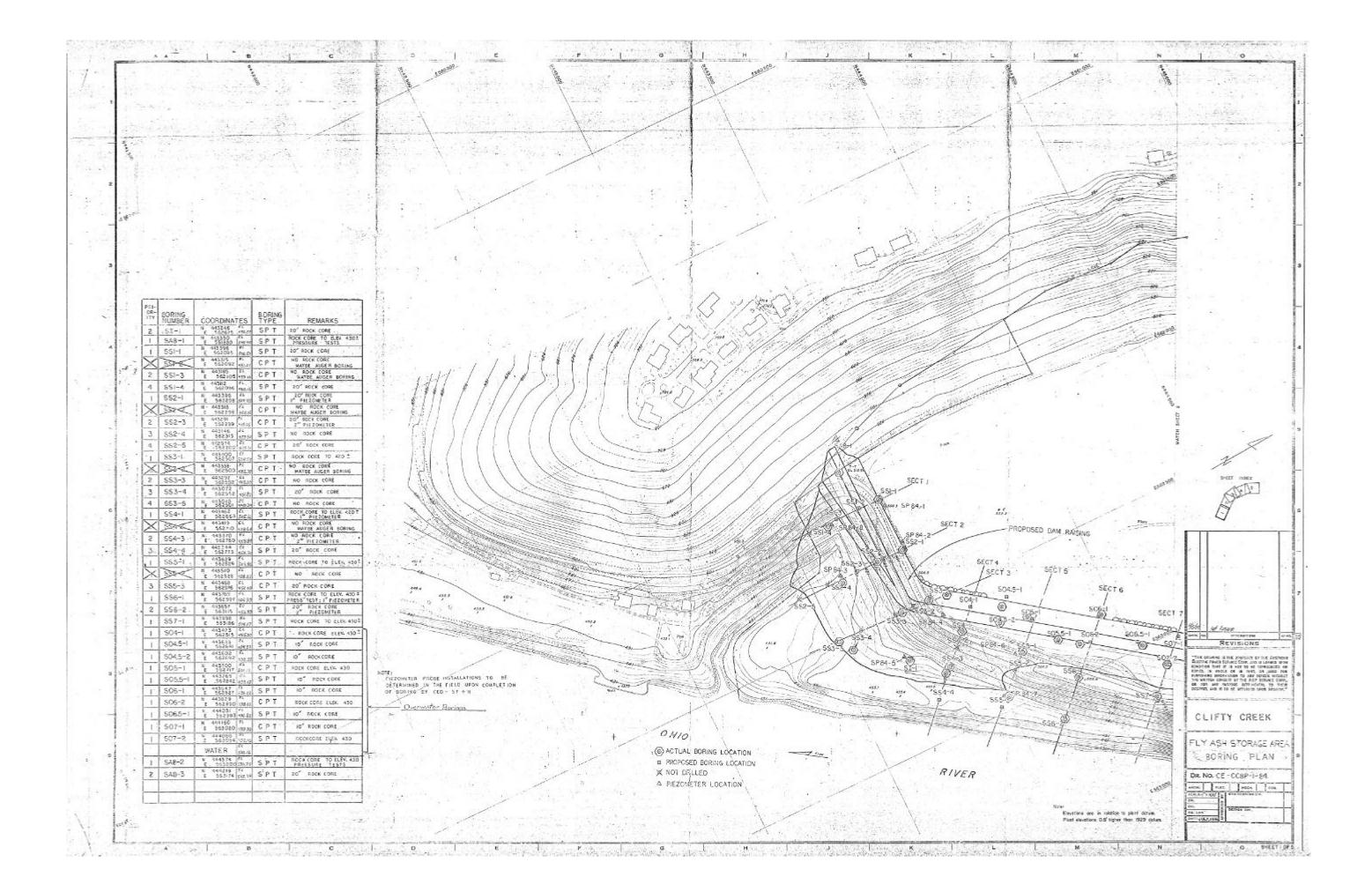
Bedrock Elevation: Not available
UTM Easting: Not available
UTM Northing: Not available
UTM Northing: Not available

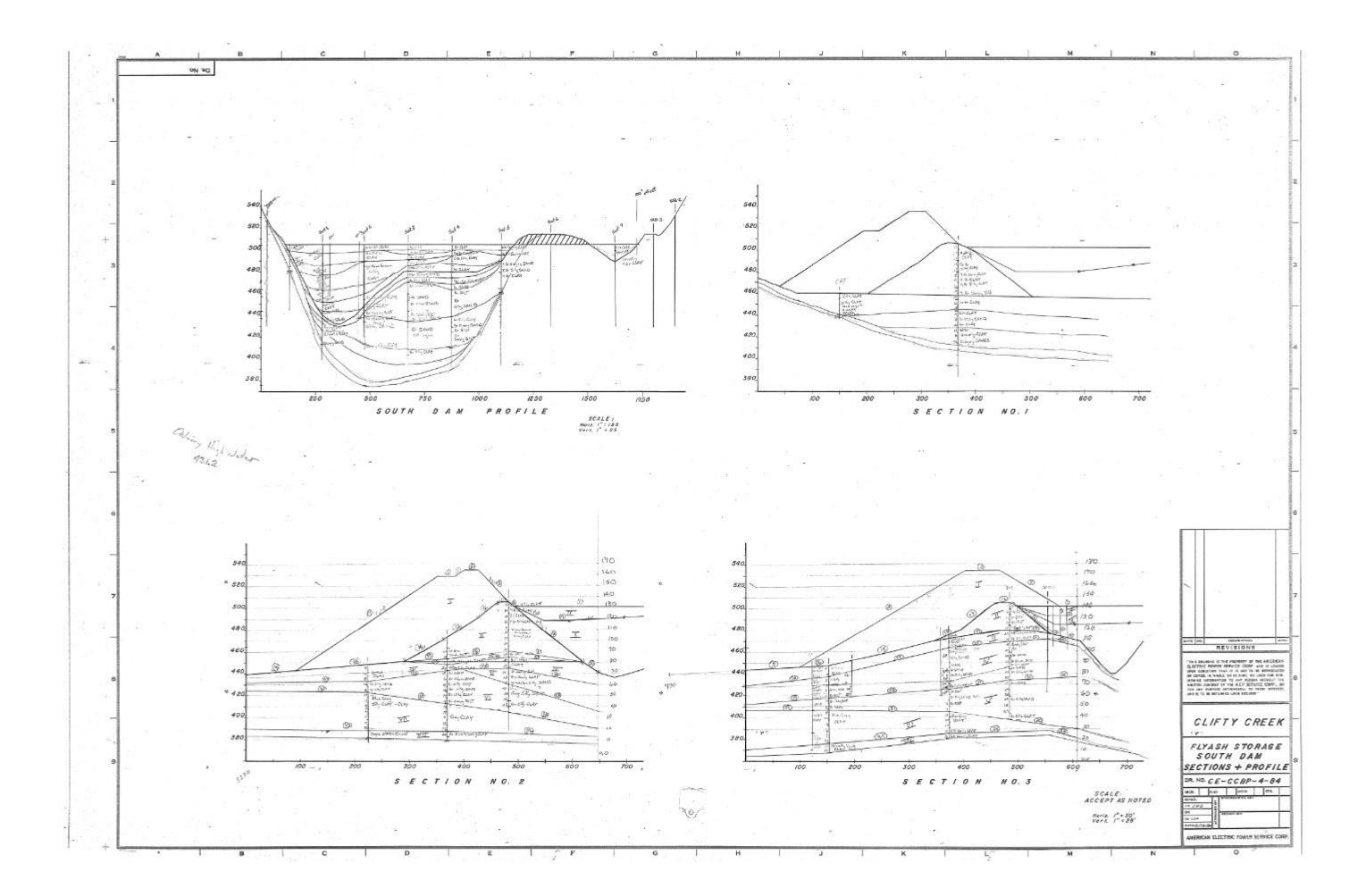
Well Log Comments SAND AND GRAVEL; S.WL. 9/49-35' AND PUMPING LEVEL OF 44'; S.W.L. 1950-48' PUMPING LEVEL-40'; S.W.L. 8/50-24' PUMPING LEVEL 16'

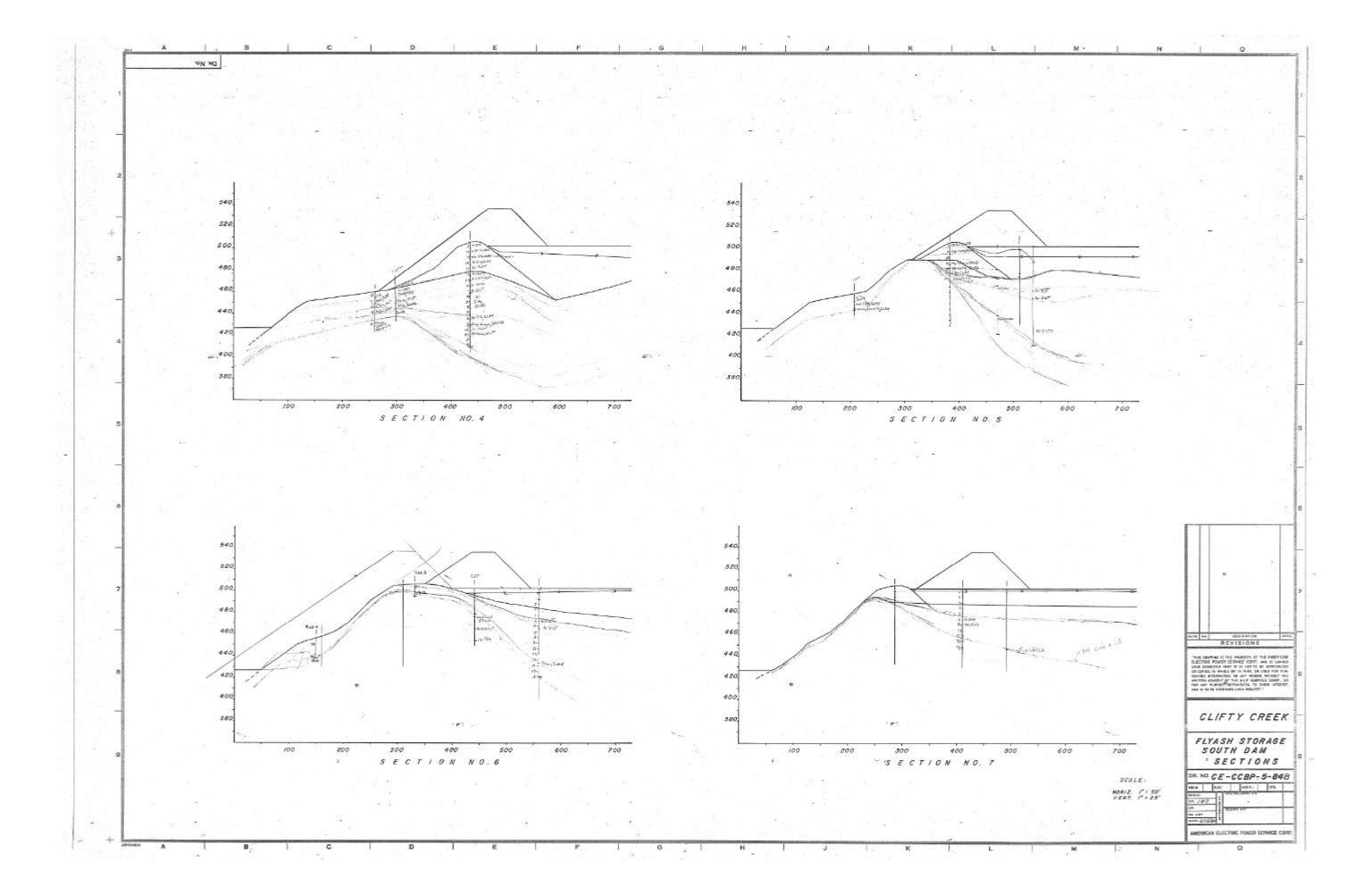
### **APPENDIX E**

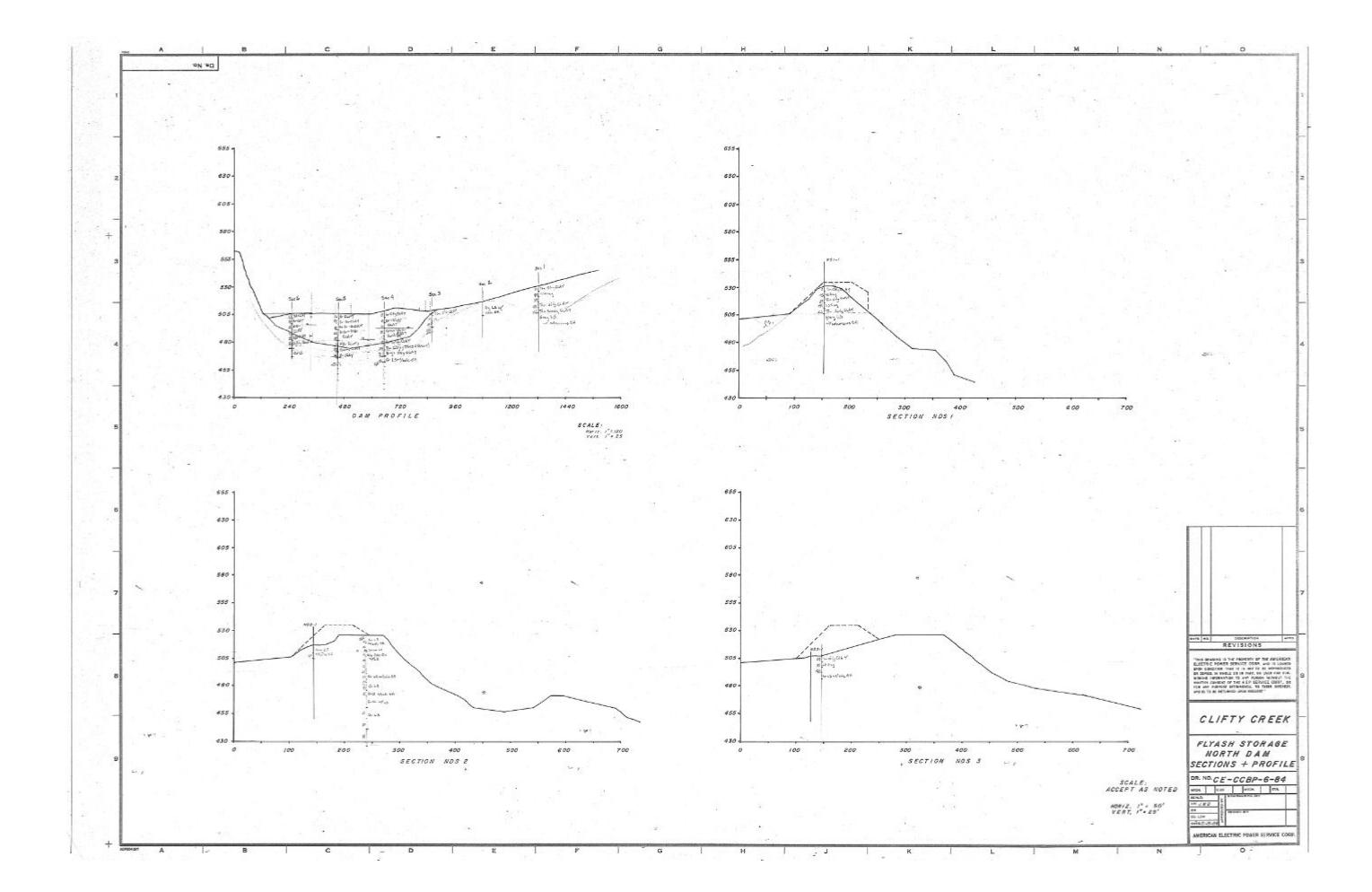
Geotechnical Data

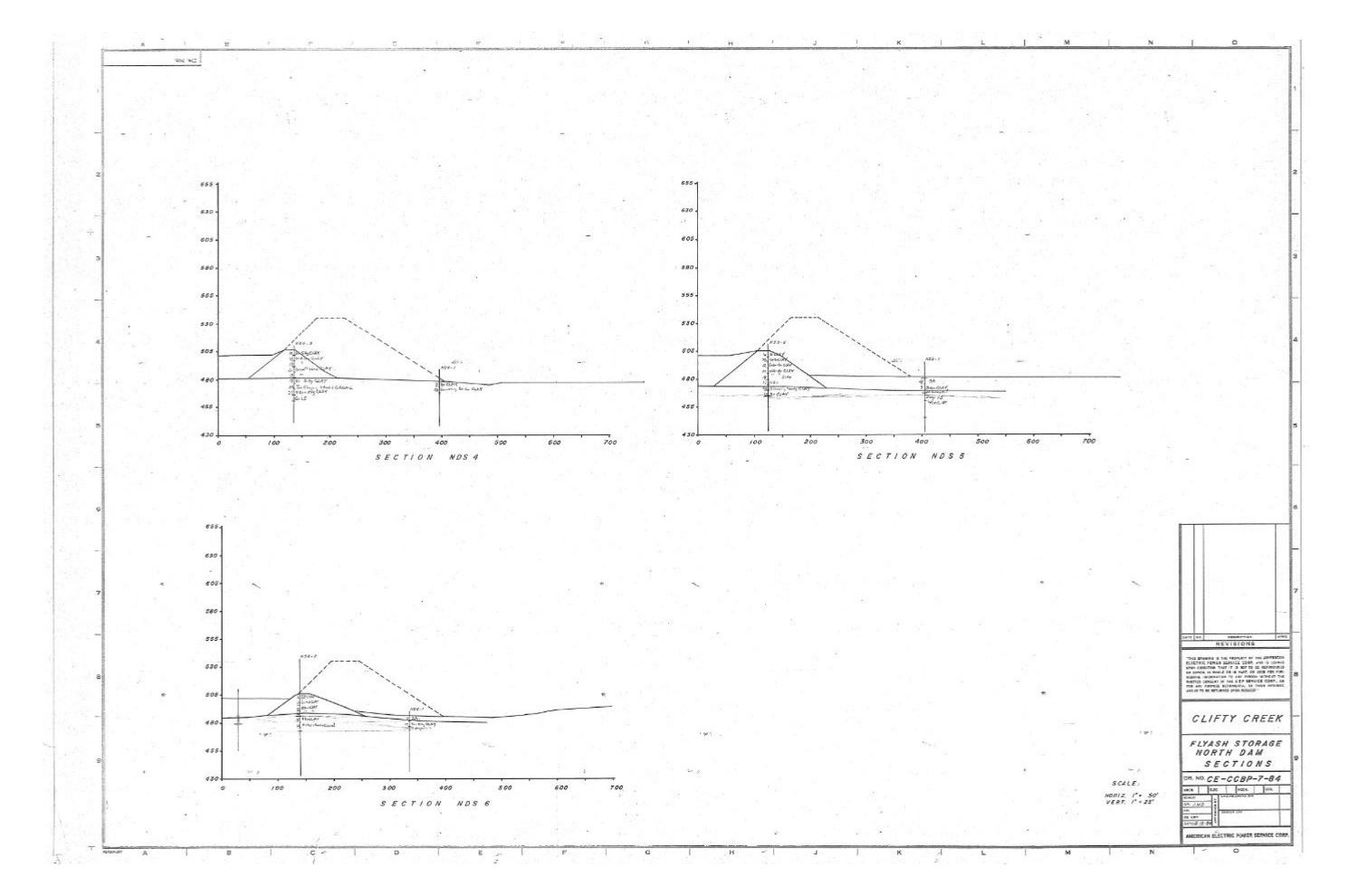
AGESC (1956)











AGESC (1984)

# Indiana - Kentucky Electric Corporation Clifty Creek Plant Madison, Indiana

Flyash Dam Raising Feasibility Report

APPENDIX G Boring Logs

January 31, 1985

Prepared by

Soils, Foundation and Hydro Section Civil Engineering Division American Electric Power Service Corporation

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## AMERICAN ELECTRIC POWER SERVICE CORPORATION A

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FORM CE-5

#### AMERICAN ELECTRIC POWER SERVICE CORPORATION

# AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

\									BORING NO. 134-Z DATE SHEET Z OF 3			
ROJI	ECT								Type of Borine Rig			
		F Boni							CASING USED SIZE DRILLING MUD USED			
					,		·		BORING BEGUN BORING COMPLETED			
	EN LEY	/EL							GROUND ELEVATION REFERRED TO			
TIM									Dat			
DAT	t								FIELD PARTY:			
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SAMP						<b>F</b>   F   F   F   F   F   F   F   F   F	FE		· ·			
7	FROM	TO	BL	ows /	6	1	ļ	<u> </u>	PLUCTUATIONS IN WATER LEVEL, NOTES ON BRILLING EASE, ETC.			
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	ļ	-			-			2 -	- GRAVELLY SAND-CHY- BR-MOIST - I'MAX SIZE - LIMESTONE FRAM.			
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<u>5</u>	22.5	24.0	10	7	10	3_		3	1 max size - LimesTone Frage			
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7_	32.5	34.0	7_	9	15	7"		3	- 1/2 max Size - Lime STane Fang.			
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## AEP CIVIL ENUMEERING LABORATORY

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inpany_							Boring No. N54-2 Date Sheet 3 of 3
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ELEVATION FEET	SAMPLE	Sample from (In f	ROD	Recovery (in feet)	Depth In Feet	Graph Log	Rock Type. Color, Quality, Drilling observations, Depths water lost, Observed fluctuations in water level, notes on drilling ease, etc.
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#### AMERICAN ELECTRIC POWER SERVICE CORPORATION

## AFP CIVIL ENGINEEDING LAROPATORY

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		F BOR	<u>/</u>		_					CASING USED SIZE DRILLING MUD USED					
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	TER LEY	VEL			7.5					GROUND ELEVATION 479.40 REFERRED TO					
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DAT	<u> </u>									FIELD PARTY: Roush - LamberT					
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SAMPLE	FROM	a TO	l a	LOWS	/ 6"	F.J		FEE	. 1	FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.					
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tion	of Bo	oring:					Casing used Size Drilling mud used
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TION	E 78.	Sample Deptrifrom-to (in feet)	ROD	very	5 = 4	Log	DESCAIPTION  Rock Type, Color, Quality, Orilling observations, Depths
ELEVATION	SAMPLE NO.		e.	Recovery (in feet)	Bepth In Feet	Graph	water lost. Observed fluctuations in water level, notes on drilling ease, etc.
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### ACH CIVIL ENGINEERING LABORATORY

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Company_							Boring No. 3 - Date Sheet 3 of				
Project _				·			Boring No. DateSheet 3 of Type of BoringRig Casing usedSizeDrilling mud used				
cation	of Bo	rıng:					Casing used Size Drilling mud used				
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# AEF CIVIL ENGINEERING LABORATORY LOG OF BORING

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ompany_							Boring No. 155-1 Date Sheet 4 of 5
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cation	of Bo	ring:			,	_	Casing used Size Drilling mud used
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### AEF CIVIL ENUMEERING LASSICATORY

Job No					_		
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							Boring No Date Sheet _5 of _2  Type of Boring Rig  Casing used Size Drilling mud used
cation							Casing used Size Drilling mud used
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ELEVATION FEET	LE NO.	Sample Depth from-to (In feet)	800	Recovery (In feet)	Depth In feet	aph Log	DESCRIPTION  Rock Type, Color, Quality, Drilling observations, Dep is water lost, Observed fluctuations in water level, notes
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FORM CE-5

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WAT	ER LEY	/EL		2S.	5					GROUND ELEVATION 504. 59 REFERRED TO
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FORM CE-5 REV. 5/81

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PROJE	ECT									TYPE OF BORING RIG
Loc	ATION O	F BORE	H G I							CASING USED DRILLING MUD USED
										BORING BEGUN BORING COMPLETED
	ER LEV	ZL .								GROUND ELEVATION REFERRED TO
TIM										DATU
DAT	Ľ						· · ·			FIELD PARTY:
ž	_	APLE PTH		TANDA NETRA		AL FRY	DEPTH			DESCRIPTION Soil Type, color, Texture, Consistency, Sampler Driving Hotes
7	IN I	FEET	RE	SISTA	NCE	5ZÓ	ELEVATION			BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OSSERVED
SAMPLE	5000		١ ,	LOWS /	, a <sup>11</sup>	TOTA LENGT RECOVE		FEE	T	FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
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				CER	140/6		FAD	<del>_</del>	CASING USED SIZE DRILLING MUD USED
Loca	TION O	F BORI	N6:						BORING BEGUN 8-21-84 BORING COMPLETED 8-21-84
WAT	ER LEV	EL		11.0	)				GROUND ELEVATION REFERRED TO
Tim	t								DAT
DATI	t		· · · · · · ·			•			FIELD PARTY: Roush + LamberT
Š	SAN	APLE	·s	TANDA	RD	, x &			DESCRIPTION
w		PTH	1	NETRA		ZE S	ELEVATION	DEPT	H SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES
APLE.	IN I	FEET	RE	SISTA	NCE	TOTAL LENGTH RECOVERY	ECE VAI IQII	FEE	T BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OSSERVE
3	FROM	TO	BI	LOWS /	<b>′</b> 6"	72	l L	·	FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
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1	2.5	4.0	7	8	5	3"	· ·	3 -	- Limp STONE FROD - MOIST - TRACE OF ORG,
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### AER CIVIL ENGINEERING LABORATORY

lab No	<u> </u>					•	,
							Boring No Date Sheet 2 of 3 _
							Type of BoringRigRigRig
Ition	of Bo	ring:					Casing used Size Drilling mud used
		· · · · · · · · · · · · · · · · · · ·					Boring begun Boring completed
Water Lev	el						Ground Elevation referred to Datum
Time					· · · · · · · · · · · · · · · · · · ·		Field Party:
Date							rield Faity.
ELEVATION	SAMPLE NO.	Sample Deptiviron-to	ROD	Recovery (in feet)	Depth 10 feet	Graph Log	DESCRIPTION  Rock Type. Color. Quality. Drilling observations. Dept: water lost. Observed fluctuations in water level. notes on drilling ease, etc.
·	<i>=</i> _	14,9	35%	4.1	15" -		GRAY HARD LIME STONE W/ CALCAREOU.  Shale Laxers
					16		
<u> </u>					-/7		CACAREOUS Shale  CORE host?
					- 18 -		GRAJ HARD LIMESTONE WY CALCARTOUS Shale LAYERS
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					20		STAPPED BOKING 19,5
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				<u> </u>	rek	No.	eTH	F.A.D	_		TYPE OF BORING SOT - COR C RIG 8-6/  CASING USED SIZE DRILLING MUD USED
		ATION O		H 6 :							BORING BEGUN 8-21-84 BORING COMPLETED 8-21-84
7 3	TIM	ER LEV	EL		//.	<u> </u>					GROUND ELEVATION 482.88 REFERRED TO DATUM
	DAT	t									FIELD PARTY: Roush- Eamber T
	ŝ	SAN	APLE	SI	ANDA	RD	]±&				DESCRIPTION
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#### AET CIVIL ENGINEERING LABORATORY

Jab No			· · · · · ·			•	
Сопралу_							Boring No. — Date Sheet 2 of
Project							Type of BoringRigRigRig
ation	of Bo	ring:					Casing used Size Drilling mud used
		<del> </del>					Boring begun Boring completed
Water Lev	/el						Ground Elevation referred to
Time							Datur
Date		ł					Field Party:
ELEVATION FEET	SAMPLE NO.	Sample Deptrifrom-to	RaD	Recovery (in feet)	Depth In feet	Graph Log	DESCRIPTION  Rock Type, Color, Quality, Orilling observations, Dep. s water lost, Observed fluctuations in water level, notes on drilling ease, etc.
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# ALP CIVIL ENGINEERING LABORATORY

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							Boring No Date She	eet <u>3</u> of <u>5</u>
<u> </u>						<del></del>	Type of BoringRig Casing usedSize Drilling muc	d used
		iring:					Boring begun Boring complete	d
ter Le	vel						Ground Elevation referred to	
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ELEVATION	SAMPLE NO.	Sample Deptrirem-to	Rad	Recovery (In feet)	Depth 1n feet	Graph Log	DESCRIPTION  Rock Type. Color. Quality. Drilling observa water lost. Observed fluctuations in water on drilling ease, etc.	ations. Depth level, notes
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# AEF CIVIL ENGINEERING LABORATORY LOG OF BORING

Company	of Bo	ring:				Boring No. Sheet 4 of 5 Type of Boring Rig Casing used Size Drilling mud used Boring begun Boring completed Ground Elevation referred to  Datu Field Party:	
ELEVATION FEET	SAMPLE NO.	Sample Deptr from-to (in feet)	Rad	Recovery (in feet)	Depth 1n feet	Graph Log	DESCRIPTION  Rock Type. Color. Quality. Drilling observations. Det he water lost. Observed fluctuations in water level. notes on drilling ease. etc.
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# ACT CIVIL ENGINEERING LABORATORY LOG OF BORING

Jab No.		£00 01	
Company		:	Boring No. V36-1 Date Sheet 5 of 5
:			Type of BoringRigRig
cation of E	Boring:		Casing used Size Drilling mud used
			Boring begun Boring completed
Vater Level			Boring begun Boring completed Ground Elevation referred to Data
Date			Field Party:
		· · · · · · · · · · · · · · · · · · ·	
	Sample Deptifrom-to (in feet) ROD	_	DESCRIPTION
ELEVATION FEET	o et o	Recovery (in feet) Depth in feet Graph Log	
E FEE	nple rom-t	n feet n feet In feet feet	Rock Type, Color, Quality, Orilling observations, Depths water lost, Observed fluctuations in water level, notes on drilling ease, etc.
급 :	eg :	E 5 5	Un Urilling ease, etc.
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Loc. WAT	ATION OF	LKE NFT BORN	YCE	ee l	ζ F	AD			BORING NO. NSG-Z DATE 8-16-84 SHEET 1 OF 3  TYPE OF BORING SPI - CORC RIG B-61  CASING USED SIZE DRILLING MUD USED  BORING BEGUN 8-16-84 BORING COMPLETED 8-16-84  GROUND ELEVATION 505.39 REFERRED TO		
DAT									FIELD PARTY: Roush- LAMbert		
SAMPLE No.	DE	FEET	PEN RES	TANDA IETRAT SISTAI .OWS /	TION	TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	Joint Fire, County, Ferroncy, Constituting, County, Constitution of the County of the		
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3	12.5	140	5_	15	8	3"		2 —	Clay- Blue Green moist, STRONG REACTION TO HCL		
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# AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY

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Location of Boring									TYPE OF BORING RIG	
Lœ	ATION O	F Bori	NG:					CASING USED SIZE DRILLING MUD USED		
Missandaria								BORING BEGUN BORING COMPLETED		
TIME								GROUND ELEVATION REFERRED TO DATU		
DATE									FIELD PARTY:	
Š	SAMPLE STANDARD								DESCRIPTION	
,	DEPTH IN FEET		PENETRATION RESISTANCE BLOWS / 6"			TOTAL LENGTH RECOVERY		DEPTH IN FEET		
SAMPLE						528	ELEVATION			
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Job No.					FO	<b>6 6</b>	DP BURING
							Boring No. 1/56-2 Date Sheet 3 of 3
			,				Type of Boring Rig Casing used Size Drilling mud used
cation	of Bo	ring:					Casing used Size Drilling mud used
					<u> </u>		Boring begun Boring completed
Water Lev	el						Ground Elevation referred to Datu
Time							Field Party:
Date		<u> </u>					Fletu raity.
ELEVATION FEET	SAMPLE NO.	Sample from-t (in fee	800	Recovery (in feet)	Depth In feet	Graph Log	DESCRIPTION  Rock Type, Color, Quality, Drilling observations, Dec 19 water lost, Observed fluctuations in water level, not 5 on drilling ease, etc.
		35.5	189	4.6	<b>t</b>	#	35.5 - 40.2
		40.2	60/0	4.6	Ł.	1	CALCAREOUS SHALL LAYERS
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# AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

		-KE							BORING NO. MAB-S DATE 8-27-84 SHEET 1 OF 5
PHOJE	ECT _C	LET	CR	حدلا	<u>FA</u>	D			Type of Borine Core NQ Rig B-6/
Loc	ATION O	F BORI	HG:						CASING USED SIZE DRILLING MUD USED BORING BEGUN 17-27-84 BORING COMPLETED 8-30-84
WAT	ER LEV	/#1						$\dashv$	GROUND ELEVATION 529.62 REFERRED TO
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DAT		-+						$\dashv$	FIELD PARTY: Roush - Lumbert
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2		MPLE		PANDA		J.₹.		DEPTH	DESCRIPTION
SAMPLE		PTH FEET	_	IETRAT SISTAN		TOTAL LENGTH RECOVER	ELEVATION	IN	SOL TIPE, COOM, TEXTORE, CONSTITUTION, SHEPLER SHIPING HOLES
۱ <u>چ</u>			ř ·			25.5		FEET	
l <u>, ≴</u>	FROM	TO	BL	.ows /	6"	_ &			FLUCTUATIONS M WATER LEVEL, NOTES ON DRILLING EASE, ETC.
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r"							ROD	1 ° <u>-</u>	
	5.8	12.8				6.1	46%	_ و	
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}	12.8	144			<del> </del>	1.9	0	3 -	BROKEN Lime STONE + CA/CAREOUS
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FORM CE-5 REV. 5/8!

### AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY

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									BORING NO. MAB-J DATE SHEET OF
HOJE	CT	·							TYPE OF BORING RIG RIG RIG DRILLING MUD USED
Loc	TION O	F BORII	16:						
									BORING BEGUN BORING COMPLETED
WAT	ER LEV	EL							GROUND ELEVATION REFERRED TO
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DATI	t .								FIELD PARTY:
PLE No.	DE	IPLE PTH FEET	PEN	ANDA ETRAT	TION	TOTAL LENGTH RECOVERY	ELEVATION		DESCRIPTION  SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTE BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVE
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(	25.5	31.0		<u> </u>	ļ	5.0	32%	6 -	GONY CHICARCOUS Shale w/ Lime STOLE LAYERS
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	210	35.0		**		4.0	55%	_	STANE LAYERS
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#### AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

,	10. <u> </u>								MAB-3 Borine No Date Sheet 3 of 5
V I	CT								TYPE OF BORING RIG
		F Boni				-		$\overline{}$	CASING USED SIZE DRILLING MUD USED
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DAT	<u> </u>		_			_			FIELD PARTY:
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3	426	45.8				2.8	26%	١ ـ	- GRAY Lime STONE
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·y	50.0	55.8				4.9	25%		- Gany hime stone uf CAlenkeous
-	30.5	33.0				7.7	23/0	1-	Shale Layers
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FORM CE-5 REV. 5/81

# AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY

_	_									BORING NO DATE SHEET 4 OF 5
	-/-					•	-			TYPE OF BORING RIG
r										CASING USED SIZE DRILLING MUD USED
ı	Loca	TION O	BOR	ING:	•					BORING BEGUN BORING COMPLETED
t	WAT	ER LEV	EL							GROUND ELEVATION REFERRED TO
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		748	25.8	<u> </u>			10.0	39%		GRAY Lime STONE
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# AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

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~MOJI	ECT								1	TYPE OF BORING RIG
Loc	ATION O	F BORI	NG:							CASING USED SIZE DRILLING MUD USED
<del>  1000</del>	ER LEV	<u> </u>								BORING BEGUN BORING COMPLETED
TIM		EL						<u> </u>	G	GROUND ELEVATION REFERRED TO DATUM
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SAMPL								FEE'	,	BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED
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İ.	95. <b>9</b>	100				5.0	RQ D 83%	,	76	Gray Hard Lime Stone
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## BOWSER-MORNER, INC.

420 DAVIS AVENUE, P.O. BOX 51, DAYTON, OHIO 45401

#### **ENGINEERING REPORT**

REPORT TO: American Electric Power Service Corp.

REPORT DATE:

October 2, 1984

1 Riverside Plaza

P.O. Box 16631

Columbus, Ohio 43216

REPORT NO.:

28089-1084-425

Attention: Mr. Kevin C. Miller

Civil Engineering Division

REPORT ON:

Submitted of Field Boring Logs, Indiana-Kentucky Electric Corporation, Clifty Creek Power Station Flyash Inpoundment,

Madison, Indaina

Bowser-Morner, Inc. was authorized to provide drilling services at the subject site by contract number 1134 dated August 24, 1984.

All soil samples, rock cores, and field boring logs were given to A.E.P. drill crews working at site. Results of the cone penetration tests performed by Woodward Clyde Consultants have not been received by Bowser-Morner, Inc. as of this date. This information is expected to be received by October 5, 1984, and will be forwarded immediately upon receipt.

All field boring logs have been rewritten to help clarify the hand written submitted field logs. All soil strata were visually classified by the driller and no attempt was made to change his logs, but only to make them more legible.

Please contact the writer if ther are any questions.

Respectfully submitted,

Bowser-Morner, Inc.

RLD/lag(#7)2-Client 2-File

Robert L. Disney, Manager Soils Exploration Department

#### Coordinates, Elevations - Clifty Creek Ash Pond Borings and Piezometers

Boring No.	North	East	Grd. Elev.			
Dor ring no.	1101 011	Laso	did: Lice	<u>.</u>		
SS1-1	443,396.608	562,095.889	506.4'			
SS1 <b>-</b> 2	443,309.415	561,945.600	511.2			
SS1-3	443,185.435	562,105.878	457.5			
SS2 <b>-</b> 1	443,396.944	562,298.268	505.2		•	
SS2 <b>-</b> 3	443,291.332	562,299.155	473.5			
SS2 <b>-</b> 4	443 <b>,1</b> 46.177	562,315.152	444.0			
SS3 <b>-</b> 1	443,400.792	562,507.540	505.7		-	
SS3 <b>-</b> 3	443,292.943	562,502.207	475.0	C= 11 2	2.4	
SS3 <b>-</b> 4	443,072.526	562,552.619	453.1	554-3	_	
SS3 <b>-</b> 5	443,040.442	562,501.381	450.3	, N 443365.4	562761.5	4581
SS4 <b>-</b> 1	443,462.922	562,663.196	504.4	44 2 3 60 IT	762 7611 2	758
SS4 <b>-</b> 4	443,344.316	562 <b>,</b> 773 <b>.</b> 019	456.5	201-7		
SS5-1	443,629.159	562,826.808	504.0	555-3	562,954.6	453.
SS6 <b>-</b> 1	443,769.260	562,997.733		443510.0	362)137.5	(0)
SS6-2	443,657.692	563,115.269	452.4			
SS7-1	443,998.052	563,186,481	506.0			
SAB-1	443,311.870	561,942.766	511.2			
SAB-3	444,219.168	563,174.539	510.3		·	
SI-1	443,246.916	562,675.227	456.9			
NS1-1	449,764.876	566,714.021	530.6			
NS2-1	449,972.633	566,848.095	517.2			
NS5-1	450,696.599	567,097.703	481.8			
NS5-2	450,593.658	566,839.317	505.4			
NS5.5-1	450,706.318	566,947.011	483.5			
NS6-1	450,808.096	566,964.043	484.4			
NS6-2	450,742.242	566,781.840	506.2			
NS4-1	450,415.751	567,159.228	477.2		•	
NS4-2	450,380.723	566,901.629	505.7		•	
NAB-5	450,031.446	566,992.941	528 <b>.</b> 8		,	
NS3-1	450,153.057	566,937.097	507.7			
Piezometer	North	East		Elevations	5	
	<del></del>	<del></del>	Ground	Casing	Top Pip	<u>e</u>
SP84-1	443,404.716	562,121.849	505.30'	507.79'	507.70	1
SP84-2	443,404.130	562,270.565	504.66	508.48	508.38	
SP84-3	443,149.098	562,289.250	440.65	444.07	.444.02	
SP84-4	443,429.340	562,578.553	504.18	507.52	507.45	
SP84-5	443,231.782	562,657.726	457.60	460.52	460.46	
SP84-6	443,603.765	562,810.631	505.04	507.77	507.68	
SP84-7	443,497.555	562,943.715	454.42	457.37	457.29	
SP84-8	443,200.487	562,104.880	456.69	459.59	459.52	
NP84-1	450,320.291	566,927.815	505.59	508.27	508.19	
NP84-2	450,441.324	566,890.223	505.48	509.03	508.98	
NP84-3	450,660.383	566,703.275	502.62	505.79	505.69	
NP84-4	450,413.695	567,161.991	477.32	480.02	479.80	
NP84-5	450,692.879	567,088.078	481.80	484.92	484.92	

5

#### Coordinates - Clifty Creek Ash Pond Borings - <u>Underwater</u>

Boring No.	North	<u>East</u>
S04-1	443,473.371	562,515.058
S04.5-1	443,633.008	562,641.125
S04.5-2	443,602.971	562,692.785
S05-1	443,700.203	562,717.288
S05.5-1	443,765.818	562,842.851
S06-1	443,947.949	562,827.761
S06.5-1	444,031.583	562,993.305
S06-2	443,829.158	562,890.528
S07-2	444,080.933	563,094.392

Water Elevation 10/2/84 500.16'

507-1 Not Lound in pond

#### OCT 2 9 1984

#### AMERICAN ELECTRIC POWER SERVICE CORPORATION



TE:

Oct. 26, 1984

SUBJECT:

Boring Locations and 765KV Tower Elevations Clifty Creek Plant fly ash dam area.

IOM:

K. L. Hern

TO:

Kevin Miller - Columbus

As per our phone conversation of Oct. 23, 1984 please find listed below the coordinates and ground elevations of borings SS-4-3 and SS-5-3 in the south end of the fly ash dam area.

Boring No.	<u>North</u>	<u>East</u>	Elev.
SS4-3	443,365.4	562,761.5	458.0' (Grd.)
SS5-3	443,510.0	562,954.6	453.5' "

Boring SO7-1 was not located due to the float being missing.

We also ran elevations on the 765KV tower legs that you requested, please find elevations listed below. Direction is noted as facing the Jefferson sub-station.

<u>Leg</u>	Ground Elev.
L/Rear	535.06
R/Rear	534.86
L/Front	511.25
R/Front	511.61

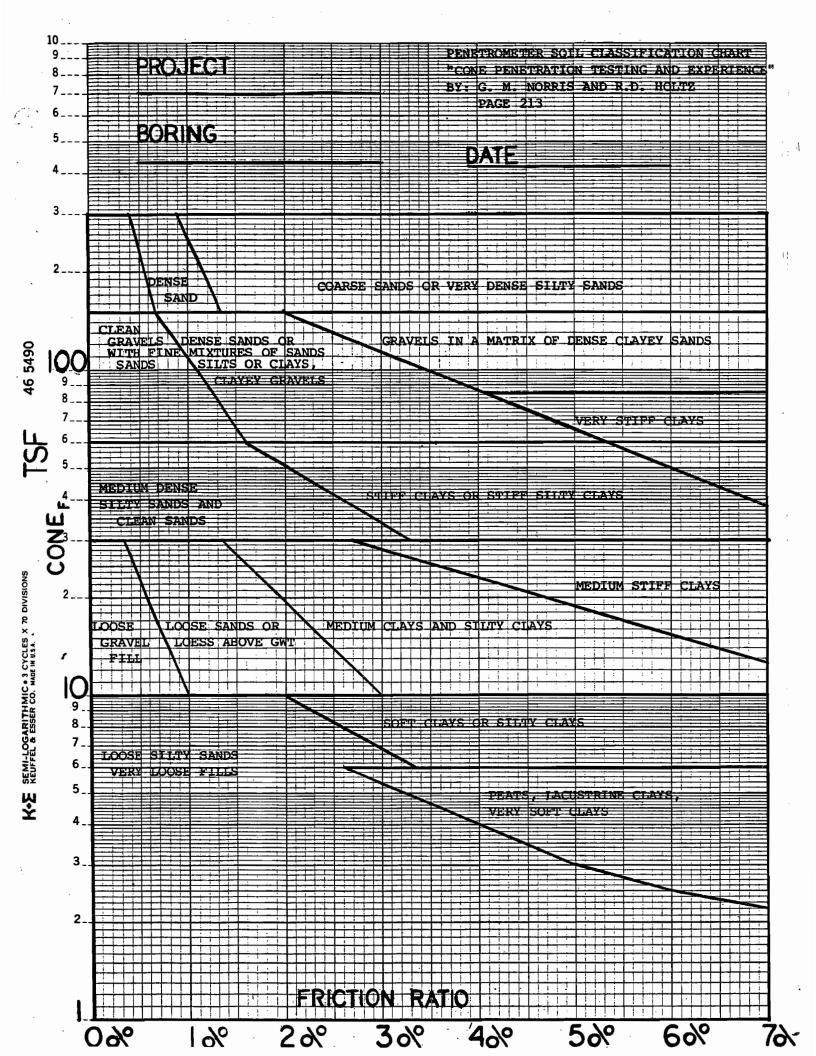
K. L. Henr, p.

K. L. Hern

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FORM CE-5 REV. 5/81

## AMERICAN ELECTRIC POWER SERVICE CORPORATION

## AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

~	TIOM C	F BORI	16:				1 Dam			CASING USED SIZE DRILLING MUD USED
										BORING BEGUN 6-27-84 BORING COMPLETED 6-27-84
	ER LEV	EL	62	. 5						GROUND ELEVATION 50552 REFERRED TO
TIME										FIELD PARTY:D
JAIL	<u>.                                    </u>									PIEGO PARTY:
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FORM CE-5 REV. 5/81

### AMERICAN ELECTRIC POWER SERVICE CORPORATION

## AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

									J.		BORING  BORING No. SSI- DATE SHEET Z of 9
ROJ	ECT										TYPE OF BORING RIG RIG RIG PRILLING MUD USED
Loc	ATION C	F Born	NG 1						٦	•	CASING USED SIZE DRILLING MUD USED
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DAT									+		FIELD PARTY:DATU
UAI									_	'	PIECO PARTY:
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₹,	FROM	TO	81	ows	/ 6"	7 2				_Ի	FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
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									•	عد	SAWdy Clay - Yellowish BR. Low DIASTICITY, Moist, Slight REACTION TO HCL
6	27.5	29.0	5	4	9	9"		ļ · ,	a -	46	DIASTICITY MOIST SLIGHT REACTION
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										<u>ا</u>	Silty clay- yellowish Br. Low
8	37.5	39.0	8	12	11	16"			B	74	21ASTICITY MOIST MODERATE
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#### AMERICAN ELECTRIC POWER SERVICE CORPORATION

## AEP CIVIL ENGINEERING LABORATORY

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. )										BORING NO. SSI-1 DATE SHEET 3 OF			
	CT								٦ .	CASING USED SIZE DRILLING MUD USED			
LOCA	TION O	F BORI	M <b>6</b> :		*					BORING BEGUN BORING COMPLETED			
WAT	ER LEV	EL		_					1	GROUND ELEVATION REFERRED TO			
TIME	!									OA			
DATE									]	FIELD PARTY:			
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SAMPLE No.	DE	APLE PTH FEET TO	PEN RE:	TANDA IETRA SISTAI .OWS /	TION	TOTAL LENGTH RECOVERY	ELEVATION		PTH N ET	SOLE TIPE, COURT, TEXTORE, CONSISTENCY, SHEPER SHIPING HOTE			
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a	us C	440	٠	,,	18	14 "			² — :	Sandy SilT - Yellowish Br. Slight PLASTICITY, moist			
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۵	47.5	49.0	11	13	11_	15"		,	3 <u>-</u>	SAME AS SAMPLE NO.			
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	52.5	5.1.0	7			15"		'	· -	Clay- Geny + BR. motiled, Low To			
		34.0		10	13	/3		;	3 <u>-</u>	med plasticity, moist			
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<u>a</u>	525	59.0	13	16	19	16"			<u> </u>	Same as Sample No. 11			
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FORM CE-5 ...

# AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY

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	NY		_					<u>.</u>	BORING NO. SSI- DATE SHEET _ 4 OF 9	
ProJE	CT								TYPE OF BORINS RIG	
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DAT	DATE								FIELD PARTY:	
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LE No.	DE	IPLE PTH FEET	PEN	TANDA IETRAT SISTAI	TION	TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN	Soit Tipe, Coch, Textone, Consistent, Charles Children Hotel	
MAPLE	1		Ι.	ows /		E E		FEET	BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.	
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13 .	42.5	64.0	4-	11	16	16"		3 –	molsT	
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					<u> </u>			7 -	Clayer SANd - BR. SATURATED,	
14	47.5	69.5	2	4	4	7"			POORly GRAded, 100 % FINE GRAIN	
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٠									Clay- GRAY. moist . med plasticity	
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16	77.5	79.0	9	10	14	16"		_ =	TO HI PINSTICITY GRAVE POORLY	
/								8 -	GRADED LIMESTONE FRAG.	
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FORM CE-5

# AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

'AP	MY						<u>.</u>			BORING NO. <u>SSI-1</u> DATE SHEET OF						
/							-			TYPE OF BORING RIG						
Loca	TION O	F Boni	H <b>G</b> :						1	CASING USED SIZE DRILLING MUD USED						
					•					BORING BEGUN BORING COMPLETED						
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DATI	<u> </u>						-		]	FIELD PARTY:						
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ect_					·		Type of BoringRig
cation	of Bo	ring;	-				Casing used Size Drilling mud used
		í — — — — —					Boring begun Boring completed Ground Elevation referred to Datum
Water Lev Time	eı						Datus
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Date							
		Sample Depth from-to (in feet)					
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A T	l m	1 4 4 2 P	8	Š =	Depth In Feet	چ	Hock Type, Color, Quality, Orilling observations, Deptimater lost, Observed fluctuations in water level, note.
ELEVATION FEET	SAMPLE	amp frc		Recovery (in feet)	ă	Graph	on drilling ease, etc.
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cation	of Bo	ring:					Casing used Size Drilling mud used Boring begun Boring completed
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Job No							
Сотралу _							Boring No. 53/-1Date Sheet 8 of Casing used Size Drilling mud used
Project _							Type of BaringRig
cation	of Bo	ring:			•		Casing used Size Drilling mud used
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Time			_				Field Party:
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중	훈	e e		53	l	8	- DESCAIPTION
ELEVATION FEET	SAMPLE	Sample Depth from-to (in feet)	900	Recovery (in feet)	Depth in feet	Graph Log	Rock Type, Color, Quality, Drilling observations, Deptember lost, Observed fluctuations in water level, not on drilling ease, etc.
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Job No.							
Company							Boring No.SSI-1 Date Sheet 9 of 9  Type of Boring Rig Rig Drilling mud used
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catio	n of B	oring:					Casing used Size Drilling mud used
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Water L	evel	ļ					Ground Elevation referred to
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<u>Date</u>		<u> </u>					Field Party:
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ELEVATION FEET	2	Sample Depth from-to (in feet)		,5 <del>=</del>		8 -	DESCRIPTION
1 E E	네 별	fe fe	900	5 5	Depth In feet		Rock Type. Color. Quality. Drilling observations, Depths
<b>1</b> 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	3	d cr	Œ	Recoverý (1n feet)	a -	Graph Log	water lost, Observed fluctuations in water level, notes on drilling ease, etc.
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FORM CE-5 REV. 5/81

JOB No.\_

## AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY

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	ER LEV			10.1		_		$\dashv$	BORING BEGUN 7-10-84 BORING COMPLETED 7-10-85 GROUND ELEVATION REFERRED TO		
IME		-	ν	RY				$\dashv$	OAT		
ATE									FIELD PARTY: ROUSH - LAMberT		
SAMPLE No.	DE IN	APLE PTH FEET TO	PEN RES	TANDAI IETRAT SISTAN	ION	TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION  SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.		
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FORM CE-5 REV. 5/81

# AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

					_				BORING NO. SSI-Z DATE	SHEET _Z_ 0# _				
/ ROJE	CT		-						TYPE OF BORING SIZE	_ RIG				
Loca	TION O	Boni	NG:						CASING USED SIZE	_ DRILLING MUD USED				
									BORING BEGUN BOR					
	ER LEV	EL		_				·	GROUND ELEVATION					
TIM									FIELS PARTY.	DA				
JATI			···						FIELD PARTY:					
WPLE No.	DE	IPLE PTH EET	PEN RES	ANDA Etrat Sistan	RO TION ICE	TOTAL ENGTH ECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION  Soil type, color, texture, consistency, sampler priving notes blows per foot on casing, depths wash water lost, observed					
SAN	FROM	TO	BL	ows /	′ 6 <sup>n</sup>	-2			PLUCTUATIONS IN WATER LEVEL, NOTE	S ON DRILLING EASE, ETC.				
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#### AMERICAN ELECTRIC POWER SERVICE CORPORATION

## AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

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4				REEK	E	A.D.			TYPE OF BORING SPT RIG B-61
		F Boni							CASING USED SIZE DRILLING MUD USED
								.	BORING BEGUN 9-/2-24 BORING COMPLETED
	ER LEV	EL		<u> </u>					GROUND ELEVATION REFERRED TO
DAT									FIELD PARTY:
UAI	<u> </u>					***			FIELD PARTY:
Š	SAN	APLE	\$	TANDA	RD	.∓≿			DESCRIPTION
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3	FROM	TO	BL	ows /	6"	. 75			FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
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									GRAVELLY SANDY CLAY- BR. MOIST 1"MAXSIZE GRAVEL CONSIST OF Lime
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#### AMERICAN ELECTRIC POWER SERVICE CORPORATION

## AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

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WAT	ER LEV	EL							GROUND ELEVATION REFERRED TO
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DAT	<u> </u>								FIELD PARTY:
2	CAN	APLE	51	ANDA	<b>R</b> D	<b>_</b> >		_	DESCRIPTION
	1	PTH		ETRA		TOTAL LENGTH RECOVERY		DEPTH	SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES
SAMPLE		FEET	RES	SISTA	4CE	PA S	ELEVATION	IN   FEET	
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ompany_							Boring No Date Sheet 3 of
reject							Type of BoringRigRig
Ition	of Bo	ring:					Casing used Size Drilling mud used
	- 1	Т					Boring begun Boring completed
<u>Water Le</u> s Time	yeı						Ground Elevation referred to Datum
Date							Field Party:
Oate							
ELEVATION FEET	SAMPLE NO.	Sample Deptifrom—to	ROD	Recovery (In feet)	Depth 1n feet	Graph Log	Hock Type, Color, Quality, Drilling observations. Dept water lost. Observed fluctuations in water level, notes on drilling ease, etc.
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iect_							Type of BoringRigRigRig
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Time							Date
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ELEVATION FEET	SAMPLE	ald all	8	500	Gepth In feet		Hock Type, Color, Quality, Drilling observations, Depth water lost, Observed fluctuations in water level, notes
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Job No							SAB-1
Company _							Boring No. Date Sheet 5 of
		<u> </u>					Type of BoringRigRig
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Water Lev	el			. <u> </u>			Ground Elevation referred to Datur
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		Sample Deptr. from-to (in feet)					
ELEVATION FEET	SAMPLE NO.	it c e		Recovery (in feet)		Log	DESCRIPTION
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Job No.					LU	5 0	P BOKING
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							Type of BoringRigRig
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ater Lev	rel						Ground Elevation referred to
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ELEVATION FEET	SAMPLE NO.	Sample Depti- from-to (in feet)	Вар	Recovery (in feet)	Depth In feet	Graph Log	DESCRIPTION  Rock Type, Color, Quality, Orilling observations, Depths water lost, Observed fluctuations in water level, notes on drilling ease, etc.
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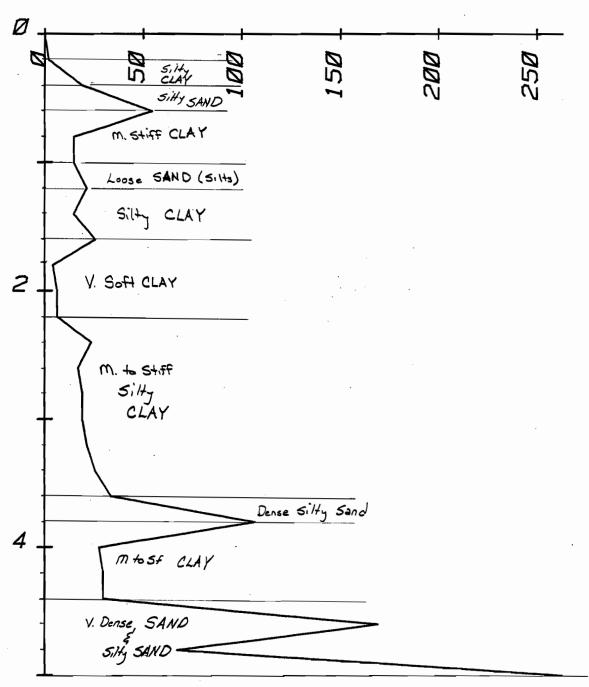
Dutch Cone Test Probe No. 551-3 Date 8-15-84 Company IKEC Job CLIFTY CREEK FAD Drillers Page / of 2 Elevation Recorder Lambert DEPTH DEPTH C or GAGE READINGS CONE SLEEVE FRICTI V ELEV. RESIST. (Ft.) (m) C + SHIGH LOW RESIST. RATIO (8) GAGE ZERO READINGS (1000N) - (1000N)  $\overline{\mathsf{c}}$ 25' SOUTH OF STAKE C+S C 1.6 0.029 0.56 19 C+S 2.4 C. 5.0 54 0.42 0.008 C+S 5.6 1.25 0.08 15 C+S 3.0 C 1.2 0.84 15 0.056 .0 C+S 2.4 1.8 21 0.28 0.013 C+S 2.2 1.2 C 0.028 0.42 15 C+S 1.8 C 2.2 0.70 25 0.028 .. C+S 3.2 1.39 0.337. C+S 2.2 0.093 0.56 6 2.0 C+S 1.2 C .4 6 0.70 0.117 C+S C 2.0 0.030 23 0.70 4 C+3 3.0 1.4 17 0.97 0.057 6 F C+S 2.8 C 1.6 19 0.70 0.037 . 8 Ī C÷S 2.6 1.6 19 0.037 0.70 3.0 C+S 2.6 1.8 0.066 21 1.39 C+S 3.8 C 2.2 0.111 25 2.78 . 4 C+S 6.2 С 3.0 33 1.39 0.042 .6 C+S 5.0 10.0 0.020. 2.09 106 .31 C+S 13.0 C 2.4 27 6.68 0.247 4.0 C+S 12.0 2.6 29 2.23 0.077 C÷S 5.8 2.6 29 1.25 0.043 4 C+S 4.4 16.0 C 334 169 0.020 6 C+S 11.0 C 5,2 1.25 0.019 67 81 C+S 8.0 0.013 25.0

FORM CEAS

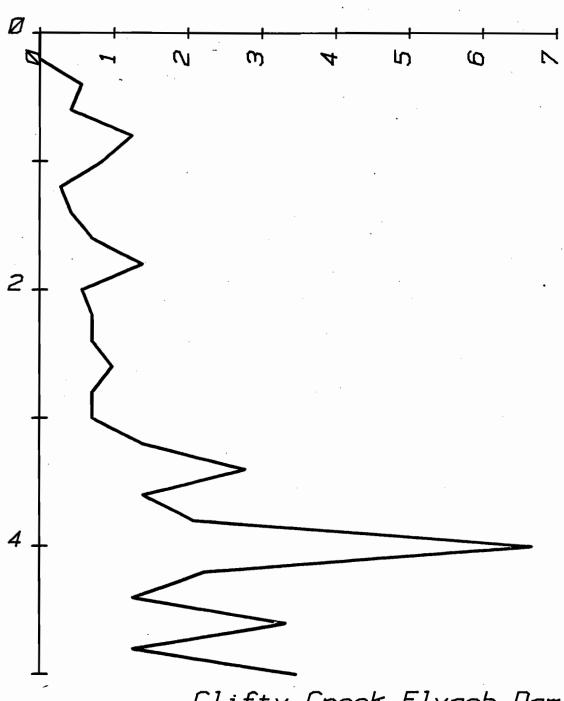
#### AMERICAN ELECTRIC POWER SERVICE CORPORATION

## AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

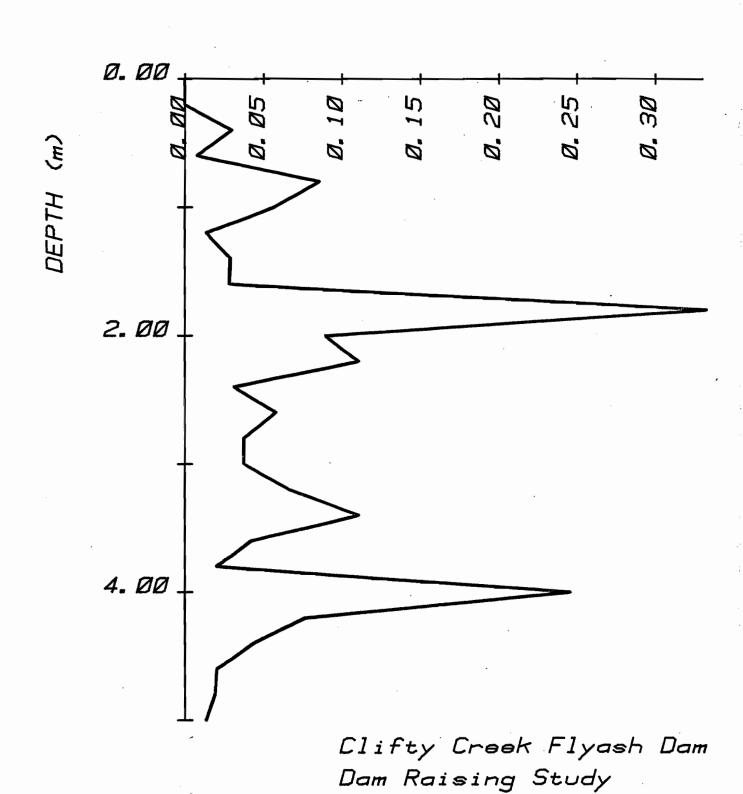
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MP	AHY								BORING No. ST-3 DATE SHEET 2 OF 2	
PROJE	CT						· · ·		TYPE OF BORING RIG  CASING USED SIZE DRILLING MUD USED	
Loc	ATION O	F BORE	H6:	_					CASING USED DRILLING MUD USED	
	\	<del>   .</del>							BORING BEGUN BORING COMPLETED	
TIM	ER LEV	/EL						$\dashv$	GROUND ELEVATION REFERRED TO	
DATI				_					FIELD PARTY: DATU	
-			_						The Parity	
SAMPLE No.	DEPT		STANDARD PENETRATION RESISTANCE BLOWS / 6"		TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DIAME AND COLDER, INAPARE, CONSISTENCY, SERVICE SHITTING HOLES		
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Clifty Creek Flyash Dam Dam Raising Study PROBE SS1-3



Clifty Creek Flyash Dam Dam Raising Study PROBE SS1-3



PROBE SS1-3

FORM CE-5 REV. 5/81

# AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY

## CIVIL ENGINEERING LABORATORY LOG OF BORING

Loc	ATION O	F BORE	H <b>G</b> :							CASING USED SIZE DRILLING MUD USED BORING BEGUN 6-28-84 BORING COMPLETED			
WAT	ER LEV	EL	-		•	2	8. S		1	GROUND ELEVATION _504.45 REFERRED TO			
TIME										DAT			
DAT	<u> </u>						10-84			FIELD PARTY: Roush - Bumg ARVER			
SAMPLE No.	DE	APLE PTH FEET TO	PEI	TANDA NETRA SISTAI LOWS /	RD TION NCE / 6 <sup>th</sup>	TOTAL LENGTH RECOVERY	ELEVATION	DEP II FE	٧	DIE 1176, COUNT, TEXTURE, CONSTRUENCY, SAMPLER ENGINES HOTEL			
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2	8.0	9.5	3	5	7	4		۰	=	TO med plasticity maist slight			
			,					•	1	Clay- Yellowish BR + GRAY BR. LOW TO med plasticity, maist, slight Rection TO HCL			
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<u></u>	18.0	19.5	6	13	1.3	12"		9	4	plasticity maist			
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FORM CE-5 REV. 5/8:

Jos No.\_

# AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

AP.	AHY			<u> </u>			•		BORING NO.SS2-1 DATE SHEET Z OF 5
-ROJ	ECT								TYPE OF BORING RIG
Loc	ATION O	F BORE	NG 1						CASING USED DRILLING MUD USED
201.					-				BORING BEGUN BORING COMPLETED
TIM	ER LEV	/26							GROUND ELEVATION REFERRED TO DATUM
DATE									FIELD PARTY:
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SAMPLE No.	DE IN	MPLE PTH FEET	PER	TANDA ETRA SISTA OWS	TION NCE	TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DIAME TO SEE ON CASIMA DEPOSIT MASH MASER LOSS CONTINUES
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1	38.0	39.5	9	12	/3	13*	<u>                                     </u>	9 —	med. plasticity, moist wy slight
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FORM CE-5

### AMERICAN ELECTRIC POWER SERVICE CORPORATION

## AEP CIVIL ENGINEERING LABORATORY

-								TYPE OF BORING RIG
	- 8						_	CASING USED SIZE DRILLING MUD USED
ATION O	F Boril	•••						BORING BEGUN BORING COMPLETED
ER LEV	EL							GROUND ELEVATION REFERRED TO
								DATO
l ·						•		FIELD PARTY:
		6:		88	<u> </u>			DESCRIPTION
-		_		KU :	125	<u> </u>	DEPTH	
		_		NCÉ	E89	ELEVATION	IN	Sair 1122, come, textone, asserter suiting water
		· .		H			FEET	BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED
FROM	то	BL	.ows /	/ 6"	œ			FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
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48.0	49.5	6	8	21	16		تـوا	Plasticity Slight Reaction TO HCL
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58.0	59.5	10	16	19	16"		_ 7	DIOIST
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	SAR DE IN I	SAMPLE DEPTH IN FEET FROM TO	SAMPLE DEPTH IN FEET RE FROM TO BL	SAMPLE DEPTH IN FEET RESISTANT BLOWS / AND HAS 7 AND HAS 7 AND HAS 7 AND HAS 10 14	SAMPLE DEPTH PENETRATION RESISTANCE BLOWS / 6"  43.0 44.5 7 6 14  48.0 49.5 6 8 21	SAMPLE DEPTH IN FEET PENETRATION RESISTANCE BLOWS / 6"  43.0 44.5 7 6 14 12"  48.0 49.5 6 8 21 16"  S.20 54.5 10 14 19 15"	SAMPLE DEPTH PENETRATION RESISTANCE BLOWS / 6"  43.0 44.5 7 6 14 12"  43.0 49.5 6 8 21 16"  SEC S4.5 10 14 19 15"	SAMPLE DEPTH PENETRATION RESISTANCE FROM TO BLOWS / 6"  43.0 44.5 7 6 14 12"  48.0 49.5 6 8 21 16"  49.0 49.5 6 8 21 16"  5.2 5.4 5 10 14 /9 /5"  5.3 6 7 7 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8

# AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

ROJE	CT									BORING NO.SS2-/ DATE SHEET 4 OF RIG
		F BORE						$\overline{}$		CASING USED SIZE DRILLING MUD USED
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	ER LEV	/EL								GROUND ELEVATION REFERRED TO
DATE										FIELD PARTY:
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Ŷ	SAI	MPLE	S	TANDA	RD	≿				DESCRIPTION
E	_	PTH	PE	NETRAT	TION	TOTAL LENGTH RECOVERY	EL EVATION	DEP		SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING HOT
7	IN	FEET	RE	SISTA	NCE		ELEVATION	FE		BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OSSERV
SAN	FROM	TO	8	LOWS /	6"	THE		' - '	• •	FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
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### AMERICAN ELECTRIC POWER SERVICE CORPORATION

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					_			_	TYPE OF BORING RIG RIG RIG RIG PRILLING MUD USED
Loc	ATION G	F BORE	NG:					-	BORING BEGUN BORING COMPLETED
WAT	ER LEV	/EL							GROUND ELEVATION REFERRED TO
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DAT	E	• ].						·	FIELD PARTY:
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7		FEET	RE	SISTA	NCE	15×8	ELEVATION	IN	
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17	83.0	84.5	10	13	15	1/2 "		. '	SATURATED, STRONG REACTION TO HEL
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19	93.0	94.5	10	14	15	16"			SITY Clay. GRAY Low Tomed.
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# Dutch Cone Test

Company	CKEC				· P	robe No. 332-3	Date_9	-15-84
Job Cl. F	Ty Cree	KFAP	<u></u>		D	rillers Rous	h + Lambe	RT.
corder_	LAMber	Τ			P	age of	Elevation	n
ELEV.	DEPTH (Ft.)	DEPTH (m)	C or C+S	GAGE R HIGH	EADINGS LOW	CONE RESIST.	SLEEVE RESIST.	FRICTION RATIO
GAGE	ZERO REAL	DINGS	·	0.0	-0.2	(1000N)	(1000N)	(8)
	.	.2	C C+S				20'SOUTH OF	ssz-2
		.4	C C+S					
		.6	C+S		7.4	79	1.11	0.014
		.8	C	·	5.4	58	2:37	.0.041
		.0	C C+S		4.8	52	2.78	0.054
		1.2	·C		6.8 9.2	73	1.67	0.023
		. 4	C+S		7.4	48	2.09	0.044
		.6		· · · · · · · · · · · · · · · · · · ·	7.2	48	1.95	0.041.
		. 8			3.6	40	1.39	0.035.
·		2.0	C+S		5.8	35	1.81	0.052
		.2	C+S C		5.0	23	2.09	0.091
	i i	.4	C+3 C		5.0 4.2 4.2	54	0.84	0.016
		.6			7.6	46	2.37	0.052
	·	.8	C÷S C		8.4	- 52	2.51	0.048
		3.0	C+S C		7.0 5.2	44	2.09	0.048
		.2	C+S C		8.2	56	2.09	0.03/
		.4	C+S C		9,0		2.09	0.030
		.6	C+S C		3.4	69	2.51	0.066
		.3 	C+S C		7.0	46	2.37	0.052
		.2	C+S C+S		7.6			
$\bigcirc$		.4	C+S					
		.6	C+S					
		.9	C+S					
		5	C		L		!	•

#### AMERICAN ELECTRIC POWER SERVICE CORPORATION

# AEP CIVIL ENGINEERING LABORATORY

)MP4										BORING NO. SSZ-3 DATE SHEET _2 OF _G
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4	27.3	47.0	- A	/7	1.5_	7.7		8	-	SiTY Clay- GRAY BR. MOIST, LOW TO Med. plasticity
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#### AMERICAN ELECTRIC POWER SERVICE CORPORATION

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	ECT								CASING USED SIZE DRILLING MUD USED
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					_			] -	Clay. GRAY, moist - Low to med.
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			_ ا	·			:	<u>`</u>	Clayey SANd - GRAY - Moist -
6	37.5	39.0	7	7	9	16"		8	properly Gended. 100% Fint Oranin
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7	ے دیں	44.0	5	7	۔	,, <i></i>		-	Silty Clay- GRAY. Low Plasticity.
-	7613	77.0		· ·	-	16"		3	mais/:
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								7	SITY SOND- GRAY - MOIST TO WET-
8	47.5	49.0	4	6	6	16"			100 % Fine Canine Fine Game hour
1								8	W ORG. MAT.
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#### AMERICAN ELECTRIC POWER SERVICE CORPORATION

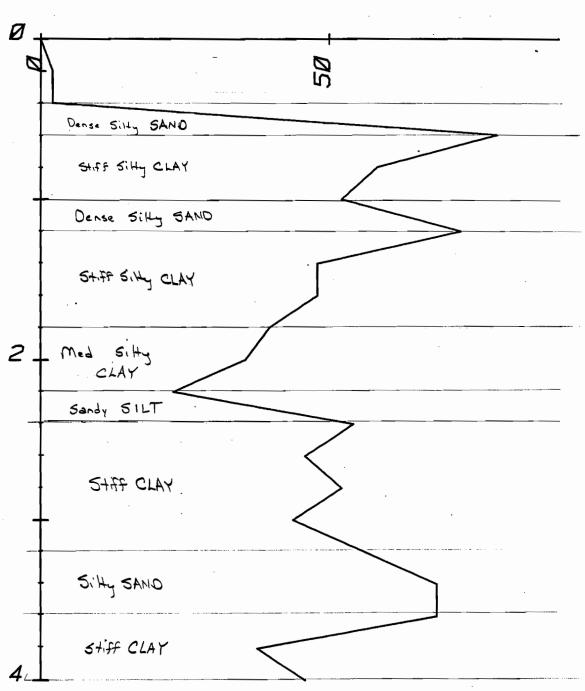
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									TYPE OF BORING RIG
									CASING USED SIZE DRILLING MUD USED
_ L∝	ATION O	F BORI	M Ø 1						BORING BEGUN BORING COMPLETED
WAT	TER LEY	/EL					_		GROUND ELEVATION REFERRED TO
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<u></u>		MPLE	_	TANDA Jetra		TOTAL LENGTH RECOVERY		DEPTH	DESCRIPTION
ָרָשׁ <u>,</u>		PTH FEET		SISTA		259	ELEVATION	IN	Soil Tire, com, Textone, consistency, samples butting notes
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<i>,</i> 9	52.0	54.0	4	9	8	16"		, -	Silty Clay - moist-GRAY - Low plasticity - w/ org. mat. (Lymite?)
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٥١	57.5	59.0	3	6	7_	75"		8	100% V-FINE GRAIN
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#### AMERICAN ELECTRIC POWER SERVICE CORPORATION

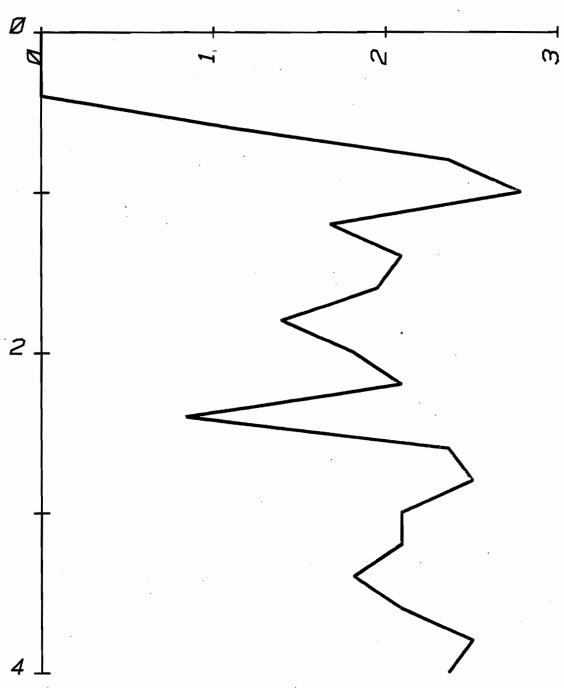
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)4	77.5	79.0	4	7	8	16"			SAME AS Sample No. 13
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<u> </u>	₹7.3	77.0	7	12	X	- X		8 -	(GRAVELLY SAND hime STAME) 1"MAX SIZE
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#### AMERICAN ELECTRIC POWER SERVICE CORPORATION

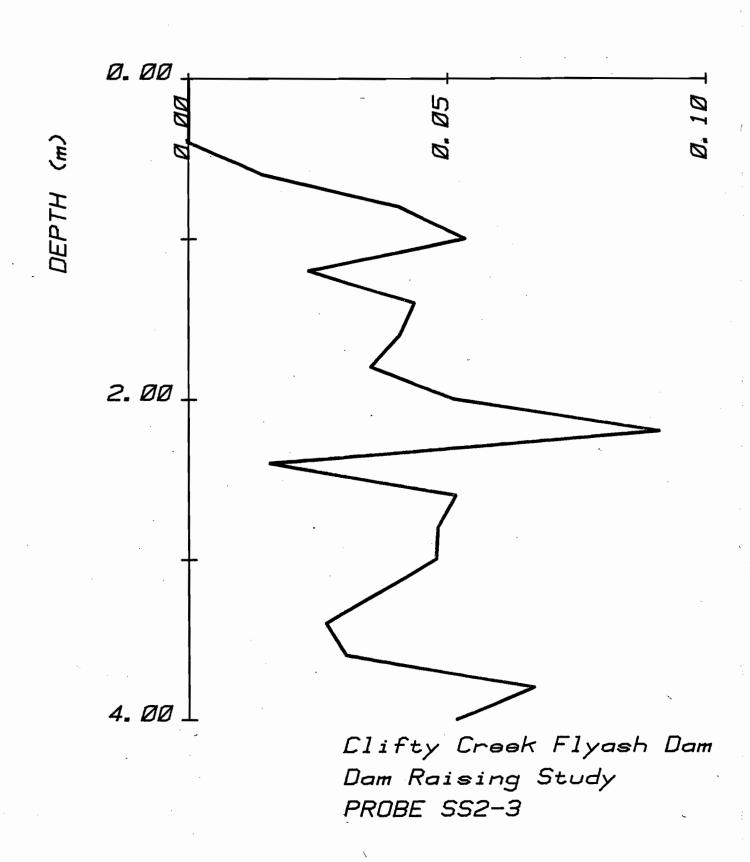
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	ER LEV	EL							GROUND ELEVATION REFERRED TO
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SAMPLE NO.	DE	APLE PTH FEET	PEN RE	TANDA IETRA SISTAI	TION NCE	TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	SOLE TIPE, COURT, TEATORE, CONSTITUTE, SAUPLER DRIVING HOTES
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Clifty Creek Flyash Dam Dam Raising Study PROBE SS2-3



Clifty Creek Flyash Dam Dam Raising Study PROBE SS2-3



# AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

MP	ANY	K	c							BORING NO.552-4 DATE 8-1-84 SHEET / OF 4			
· / PROJI	ECT C.	FT	y cr	2ce K		9 10				TYPE OF BORING SPT RIG 13-61			
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DAT			8-1-	84				·		FIELD PARTY: Roush + Lambert			
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# AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

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									CASING USED DRILLING MUD USED					
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DAT	E								FIELD PARTY:					
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#### AMERICAN ELECTRIC POWER SERVICE CORPORATION

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<u> </u>	42.5	44.0	3	4	7	16"		3 —	MOIST TO WET - TRACE OF V-FINE SAND				
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15		۔ بر ا	_		١,,	/2		=	Sity Clay. Blue GRAY SATURATED				
/ <b>-</b>	282	580	8	11		/2		8 —	Low To med plasticity . Three of				
) .		1							himr STONE FARS Sud OF Spoon				
			<del> </del>	<del>                                     </del>	+	<del>                                     </del>		9 —	- C L				
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### AMERICAN ELECTRIC POWER SERVICE CORPORATION

### AEP CIVIL ENGINEERING LABORATORY

	MY				_					BORING NO. 552-4 DATE SHEET 4 OF
IOJE	CT									TYPE OF BORING RIG
		F Boni						<del>_</del>		CASING USED SIZE DRILLING MUD USED
_OCA	TION O	F BORI	<b></b>							BORING BEGUN BORING COMPLETED
NAT	ER LEV	EL				_				GROUND ELEVATION REFERRED TO
IME										
)ATE										FIELD PARTY:
6						1			_	•
ž	_	APLE		TANDA		TOTAL LENGTH RECOVERY		DED:	- 11	DESCRIPTION
<b>w</b>		PTH		NETRA			ELEVATION	IDEP	' '	SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOT
록	1194 7	FEET	RE	SISTA	NCE			FEE	τ.	BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERY
3	FROM	TO	81	LOWS	/ 6"	. ¬₩				FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
				· .						
_				·	_		·	60	_	
	_								7	
4		<u> </u>		<del>                                     </del>	_	<del> </del>		1	_	<u> </u>
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┥				+-	<del> </del>	+		2	$\exists$	alayay Sayah Garah Garah
,		64.0	0	10		10"			$\exists$	Clayey SAND+ GRAVE !- POORIY GRADES Blue GRAY, SATURATED, LimesTone
7	44.5	97.0		70	111	10		3	ᅼ	Blue GRAY, SATURATED, LIMESTONE
		ĺ							$\exists$	FRAG.
┪	_			+	-		<u> </u>	4	ㅋ	
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### AMERICAN ELECTRIC POWER SERVICE CORPORATION

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Loc		Born			8.1		Dam		TYPE OF BORING SOT RIG 3-61  CASING USED SIZE DRILLING MUD USED  BORING BEGUN 6-26-84 BORING COMPLETED 6-26-84  GROUND ELEVATION 459.16 REFERRED TO				
TIMI	£								GROUND ELEVATION 459.16 REFERRED TO DATE  FIELD PARTY: ROUSH - LAMBERT				
SAMPLE NO.	DEI	EET	PEN RES	IETRA' SISTAI	TANCE		ELEVATION	DEPTH IN FEET	Sold its, sound, textone, consistent, american entrine notes				
								0 –	- Moved boring 5' SE OF STAKE				
							· ·	1 -					
						<u>.</u>		2 =	Clay- BR. moist, Low To med plasticit				
<u> </u>	2.5	4.0	3	4	7	10"		3 -	CL				
	-	. *		-				4 -					
_		-	<u> </u>					5 -					
سىر								6					
,	7 5	9.0	ب	6	7	10"		7 -	Sitty Clay - Yellowish Br. Low				
	713	<u>7.0</u>		6	1	10		_	Plasticity, MOIST, Slight REACTION TO HOL CL: MI				
					,			9-	service to				
	Ţ,							-					
					<u> </u>		·	2 -					
3	12.5	14.0	4	10	12	8		3	Clay- BR. moisT, Low To med plastice				
	-							4 -	C L				
					ļ			5 —					
						İ		6 -					
							,	7 -	SiT - BR. SlighT PLASTICITY, MOIST				
	17.5	19.0	3	7	10	12"	, , , , , ,	8 -	ML				
								9 —					
	· _							20-					
$\dashv$									ENGINEER				

#### AMERICAN ELECTRIC POWER SERVICE CORPORATION

_						•			BORING NO.SS3-1 DATE SHEET Z_ OF			
ROJI	ECT								TYPE OF BORING RIG			
1 00	ATION O	F BORI	Ne:						CASING USED SIZE DRILLING MUD USED			
					٠.				BORING BEGUN BORING COMPLETED			
WAT	ER LEV	/EL							GROUND ELEVATION REFERRED TO			
TIM	E								DAT			
DAT	E		•			-			FIELD PARTY:			
Š		MPLE	1	TANDA		J. E &		DEPTH	DESCRIPTION			
w		PTH		ETRA			ELEVATION	IN	Soit Tire, town, Textone, consistent, samples whiting hotes			
췿	IN	FEET	HE	SISTA	NCE			FEET	BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED			
SAM	FROM	I TO	81	ows .	/ 6"	1, 75			FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.			
V.												
		<u> </u>	-	<del>  .</del>	<del> </del>	+ -		20-				
					·	<u>'</u>		] [				
								-	· · · · · · · · · · · · · · · · · · ·			
				-	-	-		2 –	SITY CLAY- GRAY + Be MOTTLE d.			
5	22.5	24,0	7	9	13	14"		3.	how To med DIASTICITY MOIST			
						1.		-	STRONG REACTION TO HOL			
				+	<del> </del>			4 -				
					Ī		1.1	5 -				
`,									- Original ground			
, i-	ļ	<u> </u>	<u> </u>	<del> </del>	<u> </u>			6 -	3 . 3			
					ļ			7 -				
,			نس،	۱,۵				`-	Clayer Sand - Reddish BR. poorly Graded, 100% Fine Grain, moist			
ے	27.5	29.0	13	13	14_	112		8 –	GRAded, 100 % TINE GRAIN, MOIST			
		ļ		_	<u> </u>			9 -	Sc			
							1	30-				
	<del> </del>	<u> </u>	-	<del>                                     </del>	<del>                                     </del>			ı=				
								-				
		<u> </u>				-		1 2 -	SITY Clay- BR. moist. LowTo med.			
7	32.5	34.0	3	4	<u>_</u>	8		3 -	PINSTICITY			
								=	CL			
				<b>†</b>		1		4 -				
	-				ļ			5 -				
								]				
								] " =				
				+		+		7 -	Sandy sit - BR. Slight Plasticity			
8	37.5	39.0	5	7	8	12"			moist			
1									ML			
		<del> </del>			-			9 –				
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	1	į	i	!	1			i	ENGINEER			

### AMERICAN ELECTRIC POWER SERVICE CORPORATION

### AEP CIVIL ENGINEERING LABORATORY

ROJECT	_						<del>_</del>	TYPE OF BORING RIG CASING USED SIZE DRILLING MUD USED				
LOCATION C	r BORI	## :		*	.*			BORING BEGUN BORING COMPLETED				
WATER LE	YEL							GROUND ELEVATION REFERRED TO				
TIME								DAT				
DATE								FIELD PARTY:				
-   -	MPLE PTH	_	TANDA NETRA	-	STH VERY	ELEVATION	DEPTH	DESCRIPTION Soil Type, colon, Texture, Consistency, Sampler Driving Not				
3	FEET	1 .	SISTA		TOTAL LENGTH RECOVER	ELEAN! WA	FEET					
5 FROM	TO	81	LOWS	/ 6"  -	- 22			FLUCTUATIONS M WATER LEVEL, NOTES ON DRILLING EASE, ETC.				
							40-	The second secon				
							' <b>`</b>					
	1			<del> </del>	<b>_</b>		1					
							-					
	<del>                                     </del>			†			2 -	SANDY SIIT- BR. MOIST, Slight				
9 42.5	44.0	7	q	13	15"		_ =	PLASTICITY, STRONG REACTION TO HEL				
							] ] =					
	<u> </u>	,					4 -	ML				
							_					
				-	+-		5 -					
					1		=					
<del>)  </del>	<u> </u>	1		<u> </u>	<del>                                     </del>	,	6 7					
	,			,			. Œ	Not found in 4-1				
							1 13	SAND - LT BR. MOIST POORly GRAded				
0 47.5	49.0	11	12	12	13"		a _	100 % Fine GRAIN STRONG REACTION				
				1	ł			TO HOL				
		<del> </del>	+		<u> </u>		9 –	SP				
				-			- را					
					<u> </u>		<u>-</u> ه کا					
		•	·			,	] , Ξ					
						,	] 'Ξ					
			<del> </del>		ļ		2 -					
	ه ورسا	١	1				-	Clayey SAND - BR MOIST STRONG				
1 52.5	34.0	11-	13	20	15"		3 —	Reaction TO NCL 100% FINE GRAIN				
					ŀ		=					
				<u> </u>	1 .		4 -					
	<u></u>			<u> </u>			] , =					
							] ] =					
_	<u> </u>	ļ	<del> </del>	<u> </u>	ļ		6 –					
							-	Top. SiITY Clay. BR. moist Low				
	<del> </del>		+	-	<del>                                     </del>		7 -	PIASTICITY, Slight Reaction TO HCL				
2 57.5	590	ی	15	2	76"		=	<u> </u>				
N VIIG	<u> </u>	<u> </u>	<del></del>	<del>                                     </del>	<del>                                     </del>		8 =	SRI WATER				
<u> </u>		<u> </u>					] _ =					
							] ] =	BOTTOM - SAND - BR. POORLY GRADED				
	ļ	<u> </u>	<b></b>	ļ			60-	STRONG REACTION TO NCL				
								5 P				
<del></del>	<del>  _     _     _</del>	<del> </del>	+		<del> </del>		{					
								Enginesa				

#### AMERICAN ELECTRIC POWER SERVICE CORPORATION

								_	BORING NO. <u>\$53-1</u> DATE SHEET <u>4</u> or <u>5</u>
ROJE	ECT						_		TYPE OF BORING RIG RIG CASING USED SIZE DRILLING MUD USED
		r Bori					· · · · · · · · · · · · · · · · · · ·		CASING USED SIZE DRILLING MUD USED
					•				BORING BEGUN BORING COMPLETED
	ER LEV	/EL					<del>.</del>		GROUND ELEVATION REFERRED TO
TIM									FIELD PARTY:
-						··· <u>·</u> ···			FIELD PARTY:
SAMPLE NO.	DE	APLE PTH FEET	PE	TANDA NETRA SISTA	RD TION NCE	FOTAL ENGTH ECOVERY	ELEVATION	DEPTH IN FEET	SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES
3	FROM	TO	8	LOWS	/ 6"	7. 7.5			FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
		L				<u> </u>		60-	
	-	1						1	
				+	+	-		-	· · · · · · · · · · · · · · · · · · ·
						1		-	
				-	ļ <u> </u>	+		2 -	TOP
10	, , ,		١,	8	.,	16"		-	Sandy SilT - BR. WET, Slight
12	62.5	64.0	<u> </u>	18	12	1/6	_	3 –	Berrom ML
								-	SAND BR. PORLY GRAded, 100 %
	<del> </del>	<del>                                     </del>	<del>                                     </del>	+	+	+		4 -	Fine GRAIN STRONG REACTION TO
								_ 7	NeL SP
				1				5-	-
1		ł		1				-	
			<u> </u>					6 -	
						İ			
						1		] ']	Silty Sand - Br moderate Reaction
14	67.5	69.0	6	12	13	16"			To Web, WeT, POORLY GRAded
		ŀ							- / /
					ļ			9_	
		<del> </del> -		+ -	+ -		-	70-	<u> </u>
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								-	
				+	+	+		2 –	Sound Conv. Saturned Congly
٧ ا	72 5	74.0	6	7	17	1/2.0		_ 7	GRAded, 100 % Fine GRAIN, STRONG
	12.3	77.0		1		1		3 -	Reaction to HCL
		ļ							- READ TO THE STATE OF THE STAT
						1		4-	SP
			<u></u>						
							-	] 5 -	
						ļ		6 -	
					ļ			7 -	
				_				-	Same as Sample No. 15
6	77.5	79.0	13	20	17	16"		8 -	
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		i							EHOINEER

### AMERICAN ELECTRIC POWER SERVICE CORPORATION

_	_							<u>.                                    </u>	BORING NO. \$53-1 DATE SHEET 5 OF 5
,	_			-					TYPE OF BORING RIG
		r Bori					<del></del>	$\overline{}$	CASING USED SIZE DRILLING MUD USED
	I ION U	r goni							BORING BEGUN BORING COMPLETED
WAT	ER LEV	EL				<u>.</u>	<del>-</del>		GROUND ELEVATION REFERRED TO
TIME	!								DATU
DATE					· ·				FIELD PARTY:
Š				743104			T		DESCRIPTION
	• • • • • • • • • • • • • • • • • • • •	APLE	_	TANDA Netra	_	1758	ELEVATION	DEPTH	DESCRIPTION
PLE		PTH Feet	1	SISTA		<b>4</b> 95	ELEVATION	IN	SOIL TIPE, COLDR, TEXTURE, CONSISTENCY, SAMPLER DRIVING ROTES
3			1					FEET	BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED
8	FROM	TO	8	LOWS	6"	- 62			FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
	•					1			
						<u> </u>		80-	
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				<u> </u>	ļ			l 1	
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_									SAND - GRAY. POORly GRADED
1	<i>82.</i> 5	84.0	10	14	14	'y"	<u>-</u>	3	100 % Five GRAIN STRONG REALTION
						·			TO HEL. SATURATED
					ļ	<b>-</b>		4 -	SPSP_
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				1	-			5 _	·
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$\perp$				ļ		<u> </u>		6	
				<b></b>	_	ļ		7 _	
			١		١	7 "		=	SITY SAWD - GRAY POORLY GRAded
1	87.5	89.0	$\perp \perp \perp$	14	15	<del>  7  </del>		8 —	moist strong Reaction to HCL
								=	5 M
_					<del> </del>	<u> </u>		9	
								=	
$\dashv$			<del> </del>		ļ <u> </u>	<del></del>	ļ.,	90-	<u>.:_</u>
$\dashv$			<del> </del>	-	1			1 —	· · · · · · · · · · · · · · · · · · ·
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_			<del> </del>	<del></del>	<u> </u>	1		2	5
	ند دم	<b>.</b>	۱.,		,	\ <u>`</u>			Same AS Sample No. 18
4	42,5	94.0	11_	12	16	<u> </u>		3 —	
				1		Ì		_	· <del></del>
_			┼	<del> </del>	<del> </del>	<del> </del>		4 -	
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				-	<del>                                     </del>	<del> </del>		5 —	<u> </u>
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				+	<del> </del>			6 —	
_				+		<del> </del>		7	STUDIO CARROLLE AL TOTAL
,	ا ء د ۵	99.0	13	13	1,,	8"		]	SITY Clay - GRAY, Moist Slight Reaction
(0	7/13	77.0	13	13	16	-		8 –	TO HEL Slight REACTION TO HEL W/
) l								]	Fine GRAIN SAND Lens
<del>-  </del>				-		<del> </del>		9.—	
								7	
$\dashv$								100 <del>-</del>	\$70.000 \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \
									STopped boring 99.0 WATER 59.6
$\dashv$						<del>                                     </del>			
				!					ENGINEER

#### Dutch Cone Test

مؤعد والمناسد الأرباع والمنطاقية الماميط المناط المامية والأناج الأساء والمامية والمتاسطة والمتا

Company IKEC Probe No. 553-3 Date 8-7-84 Job CLIFTY CREEK FAD Drillers MACKWIGHT . LAMbers Page / of 6 Elevation Higher - 468,09 order Lambert GAGE READINGS DEPTH DEPTH C or CONE SLEEVE FRICTION ELEV. (Ft.) (m) C + SHIGH LOW RESIST. RESIST. RATIO ], GAGE ZERO READINGS (%) (1000N) (1000N) 0.0 - 0.2  $\overline{\mathsf{c}}$ C+S C 7.8 84 0 C+S C. 14.0 148 C+S C 5:29 100 0.053 C+S 17.0 C 7.0 75 6.96 0.093 .0 C+S 17.0 C 4.2 3.20 46 0.070 C+S 8.8 2.8 3.20 31 0.103 C+S 7.4 3.4 1.39 38 0.037 6 C+S 5.4 C 3.2 1.81 0.052. 35 C+S 5.8 C 3.6 1.53 40 0.038 C+S 5.8 C 4.0 0.84 44 0.019 C+S 5,2 C z. 2 25 0.044 1.11 4 C+3 3.8 C 2.6 29 0.56 0.019 C+S 3.4 C 2.2 25 0.70 0.028 C÷S 8 3.2 1,8 21 0.70 0.033 3.0 C+S 2.8 C 2,0 0.70 23 0.030 C+S 3.0 C 2.4 0.84 27 0.030 C+S 3.6 30 33 0.42 0.013 . 6 C+S 3.6 C lile 0.56 19 0.031 C+S 2,4 C 3.0 33 0.28 0.008 C+S 3.4 2.8 0.027 31 0.84 C÷S 4.0 2,6 29 0.024 0.70 C+S 3.6 C 3,2 40 0.28 0.007 C+S 4.0 2.0 0.56 0.024 23 C+S 2.0

معاصمها والشاسم الباري والمستويد فالمناف السام فالمتابات المتابات والمتابات المتابات المتابات

#### Dutch Cone Test

				•	: 1			
Job		^			Dr	illers		·
lorder		·			Pa	ge_2 of(	Elevati	on
ELEV.	DEPTH (Ft.)	DEPTH (m)	C or C+S	GAGE R HIGH	EADINGS LOW	CONE RESIST.	SLEEVE RESIST.	FRICTION RATIO
GAGE 2	ZERO REAL	DINGS	i	0.0	-0,2	(1000N) -	(1000N)	(8)
: 1			С		1,6		- 00	015
	•	. 2	_	<u> </u>	210	19	0.28	0.015
		. 4	C+S	-	3.2	35	1.95	0.056
	•		C		7.6	· · 81·		0.005
		.6	C+S C	20.0	8.2	0/	0.42	0.003
`		. 8		20.0		211	0	0
			C	12.0		127	4.18	0.033
,		<b>6.</b> 0	C+S C	18.0	9,2			
		.2	C+S	16.0	7,10	98	4,73	0.048
		. 4	C+S		9.6	86	1.11	0.013
		• • •	C		9.0			
·	·	.6			9.8	96	0.56	0.008
		. 8	C+S	13.0	9,2	98 106	2.64	0.027
			С	10.0		10626	2.78	0.026
		7.0	C+S C	14.0				<del></del>
<u> </u>		.2	C+S	12.0	9.0	96 88	2.09	0.022
			C		8.2	88 92	1.95	0.022
-	·	.41	C+3 C	11.0	8.6			
		.6	C+S	13.0	37.6	92159	3-06	0.033
	Ì		C+C	15.0		159200	4.18	0.026
		.8	C÷S C	21.0				· · · · · · · · · · · · · · · · · · ·
		.0	C+S	22.0		200,106	2.09	0.010
` ·		.2	C+S	10.0	<del></del>	106 88	4.18	0.039
		• • •	С	16.0	8.2			
		.4	C+5	12.0		88,06	2.64	0.030
	Í	.6	C+S	15.0		106	3.48	0.033
		Ĺ	С	12.0	<u> </u>	127	3.48	0.027
		.3	C+S	17.0				
		.0	C+S	15.0		18	2.78	0.024
			_ C	· .	9.2	98	4.04	0.041
		.2	C+S C	15.0		10	, , - ,	0 //
		.4	C+S					
		.6	С					
		.01	C+S C					
		.8	C+S					

# AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY

Jo e	No						L	OG OF	BORING				
710	AHY								BORING NO. SS3.3 DATE SHEET 3 OF 6				
401	ECT								TYPE OF BORING RIG				
Loc	ATION O	F BORE	N6:						CASING USED SIZE DRILLING MUD USED				
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AMERICAN ELECTRIC POSER CIVIL ENGLISHED DESCRIPTION

#### Dutch Cone Test

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1		.6	C+S	11.0	6.7	73	2.92	0.040
			С		4.2	11	2.37	0.052
<b>}</b>		.8	C+S		7.6	46	2.5/	0.000
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		.2	C+S	12.0		90	2.51	0.028
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### - AMERICAN ELECTRIC POWER SERVICE CORPORATION

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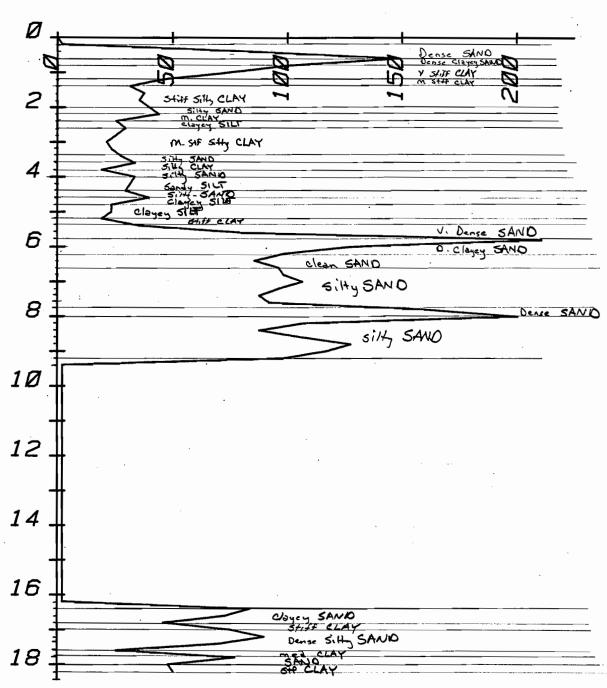
#### AMERICAN ELECTRIC POWER SERVICE CORPORATION

				,				٠.		Borine No. 53.3 Date Sheet _ S of _ C				
- )								_		TYPE OF BORING RIG				
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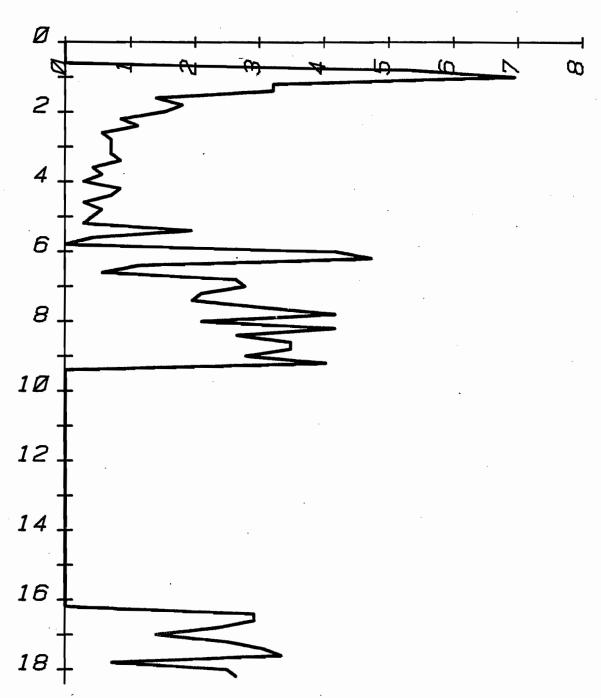
### AEP CIVIL ENGINEERING LABORATORY

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·ı	-ROJE	CT									TYPE OF BORING RIG				
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	34/5-	ER LEV	<u> 1</u>			·					BORING BEGUN BORING COMPLETED				
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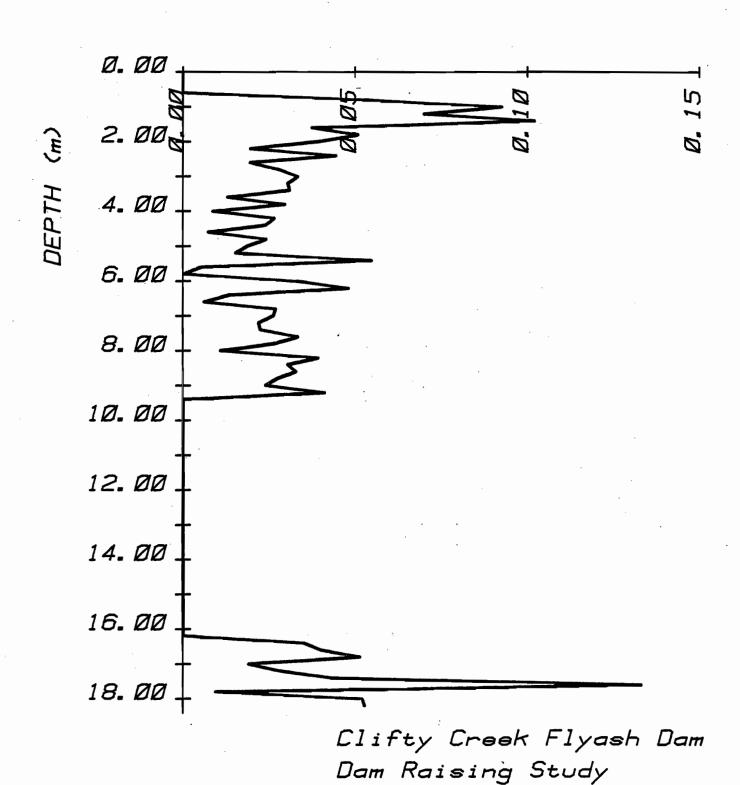


Clifty Creek Flyash Dam Dam Raising Study PROBE 553-3





Clifty Creek Flyash Dam Dam Raising Study PROBE SS3-3



PROBE SS3-3

# AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

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P	LHY	_K_E	<u>C</u>	000 10	<u> </u>	A D			BORING NO. SS3-4 DATE \$-1-84 SHEET 1 OF 7  TYPE OF BORING SPT-CORE RIG B-61
		F BOR			·- ·	<i></i>		_	CASING USED SIZE DRILLING MUD USED
				_					BORING BEGUN 8-1-84 BORING COMPLETED
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#### AMERICAN ELECTRIC POWER SERVICE CORPORATION

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		F BORI							CASING USED SIZE DRILLING MUD USED  BORING BEGUN BORING COMPLETED			
WAT	TER LEV	/EL					· · · · · · · · · · · · · · · · · · ·		GROUND ELEVATION REFERRED TO DATE			
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SAMPLE No.	DEPTH		STANDARD PENETRATION RESISTANCE BLOWS / 6"			TOT AL LENGTH RECOVERY	ELEVATION	OEPT IN FEE1	SOLE TIPE, COLDMI, TEATONE, CONSTITUTION, SAMPLER SHIVING HOLES			
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### AMERICAN ELECTRIC POWER SERVICE CORPORATION

CASING USED SIZE DRILLING MUD USED  BORING BEGUN BORING COMPLETED  GROUND ELEVATION REFERRED TO  FIELD PARTY:  DESCRIPTION  DEPTH SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING	~									BORING NO. <u>SS3-4</u> DATE SHEET <u>3</u> OF <u>7</u>			
BORNES BEGUN BORNES CONFILETED GROUND ELEVATION GROUND ELEVATION MATERIELY ELEVEL FINE ATTE TO THE COLOR TEXTURE, COLOR, TEXTU	ROJE	ECT								TYPE OF BORING RIG			
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SAMPLE STANDARD DEPTH RESISTANCE ROOMS / 8" PROM TO BLOWS / 8" PROM TO	W		. <del></del>			•		•					
SAMPLE STANDARD DEPTH WITH PRET RESISTANCE BLOWS / 8" PLOWS FRE FOOT OR CASHING, DEPTHE WATER LOST, OF FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE.  9 93.0 44.5 4 8 11 16" PLANTICLE TO THE WATER LEVEL, NOTES ON DRILLING EASE.  9 93.0 44.5 4 8 11 16" PLANTICLE TO THE WATER LEVEL OF THE WATE			VEL						$\dashv$				
SAMPLE STANDARD DEPTH PENETRATION PRESISTANCE BLOWS / 6" DEPTH RESISTANCE BLOWS / 6" D	DATE												
DEPTH RESISTANCE RESISTANCE SLOWS / 6"  STORY TO SILVATION FEET FROM TO SILVATION FEET FROM TO SILVATION FEET FROM TO SILVATION FEET FROM TO SILVATION FEET FROM TO SILVATION FEET FROM TO SILVATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, SILVATION FEET FROM TO SILVATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, SILVATION FEET FROM TO SILVATION													
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3 43.0 44.5 4 8 11 16"  4 Clay - Blue Gray meist, Low to med. Plasticity  5 Same as Sample No. 9  5 Same as Sample No. 9  5 Same as Sample No. 9  5 Same as Sample No. 9  5 Same as Sample No. 9  5 Same as Sample No. 9  5 Same as Sample No. 9  5 Same as Sample No. 9  5 Same as Sample No. 9  5 Same as Sample No. 9	S				1		-						
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### AMERICAN ELECTRIC POWER SERVICE CORPORATION

MP	ANY									BORING NO. 553-4 DATE SHEET 4 OF 7
POJE	CT									TYPE OF BORING RIG
Loc	ATION O	F Boni	NG 1							CASING USED SIZE DRILLING MUD USED
										BORING BEGUN BORING COMPLETED
	WATER LEVEL TIME									GROUND ELEVATION REFERRED TO
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### AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

Job No. 🗀										
Company_						Boring NoSS3-4 Date Sheet 5 of Type of Boring Core Rig B-6/ Casing used Size Drilling mud used				
Priect L	J. F.I	y Creek	< FA	D			Type of Boring Come Rig 23-61			
cation	of Bo	ring:					Casing used Size Drilling mud used			
Water Lev							Boring begun 8-14-84 Boring completed 8-14-8 /			
Time	/61					_	Datu			
Date						Ground Elevation referred to Datu Field Party: Roush - Lambers F				
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ELEVATION	SAMPLE NO.	et e		Recovery (in feet)	5 = 1	Graph Log	Back Tues Color Culting Colling Shares			
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### AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

Job No.					E.O.	•	ss3-t
ompany		· -· · ·					Boring No. Date Sheet 6 of 7
;							Type of BoringRigRigRig
catio	n of Bo	ring:					Casing used Size Drilling mud used
Vater Le	vel						Boring begun Boring completed Ground Elevation referred to Data
Time							Date
Tate		<u>.</u>					Field Party:
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ELEVATION FEET	SAMPLE NO.	Sample Deptr from-to (In feet)	800	Recovery (in feet)	Depth 1n feet	Graph Log	DESCRIPTION  Rock Type, Color, Quality, Drilling observations, Depthswater lost, Observed fluctuations in water level, notes on drilling ease, etc.
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### AEP CIVIL ENGINEERING LABORATORY

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-							Type of Boring Rig  Casing used Size Drilling mud used
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		1					Boring begun Boring completed Ground Elevation referred to
Water Le	/ei				_		Ground Elevation referred to
Time Date				-			Field Party:
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		Sample Depti from-to (In feet)					
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111111111111111111111111111111111111111	SAMPLE	S - S	] _	# S		Gra	on drilling ease, etc.
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	DEPTH	DEPTH	C or	GAGE B	EADINGS	CONE	SLEEVE	FRICTION							
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MICRICAN ELECTRIC TOWER SEVEL ENGINEERING INDURENCES.

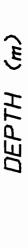
_Company		·		Probe No. SS3-5 Date									
lob					Drillers								
order								Elevation					
ELEV.	DEPTH (Ft.)	DEPTH (m)	C or C+S	CAGE READING		CONE RESIST.		SLEEVE RESIST.	FRICTION RATIO				
GAGE	ZERO REAL	DINGS	•	0.0	-0.2	(10	00N)	(1000N)	(%)				
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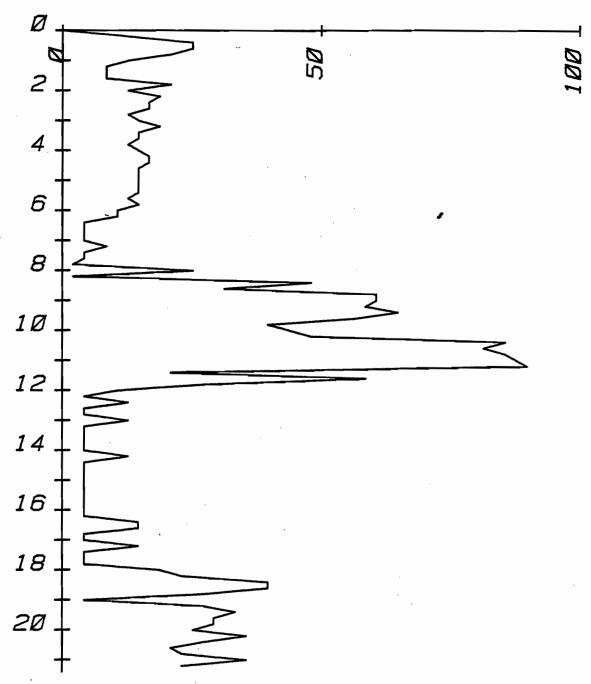
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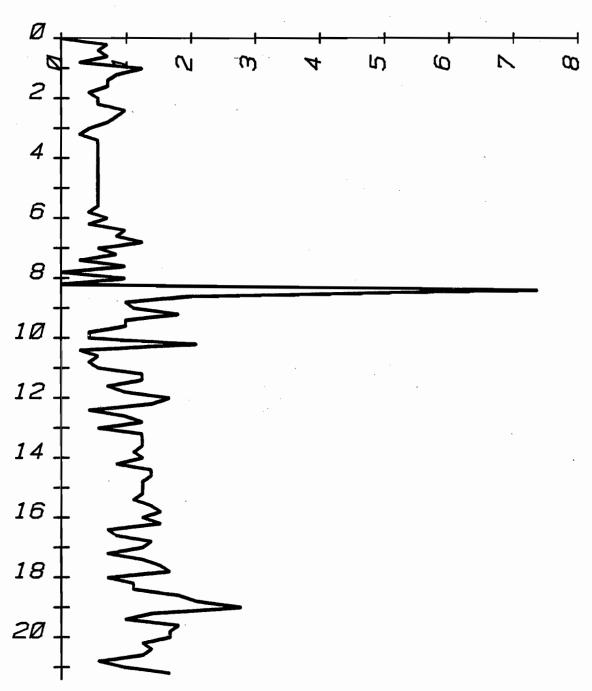
AMERICAN LEECIKIC POWER CLASE ENGLISHEDING ENDORSCORE

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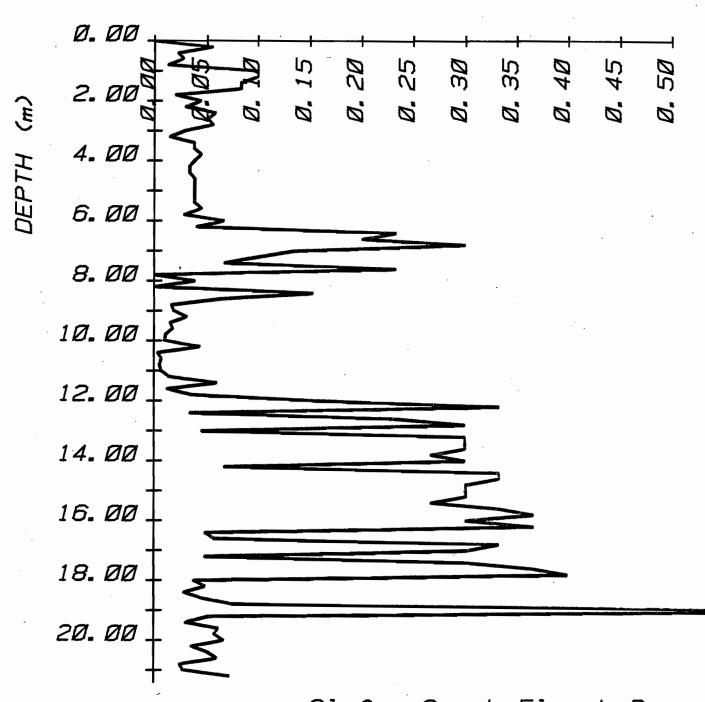




Clifty Creek Flyash Dam Dam Raising Study PROBE SS3-5



Clifty Creek Flyash Dam Dam Raising Study PROBE SS3-5



Clifty Creek Flyash Dam Dam Raising Study PROBE SS3-5

FORM CE'S REV. 5/81

	No								Barrie No ST-1 Days 7-25-9/1 Sugar 1 or 9					
-}MP ~And	ANY	11.57	-	Week	رے د	v 19.	Sh Dan	_	BORING NO. SI-1 DATE 7-25-84 SHEET 1 ( TYPE OF BORING SPT CORE RIG B-61					
	ATION O			7000 F		7		<del>-</del>	CASING USED SIZE DRILLING MUD USED					
Lac	ATRON U	P BUN							Borine Begun 7-25-84 Boring Completed					
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FORM CE'S REV. 5/6!

### AMERICAN ELECTRIC POWER SERVICE CORPORATION

## AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

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FORM CE'S

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FORM CE-5

### AMERICAN ELECTRIC POWER SERVICE CORPORATION

## AEP CIVIL ENGINEERING LABORATORY

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## AEP CIVIL ENGINEERING LABORATORY

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							Type of BoringRigRigRig			
cation	of Bo	ring:					Boring begun Boring completed			
Vater Lev	vel						Ground Elevation referred to			
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Date							Field Party:			
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z	\$ .	Sample Depth from-to (in feet)	į .	>		<u>-</u>	DESCRIPTION			
ELEVATION FEET	W W	e - t - 0		Recovery (in feet)	5 5 4	Graph Log	Rock Type, Color, Quality, Drilling observations, Depth			
EVAT: FEET	SAMPLE	1 g u	8		Depth In feet	ğ	water lost. Coserved fluctuations in water level, notes			
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4.3		84.9	ا به برت	}		1	Shale Gray			
		94.9	56%	70.0	- 85					
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## . AEP CIVIL ENGINEERING LABORATORY

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отралу_							Boring No. State Sheet of Sheet of Casing used Size Drilling mud used
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ation	of Bo	ring:					Casing used Size Driffing mud used
		<del></del>	.—				Boring begun Boring completed
Water Lev Time	vei						Ground Elevation Porting completed Datum
Date							Field Party:
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ELEVATION FEET	SAMPLE No.	Sample Depth from-to (in feet)	Rad	Recovery (in feet)	Deptn 10 feet	Graph Log	DESCRIPTION  Rock Type, Color, Quality, Drilling observations, Dept. water lost, Observed fluctuations in water level, notes on drilling ease, etc.
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# AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

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mpany_							Boring No. 521 DateSheet 7 of 8
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ation	of Bo	ring:					Casing used Size Drilling mud used
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ater Lev	ei						Boring begun Boring completed Ground Elevation referred to Datur
Time ite							Field Party:
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		Sample Depth from-to (in feet)					
ELEVATION FEET	8.	et te	1	Recovery (in feet)	بد ۽ ا	Log	DESCRIPTION
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### AEP CIVIL ENGINEERING LABORATORY

Job No							
Company							Boring No. SI-1 Date Sheet 8 of
							Type of BoringRig Casing used Size Drilling mud used
cation	of Bo	ring:					Casing used Size Drilling mud used
							Boring begun Boring completed Ground Elevation referred to Detur
Water Lev	el						Ground Elevation referred to
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Date							Treat arty.
		Sample Depth from-to (in feet)					PECCUTATION
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# AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY

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				·	· .				BORING BEGUN 7-11-84 BORING COMPLETED 7-11-84			
	R LEV	_	528						GROUND ELEVATION SOLO. REFERRED TO			
ATE			7-11-8	3 4					FIELD PARTY: ROUSH-LAMBERT			
		·							PIELD PARTY: KOUSH - LIMM DER I			
SAMPLE NO.	SAM DEF IN F	TH	PEN RES	TANDA JETRAT SISTAL	TION	TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION  SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOT BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERV FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.			
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Loc	ATION O	F BORIS	18:						CASING USED SIZE DRILLING MUD USED BORING BEGUN BORING COMPLETED
WAT	ER LEV	EL							GROUND ELEVATION REFERRED TO
TIM	2								
DAT	£								FIELD PARTY:
SAMPLE No.	DE	APLE PTH FEET	PEN	TANDA IETRA SISTAI	TION	TOTAL LENGTH RECOVERY	ELEVATION	DEPT IN FEE	SOIL TIPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING RO
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								<del>_</del> ,	CASING USED SIZE DRILLING MUD USED
Loc	ATION O	F Boni	NG: .				· . ·		BORING BEGUN BORING COMPLETED
WAT	ER LEV	EL .							GROUND ELEVATION REFERRED TO
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MPLE		PTH FEET	RE	IETRAT SISTAM .OWS /	ICE	TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	SOIL TIPE, COLDA, TEXTURE, CONSISTENCY, SAMPLEN DRIVING HOTES
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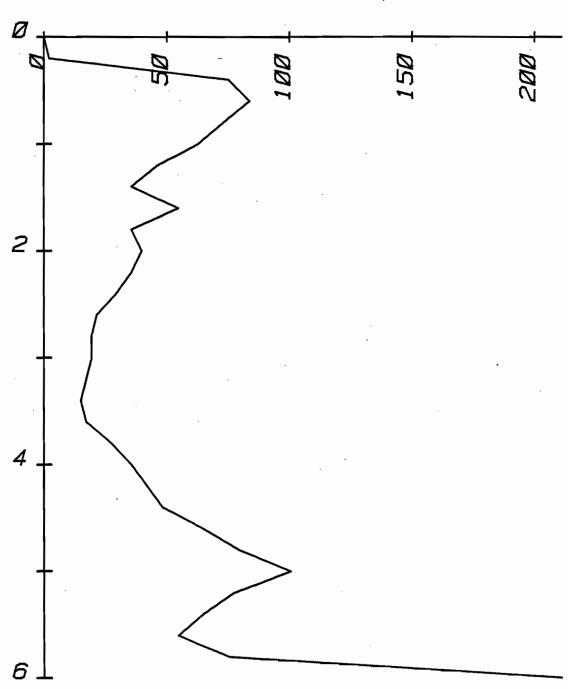
		<u></u>							BORING NO. SS4-1 DATE SHEET 4 OF 5			
									CASING USED SIZE DRILLING MUD USED			
Loc	ATION O	F BORI	N <b>6</b> :						BORING BEGUN BORING COMPLETED			
WAT	ER LEV	EL .					<del>-</del>		GROUND ELEVATION REFERRED TO			
TIM				•					DATU			
DAT			,						FIELD PARTY:			
YE No.	DE	MPLE PTH FEET	PEN	TANDA IETRA SISTA	RD TION	NGTH OVERY	ELEVATION	DEPTH IN	DESCRIPTION  Soil type, color, texture, consistency, Sampler Driving Notes Blows per foot on casing, depths wash water lost, observed			
Ž	FROM		l	ows ,	/ 6"			FEET	FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.			
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AMPLE DEPTH FEET DIM TO	ST PEN RE	TANDA METRAT SISTAN	RD TION ICE	TOTAL LENGTH RECOVERY		1	PTH	TYPE OF BORING RIG  CASING USED SIZE DRILLING MUD USED  BORING BEGUN BORING COMPLETED  GROUND ELEVATION REFERRED TO  DATU  FIELD PARTY:  DESCRIPTION  SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED
AMPLE DEPTH FEET	S PEN RE	SISTAM LOWS /	ION	TOTAL LENGTH RECOVERY	ELEVATION	1	N	BORING COMPLETED  GROUND ELEVATION REFERRED TO  FIELD PARTY:  DESCRIPTION  Soil type, color, texture, consistency, sampler driving notes
AMPLE DEPTH FEET DM TO	PEN RE BL	SISTAM LOWS /	ION	TOTAL LENGTH RECOVERY	ELEVATION	1	N	FIELD PARTY:  DESCRIPTION  Soil type, color, Texture, Consistency, Sampler Driving Notes
AMPLE DEPTH FEET DM TO	PEN RE BL	SISTAM LOWS /	ION	TOTAL LENGTH RECOVERY	ELEVATION	1	N	DESCRIPTION  Soil Type, color, Texture, Consistency, Sampler Driving Notes
PEPTH FEET	PEN RE BL	SISTAM LOWS /	ION	TOTAL LENGTH RECOVERY	ELEVATION	1	N	DESCRIPTION  Soil type, color, texture, consistency, sampler driving notes
PEPTH FEET	PEN RE BL	SISTAM LOWS /	ION	TOTAL LENGTH RECOVERY	ELEVATION	1	N	DESCRIPTION  Soil type, color, Texture, Consistency, Sampler Driving Notes
PEPTH FEET	PEN RE BL	SISTAM LOWS /	ION	LENGTH RECOVERY	ELEVATION	1	N	SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING HOTES
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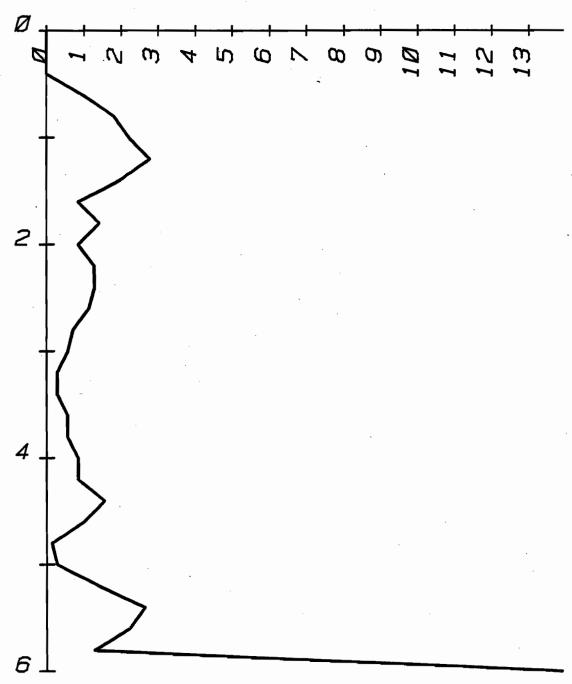
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Company			<u>-</u>	· ·	Pr	obe No. <u>.\$.54</u>	Date	
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ELEV.	DEPTH (Ft.)	DEPTH (m)	C or C+S	GAGE RI HIGH	EADINGS LOW	CONE RESIST.	SLEEVE RESIST.	FRICTION RATIO
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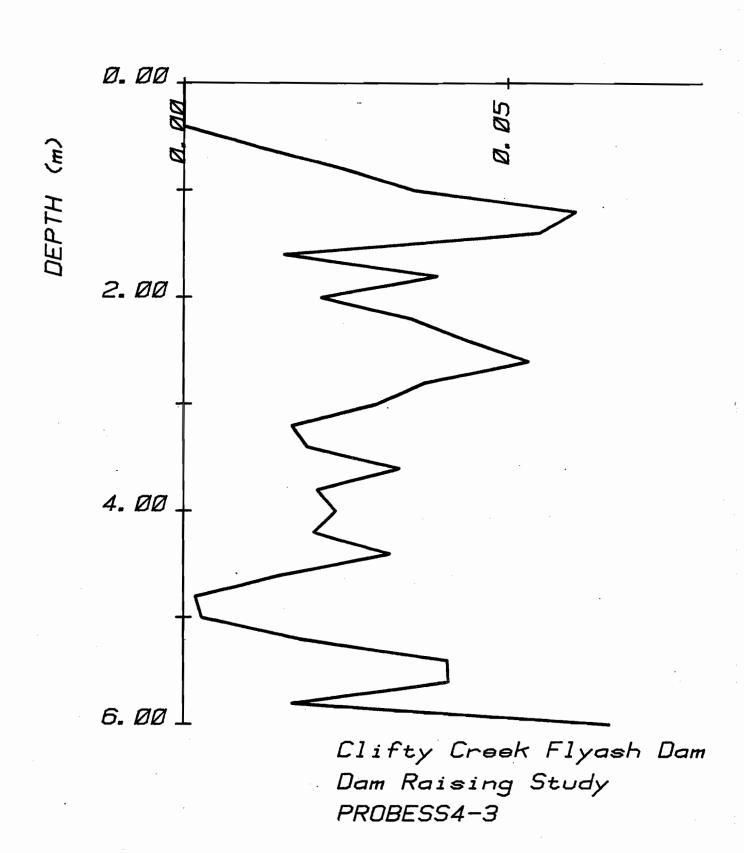




Clifty Creek Flyash Dam Dam Raising Study PROBE SS4-3



Clifty Creek Flyash Dam Dam Raising Study PROBE SS4-3



### AMERICAN ELECTRIC POWER SERVICE CORPORATION

## AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

_										
		KE						_		BORING NO. 554-4 DATE 7-26-84 SHEET / OF
ROJ	ECT _C	JIET,	1 CR	EEIL	FLY	ASI	1 12Am			TYPE OF BORING SPT - CORT RIG B-61
Lœ	ATION (	F BOR	HO:							CASING USED DRILLING MUD USED
					·		·····	<del></del>		BORING BEGUN 7-24-84 BORING COMPLETED
TIM	ER LE	VEL								GROUND ELEVATION 450.70 = REFERRED TO
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UAI										FIELD PARTY: NEDGIA F ARM BETCI
Ŷ	SΔ	MPLE	s	TANDA	RD	≿				DESCRIPTION
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SAMPL	FROM	A TO	l a	LOWS	/ 8"°	ڳر∟ا		"	'	FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
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_										BORING NO.SS4-4 DATE SHEET _2 of _6
3										TYPE OF BORING RIG
				-						CASING USED SIZE DRILLING MUD USED
Loc	ATION O	F BOR	NG:						ŀ	BORING BEGUN BORING COMPLETED
WAT	ER LEV	/EL	<del>-</del>							GROUND ELEVATION REFERRED TO
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DAT									١.	FIELD PARTY:
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Ŷ	SAL	MPLE	SI	TANDA	RD					DESCRIPTION
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7		FEET	RE!	SISTA	NCE	TOTAL LENGTH RECOVERY	ELEVATION			BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVES
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## ACP CIVIL CNOWCERING LABORATORY

							Boring No. <u>554-4</u> Date <u>7-31-84</u> Sheet <u>3</u> of <u>6</u>
7							Type of Boring Rid
cation		or:no.					Type of BoringRigRigRig
Cation	0, 0,	J. 11.19.		_		Boring begun Boring completed Ground Elevation referred to	
Nater Le	vel						Ground Elevation referred to
Time					-		Date.
<u>Date</u>		<u> </u>	·	_			Field Party:
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ELEVATION	25	Sample Deptrifrom—to (in feet)	]	5=		Leg	- DESCRIPTION
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	SAMPLE	In In		Recovery (in feet)	- a	Graph	water lost. Observed fluctuations in water level, notes on drilling ease, etc.
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## . AEP CIVIL ENGINEERING LABORATORY

Job No					LO	6 0	or Boring
Company						Boring No Date Sheet 4 of 2	
							Type of BoringRigRig
cation	of Bo	rıng:			•		Casing used Size Drilling mud used
Water Lev	el	ĺ					Boring begun Boring completed  Ground Elevation referred to
Time	<u> </u>						Ground Elevation referred to Data
Date							Field Party:
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ELEVATION FEET.	SAMPLE NO.	Sample Deptr. from-to (in feet)	900	Recovery (in feet)	Deptn 1n feet	Graph Log	DESCRIPTION  Rock Type. Color, Quality. Drilling observations. Det he water lost. Observed fluctuations in water level, notes on drilling ease, etc.
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### AEP CIVIL ENGINEERING LABORATORY

Job No.					FO	G OF	F BORING
_							Boring No. 559-4 Date Sheet 5 of  Type of Boring Rig  Casing used Size Drilling mud used
7							Type of Boring Ria
ocation	of Bo	ring:					Casing used Size Drilling mud used
					·		Boring begun Boring completed
Water Lev	rel						Ground Elevation referred to
Time							Field Party: Da
Date							Fleid Faity.
ELEVATION	SAMPLE NO.	Sample Depti- from-to (in feet)	900	Recovery (in feet)	Deptn 1n feet	Graph Log	DESCRIPTION  Rock Type, Color, Quality, Orilling observations, Dept water lost, Observed fluctuations in water level, note on drilling ease, etc.
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## . . . AEP CIVIL ENGINEERING LABORATORY

Job No.			d of boximo
Сотралу			Boring No.5544 Date Sheet 6 of 6
			Type of BoringRigRigRig
ocation of B	oring:	•	Casing used Size Drilling mud used Boring begun Boring completed
Water Level			Boring begun Boring completed Ground Elevation referred to
Time			Field Party:
Date			Field Farty:
ELEVATION FEET	Sample Depti- from-to (in feet) ROD	Recovery (in feet) Depth in feet	DESCRIPTION  Rock Type, Color, Quality, Drilling observations, Dr the water lost, Observed fluctuations in water level, note: on drilling ease, etc.
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			Stopped boxing 54.8

HP	NO	CKE	c			-		· ·	BORING NO. 5544 DATE 7/26/84 SHEET 1 OF
-ROJI	ECT _C	Lifty	CKF	lyash	Stock	ige F	KeA		Type of Borine Split spoon Rig 5-61
		F BOR							CASING USED SIZE DRILLING MUD USED BORING BEGUN 7/26/84 BORING COMPLETED
				_					BORING BEGUN 7/26/84 BORING COMPLETED
	TER LEY	/EL							GROUND ELEVATION 450.70 REPERRED TO
TIM									DATU
DAT	E		_						FIELD PARTY: COMME & MIKE
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1 2		FEET	RE	SISTA	NCE	TOTAL LENGTH RECOVER	ELEVATION		
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WAT	ER LEV	EL							GROUND ELEVATION 450.70 REFERRED TO
TIME									DATU
DAT	E								FIELD PARTY: COONIE & MIKE
SAMPLE		APLE		TANDA		JI &			DESCRIPTION
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### AMERICAN ELECTRIC POWER SERVICE CORPORATION

## AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

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Jos No.\_ MPANY IKEC BORING NO. 555-1 DATE 6-20-84 SHEET 1 OF 7 PROJECT CLIFTY CREEK FIY ASH STORAGE AREAYPE OF BORING SOT - CORE RIG 13-61 CASING USED \_\_\_\_\_ SIZE \_\_\_\_ DRILLING MUD USED \_ LOCATION OF BORING: BORING BEGUN 6-20-84 BORING COMPLETED 6-21-84 GROUND ELEVATION SO4.18 REFERRED TO DRY TO TOP OF ROCK WATER LEVEL DATUM FIELD PARTY: Roush + LamberT DATE DESCRIPTION SAMPLE STANDARD DEPTH SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES DEPTH PENETRATION ELEVATION IN IN FEET RESISTANCE BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FEET FROM TO BLOWS / 6" FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC. Sandy Clay- DR. BR., moist, Low To SANdy Clay - Yellowish BR., moist, bow To med. Plasticity Same AS SAMPle NO. Z 3 12.5 14.0 Clayer SAND - Yellowish BR. Moist, 100% Fine GRAIN, PORLY GRADE & 19.0 10 20. EHOINEER

FORM CE15 REV. 5/61

#### AMERICAN ELECTRIC POWER SERVICE CORPORATION

		_							BORING NO. 555-/ DATE SHEET 2 OF 7
/									TYPE OF BORING RIG
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DAT	E					·			FIELD PARTY:
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ect _						Type of BoringRig
ect_ cation	of Bo	ring:				Casing used Size Drilling mud used
W-A 1 -		1				Boring begun Boring completed
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Date						Field Party:
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ELEVATION	SAMPLE NO.	Sample Depth from-to (in feet)	BaD	Recovery (in feet)	Depth in feet	DESCRIPTION  Rock Type. Color. Quality. Drilling observations. Depths water lost. Observed fluctuations in water level. notes on drilling ease, etc.
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Job No						•					
Company _							Boring No. Date Sheet 4 of				
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ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	Bab	Recovery (in feet)	Depth In feet	Graph Log	DESCRIPTION  Rock Type, Color, Quality, Orilling observations, Dept s water lost, Observed fluctuations in water level, note on drilling ease, etc.				
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ELEVATION FEET	SAMPLE	Sample Depth from-to (In feet)	Rad	Recovery (in feet)	Depth In feet	Graph t	Rock Type, Colo water lost, Cbs on drilling eas	served fluctu	Drilling observations ations in water level	. De
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Date								Field Party:				
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					,		Boring No Date Sheet _7 of _7  Type of Boring Rig  Casing used Size Drilling mud used
							Type of BoringRig
tion	of Bo	oring:					Casing used Size Drilling mud used
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ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	ROO	Recovery (in feet)	Depth 1n feet	Graph Log	DESCRIPTION  Rock Type, Color. Quality, Drilling observations, Depths water lost, Observed fluctuations in water level, notes on drilling ease, etc.
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							12.3 - 72.7 GRAY CALCAREOUS Shale
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					78	يحورينان ويران سائدها	Stopped boring 77.2
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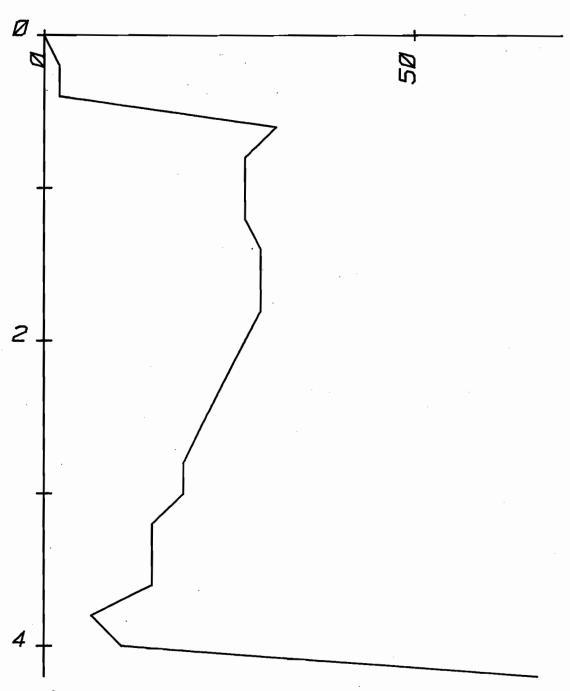
Dutch Cone Test

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rder_	Lamber	T	·		P	age / of <u>\forall </u>	Elevati	on_451.00
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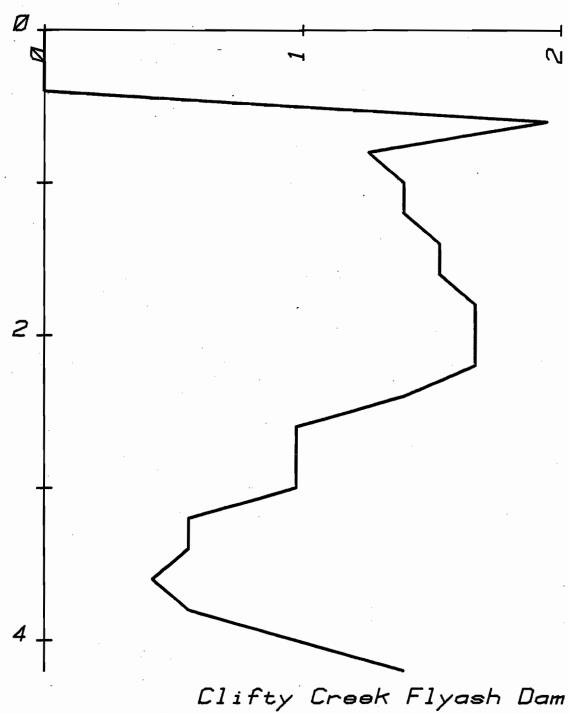
ation	of Bo	ring:				Boring No_Ssc-3Date Sheet of  Type of Boring Rig  Casing used Size Drilling mud used  Boring begun Boring completed  Ground Elevation referred to  Datum					
Time Nate							Field Party:				
ELEVATION	SAMPLE NO.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth In feet	Graph Log	DESCRIPTION  Rock Type. Color. Quality. Drilling observations. Depths water lost. Observed fluctuations in water level, notes on drilling ease, etc.				
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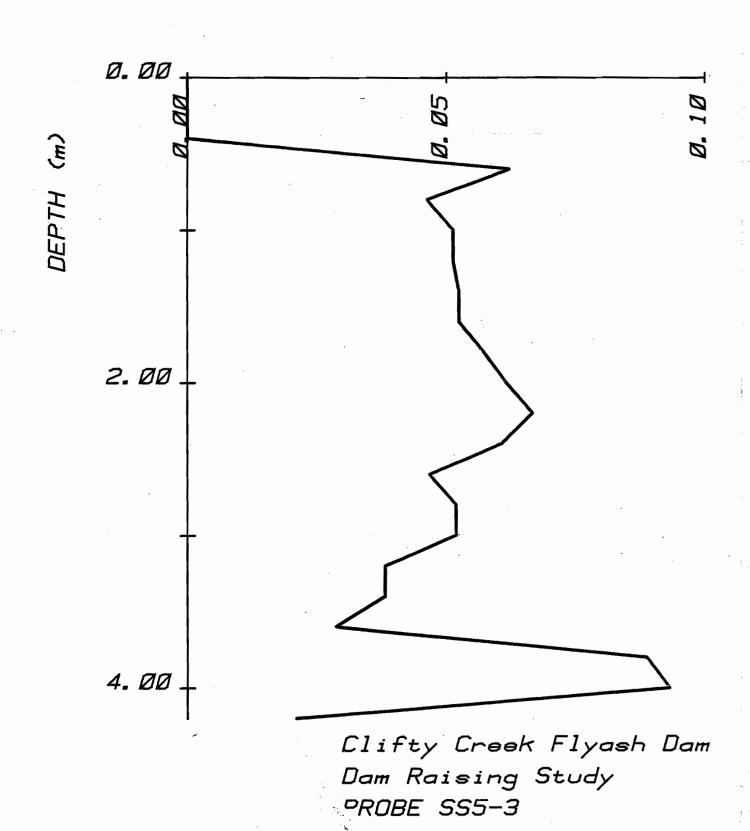
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Clifty Creek Flyash Dam Dam Raising Study PROBE SS5-3



Clifty Creek Flyash Dam Dam Raising Study PROBE 555-3



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# AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

	NO	KEC			<u> </u>		h Dam		BORING NO. SSG-1 DATE 7-11-84 SHEET 1 OF 10  TYPE OF BORING SOT- CORC RIG B-61
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mpany_							Boring No_ <u>SS&amp;-/</u> Date Sheet <u>2</u> of <u>10</u>
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LOG OF BORING Boring No. SS(-) Date Sheet 4 of 11

Type of Boring Rig
Casing used Size Drilling mud used
Boring begun Boring completed
Ground Elevation referred to
Datum Company \_\_\_\_\_ ation of Boring: Water Level Time Field Party: Date Sample Depti DESCRIPTION ELEVATION FEET SAMPLE NO. from-to (in feet) Recovery (in feet) Graph Log Depth In Feet Rock Type, Color, Quality, Drilling observations, Deptiwater lost, Observed fluctuations in water level, note on drilling ease, etc. 90

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c-pany_							Boring No. <u>\$\$6-1</u> Date Sheet _5_ of _/6
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Time							Datum				
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							Type of BoringRigRig				
ation	of Bo	ring:					Casing used Size Drilling mud used				
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Time							Datum				
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ELEVATION FEET	SAMPLE NO.	Sample Depth from—to (In feet)	Rao	Recovery (in feet)	Depth 1n feet	Graph Log	water lost, Observed fluctuations in water level, note on drilling ease, etc.				
1		55.6	1	1	t :	1	GRAY CACCAREOUS SHALE W				
	ļ	60,6	<u> </u>	<del>-</del>	•	1	Lime STone Layers				
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Спралу							Boring No. <u>\$\$6-1</u> Date Sheet <u>9</u> of <u>'0</u>				
ect_							Type of BoringRigRigRig				
ation	of Bo	oring:					Casing used Size Drilling mud used				
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D te							Field Party:				
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ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	Rao	Recovery (in feet)	Depth In feet	Graph Log	DESCRIPTION  Rock Type. Color. Quality. Orilling observations. Depths water lost. Observed fluctuations in water level. notes on drilling ease. etc.				
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Company  Company  Ation of Bo  Water Level  Time  Date	oring:				Boring No. SS-/ Date Sheet /O of / Type of Boring Rig Casing used Size Drilling mud used Boring begun Boring completed Ground Elevation referred to Datum Field Party:				
ELEVATION FEET SAMPLE NO.	Sample Deptr from-to (in feet)	ROD	Recovery (in feet)	Depth in feet	Graph Log	DESCRIPTION  Rock Type, Color, Quality, Drilling observations, Dept; water lost, Observed fluctuations in water level, note on drilling ease, etc.			
				70 71 72 73 74 75		Stocked bearing 75/ ET			
						Stopped boring 75.6 FT			

FORM CE-5 REV. 5/81

JOS No.

### AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY

		IKE		ce K	FI	y <del>A</del> .S.1	n Dam	_	BORING NO. SS6-Z DATE 7.29-84 SMEET 1 OF 4  TYPE OF BORING SPT- CORE RIG 8-61							
*3		F Bori			/			_	CASING USED SIZE DRILLING MUD USED							
<u></u>	TER LE	<u>,,,  </u>	•	12					BORING BEGUN 7-24-84 BORING COMPLETED 7-25-2							
Ti		YEL		13	4				GROUND ELEVATION 452,99 REFERRED TO DATE							
DA									FIELD PARTY: Roush - Lambert							
	Т							_								
Ź	I .	MPLE		TANDA JETRA		TOTAL LENGTH RECOVERY		DEPT	DESCRIPTION DESCRIPTION							
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SAMPLE	FROM	A TO		.ows /				FEE	PLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.							
<u></u>	PROM	10		-0w3 /		<del>                                     </del>		<del> </del>	PERSONAL IN WATER DEVEL, NOTED ON BRIDGING BASE, ETC.							
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1	3.0	4.5	6	8	/z	5"		. م ا	- Silty Clay. DR. BR. maist, Low to med - Plasticity, Slight Real Tion TO HCL							
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2	8.0	9,5	3	3	6	13"		9.	106 % Fine GRAIN, MOIST							
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3	13.0	14,5	2	2	3	15"			TIDO % Fine Ganin							
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Job No									
Company_							Boring No <i>556-2</i> Date Sheet <u>Z</u> of _/		
							Type of Boring Rig Casing used Size Drilling mud used		
ation	of Bo	rıng:					Casing used Size Drilling mud used		
		T				Boring begun Boring completed			
Water Lev	/61					Ground Elevation referred to Datum			
Time							Field Party:		
Date		<u> </u>					rield larty.		
Z	No.	epth b	. ,	2-5		2	DESCRIPTION		
ELEVATION FEET	SAMPLE	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth In feet	Graph Log	Rock Type, Color, Quality, Drilling observations, Dept: ; water lost, Observed fluctuations in water level, note		
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		24.5	12%	3.2			GRAY HARD LIME STONE W/ LAMINATED		
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	ob No					LO	G. C	OF BORING
Type of Boring Rig Casing used Size Drilling mud used Boring begun Boring completed Ground Elevation referred to Date    Second Elevation   Second	್ಷಿಂದಾರವಾ 💆							Boring No. <i>5562</i> Date Sheet <u>3</u> of <u>4</u>
Boring begun Boring completed Ground Elevation referred to Date    Solid Party:	piect							Type of BoringRig
Time Tate  Total Tate   Total T	ation	of Bo	ring:			_		Casing used Size Drilling mud used
Time  Tate  Time  Total of the party:  DESCRIPTION  DESCRIPTION  Rock Type, Color, Quality, Drilling observations. Deoth water lost. Observed fluctuations in water level, notes on drilling ease, etc.  29.5  35.2  39.5-35.2  Gray Hand Lime 5 Towe w/ Lamina 76/  Layers of Calendeous Shale								Boring begun Boring completed
DESCRIPTION  DESCRIPTION  Rock Type. Color. Quality, Drilling observations. Deoth water lost. Observed fluctuations in water level, notes on drilling ease, etc.  29.5  35.2  29.5  35.2  29.5  35.2  29.5  35.2  29.5  35.2  29.5  35.2  29.5  35.2  29.5  35.2  29.5  35.2  29.5  35.2  29.5  35.2  29.5  35.2  29.5  35.2  29.5  35.2  29.5  35.2  29.5  35.2  30.6  29.5  35.2  30.6  29.5  35.2  30.6  29.5  30.6  29.5  30.6  29.5  30.6  29.5  35.2		rei						Ground Elevation Teleffed to
DESCRIPTION  DESCRIPTION  Rock Type, Color, Quality, Drilling observations, Depth water lost, Observed fluctuations in water level, notes on drilling ease, etc.  29.5-35.2  Gray Hard Lime STowe w/ Laminate/ Layers of Calareous Shale								
29.5 35.2  29.5  2								
29.5 35.2  29.5  29.5  29.5-35,2  Gray Hard hims Stone w/ Laminate/ Layers of Chloreous Shale	ELEVATION FEET	SAMPLE NO.	Sample Depti- from-to (in feet)	RDD	Recovery (in feet)	Depth In Feet	Graph Log	DESCRIPTION  Rock Type. Color. Quality. Drilling observations. Depth water lost. Observed fluctuations in water level, notes on drilling ease, etc.
29.5 35.2  29.5  29.5  29.5-35,2  Gray Hard hims Stone w/ Laminate/ Layers of Chloreous Shale		ĺ				t :	$\Gamma$	
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29.5 35.2  29.5-35,2  Gray Hard hims stowe w/ Laminate/ Layers of Calenreous Shale		=			1	-	}	
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29.5 35.2  29.5-35,2  Gray Hard hims stowe w/ Laminate/ Layers of Calonregus Shale								
29.5 - 35.2  29.5 - 35.2  GREY HARD LIME STOWE W/ LAWINATE/  LAYERS OF CALCAREOUS Shale						F " -		
29.5 - 35.2  29.5 - 35.2  GREY HARD LIME STOWE W/ LAWINATE/  LAYERS OF CALCAREOUS Shale						•		
29.5 - 35.2  29.5 - 35.2  GREY HARD LIME STOWE W/ LAWINATE/  LAYERS OF CALCAREOUS Shale					1	_		
29.5 - 35.2  29.5 - 35.2  GREY HARD LIME STOWE W/ LAWINATE/  LAYERS OF CALCAREOUS Shale	, ·				-	<b>,</b>		
29.5 - 35.2  29.5 - 35.2  GREY HARD LIME STOWE W/ LAWINATE/  LAYERS OF CALCAREOUS Shale	, i				1.	t =		
29.5-35,2  GRAY HARD LIME STONE W/ LAMINATE!  LAYERS OF CALCARROUS Shale	, i		29.5		1	79		
GREY HARD LIME STOWE W/ LAMINATE!  LAYERS OF CHICAREOUS Shale	*· .h				5.6			10 4 26 3
	- 75.5							GRAY NARA LIMA STOWE WI LAWINATES
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Company_			<u>-</u> -		·		Boring No. 556-2 Date Sheet 4 of
Project					· .		Boring No. <u>SSC-2</u> Date Sheet <u>4</u> of Type of Boring Rig Casing used Size Drilling mud used
ation	of Bo	ring:					Casing used Size Drilling mud used
Water Lev							Boring begun Boring completed Ground Elevation referred to Datur
Time							Datur
Date							Field Party:
ELEVATION	SAMPLE NO.	Sample Deptrifrom-to	RDD	Recovery (in feet)	Depth In feet	Graph Log	DESCRIPTION  Rock Type, Color, Quality, Drilling observations, Dep is water lost, Observed fluctuations in water level, not on drilling ease, etc.
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		35. E 39.5	O	4.1			GRAY HARD Lime STONE W/ LAMINATED
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FORM CE-5 REV. 5/61

#### AMERICAN ELECTRIC POWER SERVICE CORPORATION

:1	No						•		•	557-1
	ANYI			10	<b>-</b>		<u>.                                    </u>	_		BORING NO DATE 6-19-84 SHEET 1 OF 10  TYPE OF BORING SPT-CORE RIG B-61
·				CEK	riy r	5 h L	)Am	_		CASING USED SIZE DRILLING MUD USED
Loc	ATION O	F Bó	RING:							BORING BEGUN 6-19-84 BORING COMPLETED 6-19-84
WAT	ER LEV	/EL	DRY	To To		E 12-	. K	-	1	GROUND ELEVATION 504,17 REFERRED TO
TIM	E		DAY.	1012	P	- 150	<u> </u>		1	DATUM
DAT	E								]	FIELD PARTY: Roush + LAMberT
Ž	1			741104			1			DESCRIPTION
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7 7		FEET		SISTA	NCE	520	ELEVATION	_!	N	
1 3	FROM			LOWS	/ e <sup>H</sup>			FE	ET	PLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
<u> </u>	FROM	1		LUWS	<del>/ 0</del>	<del>                                     </del>	ELEVATION	-	٠.	PEGGIONIONS IN MAICH EBYES, NOISE ON SAISEING CASE, E1C.
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7)					501	١		۱ '	-	GRAVELLY Clay. Yellowish BR. med.
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									=	GRAVELLY CLAY. Yellowish BR. med.  PHASTICITY, STRONG REALTION TO HCh.  GRAVEL CONSIST OF ANGULAR hime STONE  FRAG. 34" MAY Size  AUGER REFUSAL 14.4  CL
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Company _						Boring No. Date Sheet Z of J		
Project_							Type of BoringRig	
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Date							Field Party:	
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		Sample Depth from-to (in feet)	ĺ		i	۱_	negon To Trou	
ELEVATION FEET	2	E 6 8	·_	Recovery (In feet)	سے ع	100	DESCRIPTION	
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	SAMPLE	fr fr	_	£ 5	ă	Graph	on drilling ease, etc.	
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		14.4-15.5	0	1.1	<b>{</b>		14.4 - 14.5	
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	i ,	15.5 - 25.5	0	9.2		1	16.5-15.6 GRAY HARD Lime STONE	
		10.0 - 20. 5		1		l	15.6-16.2 SOFT GRAY+GRAY BR. Shaley	
					E., 3		Lime STONE	
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Потралу_						557-1 Boring No Date Sheet 3 of _ro
==iect						Type of Boring Rig  Casing used Size Drilling mud used
cation	of Bo	ring:				Casing used Size Drilling mud used
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Time	<u> </u>		-,			Ground Elevation Datus
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ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth In feet	DESCRIPTION  Rock Type, Color, Quality, Drilling observations, Depths water lost, Observed fluctuations in water level, notes on drilling ease, etc.
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Job No										
Company_		·				Sheet 4 ofSheet 4 of				
							Type of BoringRigRigRigRig			
cation	of Bo	oring:					Casing used Size Drilling mud used			
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ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	ROD	Recovery (in feet)	Depth In Feet	Graph Log	DESCRIPTION  Rock Type. Color. Quality. Drilling observations. Dep s water lost. Observed fluctuations in water level. not so on drilling ease, etc.			
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		30.5-33.3		7.7		ł	30.5-35.5 GRAY HARD Lime STORE			
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cation of Boring:  Nater Level  Time  Date							Type of BoringRig
ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	99	Recovery (in feet)	Bepth in feet	Graph Log	DESCRIPTION  Rock Type, Color, Quality, Drilling observations, Department lost, Observed fluctuations in water level, not on drilling ease, etc.
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Job No			·				557-1	
Company_						Boring No Date Sheet of _/ Type of Boring Rig Casing used Size Drilling mud used		
ect_							Type of BoringRig	
cation	of Bo	rıng:					Casing used Size Drilling mud used	
l		· <del>-</del>	<del></del>		<u> </u>	Boring begun Boring completed Ground Elevation referred to		
Water Lev Time	/61					Ground Elevation Datum		
Date						Field Party:		
Date								
ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	780 1	Recovery (in feet)	Depth In feet	Graph Log	DESCRIPTION  Rock Type. Color. Quality. Drilling observations. Dept ; water lost. Observed fluctuations in water level. noti-	
ELE	54	Sam fr (1r		# 5	-	چ	on drilling ease, etc.	
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Job No							SS7-1			
		·					Boring No. DateSheet / of 10			
iect							Type of BoringRigRigRig			
cation	of Bo	ring:					Casing used Size Drilling mud used			
Vater Lev		г——					Boring begun Boring completed Ground Elevation referred to			
Time	61						Ground Elevation Date			
Date				_			Field Party:			
1 -	8	Sample Depth from-to (in feet)		>_			DESCRIPTION			
ELEVATION FEET	. Z	e t	6	Recovery (In feet)	5 6 25	2	Rock Type, Color, Quality, Drilling observations, Depth:			
EVA'	SAMPLE	rom Tom	900	99 6	Depth 1n feet	Graph	water lost. Observed fluctuations in water level, notes			
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ect			<u>-</u> -			Type of BoringRigRigRig			
Cation	of Bo	ring:					Casing used Size Drilling mud used		
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ELEVATION FEET	SAMPLE NO.	to te		Recovery (in feet)	등급	Graph Log	Rock Type, Color, Quality, Drilling observations, Dept		
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Job No		<del></del>				
Company						Boring No Date Sheet 9 of 10
Pojert						Type of BoringRig
cation	of Bo	ring:				Type of BoringRigRig
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<u>Date</u>		<u> </u>				Field Party:
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ELEVATION	ų,	e tr		Recovery (in feet)	Depth In feet	Rock Type, Color, Quality, Drilling observations, Depths
EVAT	SAMPLE	1 0 c	2	9 -	i e	Rock Type. Color. Quality. Drilling observations. Depths water lost. Observed fluctuations in water level, notes on drilling ease, etc.
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## AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

Company_							Boring No Date Sheet <u>/6</u> of <u>/</u>
P-ect_	(0-						Type of BoringRig  Casing used Size Drilling mud used
ation	01 80	ring;					Boring begun Boring completed
Water Lev	/el						Boring begun Boring completed Ground Elevation referred to
Time							Datum
Date		·	·-				Field Party:
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音	2	Dep Ct	1	2=		L68	DESCRIPTION
ELEVATION FEET	SAMPLE NO.	Sample Depth from-to (in feet)	Ago	Recovery (in feet)	Depth In feet	Graph L	Rock Type, Color, Quality, Orilling observations, Dept water lost, Observed fluctuations in water level, not on drilling ease, etc.
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FORM CE-5

#### AMERICAN ELECTRIC POWER SERVICE CORPORATION

## AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

Loc	ATION O	# Bori	M <b>6</b> :			:	F.A.D.		TYPE OF BORING SPT RIG 8-6/  CASING USED SIZE DRILLING MUD USED  BORING BEGUN 8-22-84 BORING COMPLETED 8-23-84
WAT	ER LEV	/EL							GROUND ELEVATION 529.42 REFERRED TO
TIM									DAT
DAT	<u> </u>								FIELD PARTY: Roush + LamberT
SAMPLE No.	DE	MPLE PTH FEET	PET	TANDA NETRA SISTAI	TION	TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION  SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, DESERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
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#### AMERICAN ELECTRIC POWER SERVICE CORPORATION

## AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

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ROJE	CT								TYPE OF BORING RIG		
Loca	ATION O	F BORI	N 6 1						CASING USED SIZE DRILLING MUD USED		
									BORING BEGUN BORING COMPLETED		
WAT TIM	ER LEV	EL				<u>-</u>			GROUND ELEVATION REFERRED TO		
DAT	<del>-</del>								FIELD PARTY:		
JA. 1		<u>_</u>							FIELD FARIT!		
LE No.	DE	APLE PTH FEET	PEN	TANDA IETRAT SISTAN	TION	TOTAL LENGTH RECOVERY	ELEVATION		John Tire, Count, Tentone, Conditional, Control of Control		
SAMPL	l	то	l	ows /		RECET		FEET	FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.		
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cation	of Go						Type of BoringRigRigRig
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Vater Lev	/el						Ground Elevation referred to
Time							Oatu
Date							Field Party:
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ELEVATION FEET	SAMPLE NO.	Sample Deptire from-to	900	Recovery (in feet)	Depth in feet	h 1.09	DESCRIPTION  Rock Type, Color, Quality, Orilling observations, Depths
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ر در	35.3 39.0	22%	3.7	- 35 -	26	RAY CHICARCOUS SHALE	
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LOG OF BORING

Job No						.06	U	P BORING
Company_								Boring No. NSI-Date Sheet 4 of
								Type of BoringRigRig
Eation	of Bo	ring:						Casing used Size Drilling mud used
Water Lev	/61	<u> </u>					_	Boring begun Boring completed Ground Elevation referred to
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ELEVATION FEET	SAMPLE NO.	Sample Deptrirom-to (in feet)	Rad	Recovery (in feet)	Depth 1n		Graph Log	DESCRIPTION  Rock Type. Color. Quality, Drilling observations. Dep s water lost. Observed fluctuations in water level. notes on drilling ease, etc.
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FORM CE-5 REV. 5/81

#### AMERICAN ELECTRIC POWER SERVICE CORPORATION

## AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

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FORM CE-5 REV. 5/81

#### AMERICAN ELECTRIC POWER SERVICE CORPORATION

## AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

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## AEP CIVIL ENGINEERING LABORATORY

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FORM CE-5 REV. 5/81

## AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

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### AEP CIVIL ENGINEERING LABORATORY

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#### AEP CIVIL ENGINEERING LABORATORY

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# Indiana - Kentucky Electric Corporation Clifty Creek Plant Madison, Indiana

Flyash Dam Raising Feasibility Report

APPENDIX H
Boring Location Plan and Profile
Drawings CE-CCBP - 1 to 7 - 84

January 31, 1985

Prepared by

Soils, Foundation and Hydro Section Civil Engineering Division American Electric Power Service Corporation

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#### **BORING LOG TERMINOLOGY**

#### Stratum Depth:

Distance in feet and/or inches below ground surface.

#### Stratum Elevation:

Elevation in feet below ground surface elevation.

#### **Description of Materials:**

Major types of soil material existing at boring location. Soil classification based on one of the following systems: Unified Soil Classification System, Ohio State Highway Classification System, Highway Research Board Classification System, Federal Aviation Authority Classification System, Visual Classification.

#### Sample No.:

Sample numbers are designated consecutively, increasing with depth for each boring.

#### Sample Type:

"A" Split spoon, 2" O.D., 1-3/8" I.D., 18" in length.

"B" One of the following:

Power Auger Sample
Piston Sample
Diamond Bit NX: BX: AX:
Housel Sample
Wash Sample
Denison Sample

"C" Shelby Tube 3" O.D. except where noted.

#### Sample Depth:

Depth below top of ground at which appropriate sample was taken.

#### Blows per 6" on Sampler:

The number of blows required to drive a 2" O.D., 1-3/8" I.D., split spoon sampler, using a 140 pound hammer with a 30 inch free fall, is recorded for 6" drive increments. (Example: 3/8/9)

#### "N" Blows/Ft.:

Standard penetration resistance. This value is based on the total number of blows required for the last 12'' of penetration. (Example: 3/8/9: N = 8 + 9 = 17)

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#### Water Observations:

Depth of water recorded in test boring is measured from top of ground to top of water level. Initial depth indicates water level during boring, completion depth indicates water level immediately after boring, and depth after "X" number hours indicates water level after letting water rise or fall over a time period. Water observations in pervious soil are considered reliable ground water levels for that date. Water observations in impervious soils can not be considered accurate ground water measurements for that date unless records are made over several days' time. Factors such as weather, soil porosity, etc., will cause the ground water level to fluctuate for both pervious and impervious soils.

#### SOIL DESCRIPTION

#### Color:

When the color of the soil is uniform throughout, the color recorded will be such as brown, grey, black and may be modified by adjectives such as light and dark. If the soil's predominant color is shaded by a secondary color, the secondary color precedes the primary color, such as grey-brown, yellow-brown. If two major and distinct colors are swirled throughout the soil, the colors will be modified by the term mottled, such as: mottled brown and grey.

Particle Size	Visual	Soil C	Components
Boulders	Larger than 8"	Major Component:	Minor Component Term
Cobbles	8" to 3"	Gravel	Trace 1-10%
Gravel—Coarse	3" to 3/4"	Sand	Some 11-35%
Fine	2 mm. to 3/4"	Silt	And 36-50%
Sand —Coarse	2 mm0.6 mm.	Clay	
	(Pencil lead size)	•	
-Medium	0.6 mm0.2 mm.	Moist	ure Content
	(Table sugar and salt size)	Term	Relative Moisture
-Fine	0.2 mm0.06 mm.	Dry	Powdery
	(Powdered sugar and	Damp	Moisture content
	human hair size)	•	below plastic limit
Silt	0.06 mm0.002 mm.	Moist	Moisture content
Clay	0.002 and smaller		above plastic limit
	(Particle size of both		but below liquid
	Silt and Clay not visible		limit
	to naked eye)	Wet	Moisture content
			above liquid limit
Condition of Soi	Relative to Compactness	Condition of Soil	Relative to Consistency
Gra	nular Material		ive Material
Very Loose	5 blows/ft. or less	Very Soft	3 blows/ft. or less
Loose	6 to 10 blows/ft.	Soft	4 to 5 blows/ft.
Medium Dense	11 to 30 blows/ft.	Medium Stiff	6 to 10 blows/ft.
Dense	30 to 50 blows/ft.	Stiff	11 to 15 blows/ft.
. Very Dense	51 blows/ft. or more	Very Stiff	16 to 30 blows/ft.
		Hard	31 blows/ft. or more

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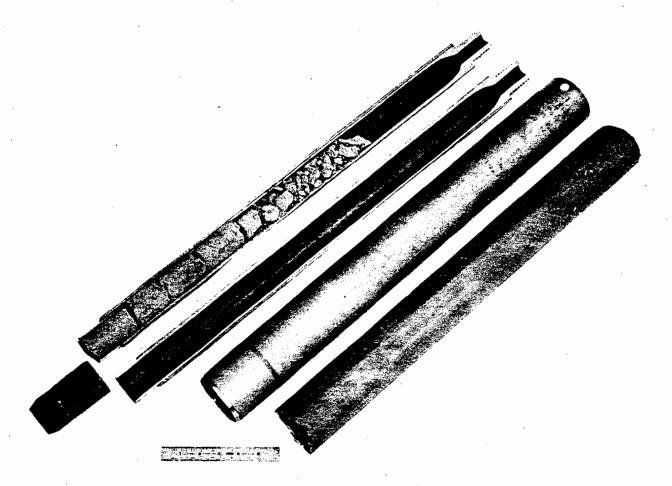
#### STANDARD PENETRATION RESISTANCE (ASTM D1586)

The purpose of this test is to determine the relative consistency of the soils in a boring, or from boring to boring over the site. This method consists of making a hole in the ground and driving a 2 inch O.D. split spoon sampler into the soil with a 140 pound hammer dropped from a height of 30 inches. The sampler is driven 18 inches and the number of blows recorded for each 6 inches of penetration. Values of standard penetration (N) are determined in blows per foot, summarizing the blows required for the last two 6 inch increments of penetration.

Example: 2-6-8; N = 14

#### THIN-WALLED SAMPLER (ASTM D1587)

The purpose of the thin-walled sampler is to recover a relatively undisturbed soil sample for laboratory tests. The sampler is a thin-walled seamless tube with a 3 inch outside diameter, which is hydraulically pressed into the ground, at a constant rate. The ends are then sealed to prevent soil moisture loss, and the tube is returned to the laboratory for tests.



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## BOWSER-MORNER, INC. 420 DAVIS AVENUE, P.O. BOX 51, DAYTON, OHIO 45401

#### ENGINEERING REPORT

REPORT TO: American Electric Power Service Corp.

REPORT DATE: Oc

October 8, 1984

1 Riverside Plaza P.O. Box 16631

REPORT NO.:

28089-1084-436

Columbus, Ohio 43216

Attention:

Mr. Kevin C. Miller,

Civil Engineering Division

REPORT ON:

Cone Penetration Test Results, Indiana-Kentucky Electric

Corporation, Clifty Creek Power Station Flyash Impoundment,

Madison, Indiana

Bowser-Morner, Inc., submitted copies of field boring logs in Bowser-Morner Report No. 28089-1084-425, dated October 2, 1984. Results of the cone penetration tests performed by Woodward-Clyde Consultants were not available at that time.

The results of the cone penetration tests are included with this report. If there are any questions regarding the test results, please contact the writer, or if you prefer, contact Mr. Frederick W. Johnson, Senior Project Engineer, Woodward-Clyde Consultants.

Respectfully submitted,

Bowser-Morner, Inc.

Robert L. Disney, Manager

Soils Exploration Department

RLD/mjj(#95) 2-Client 2-File 11 East Adams Street Suite 1500 Chicago, Illinois 60603 312-939-1000 Telex 253875 (WOODWARD CGO)

### **Woodward-Clyde Consultants**

October 3, 1984 84C3092

Bowser-Morner 420 Davis Avenue P.O. Box 51 Dayton, Ohio 45401

Attention:

Mr. Robert Disney

Re:

Cone Penetration Test Results

AEP Indiana-Kentucky Electric Corporation Clifty Creek Plant Flyash Impoundment

Near Madison, Indiana

#### Gentlemen:

This letter transmits the results of four cone penetration test (CPT) borings made at the referenced project on September 19, 20, 21 and 22, 1984. These CPT borings were made in accordance to our August 31, 1984 proposal. The CPT data are summarized in Tables 1 thru 4 and are shown in Figures 3 thru 6. These figures show the cone resistance in lb/in<sup>2</sup> vs depth for each of the CPT borings. A brief discussion of the WCC cone penetrometer system has also been included at the request of Mr. Kevin Miller of AEP.

It was a pleasure to do business with you. If additional information is required please contact us.

Very truly yours,

Frederick W. Johnson

Senior Project Engineer

Frederick w Jehrson



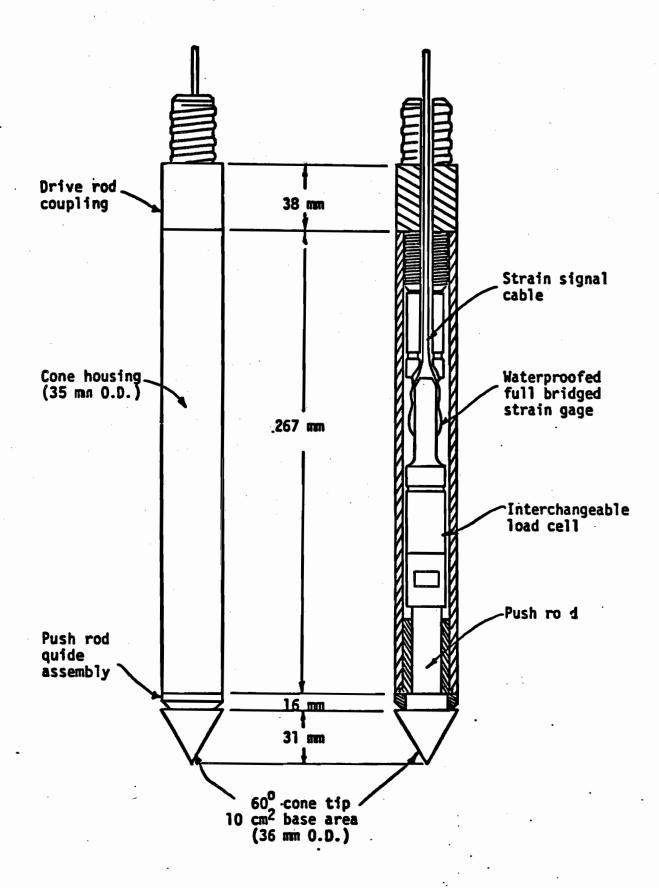
#### THE WOODWARD-CLYDE CONSULTANTS CONE-PENETROMETER SYSTEM

The Woodward-Clyde Consultants (WCC) cone penetrometer system is patterned after the 60 degree,  $10 \text{ cm}^2$  electronic Dutch cone employed in continuous (static) penetration testing and is shown in Figure 1. During cone-penetration resistance testing (CPT), the drill-rig advances the cone at a rate of about  $5\pm1$  ft/minute and continuous cone penetration is usually obtained at intervals of at least 5 feet, Figure 2.

The point resistance  $(q_c)$  is sensed by the load-cell just above the cone and depth events are sensed by a  $360^o$  potentiometer attached to the drive system. The analog (voltage) signals from the load-cell are sent via a signal cable to a strip chart recorder. There the signals are conditioned and displayed in real time. Three signal amplification ranges are available to provide an appropriate scale for the  $q_c$  reaction range anticipated.

As the strip chart record represents a voltage analog of  $\mathbf{q_c}$ , the record must be converted to load. A conversion factor is given for each of three signal amplification ranges and a load factor, given for the load-cell, is applied to convert the analog record of each CPT run.

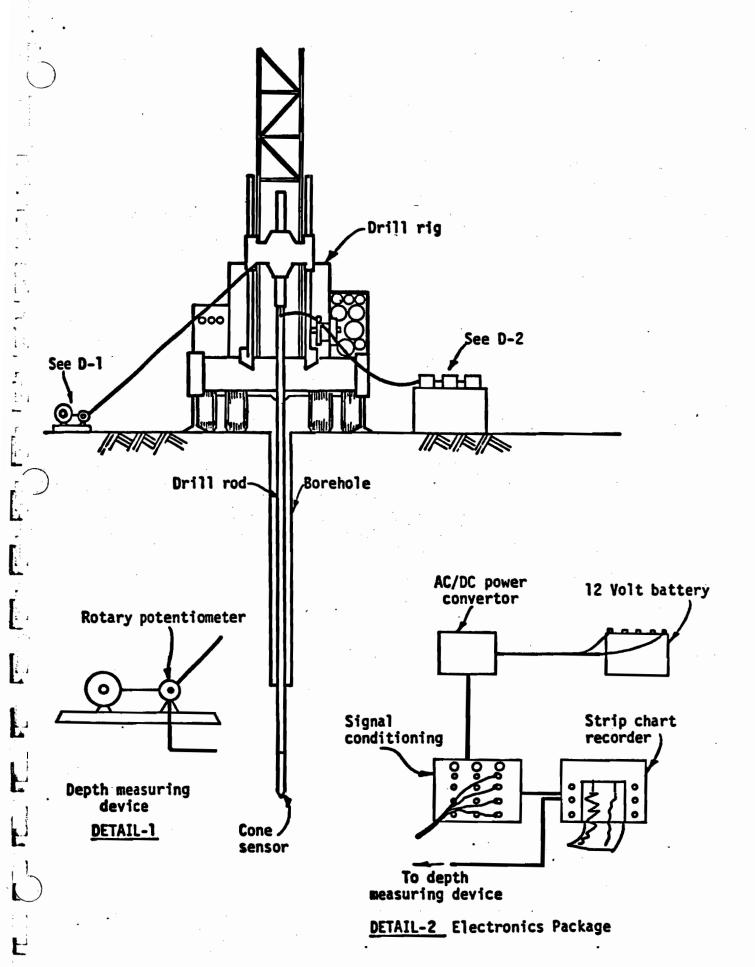
CPT data can be simply reduced by scaling the field record with a proportional divider, by conversion with a hand-held calculator, or alternatively, with a small computer program using either a commercial time-share facility or an in-house microcomputer. The program input for each continuous advance is the analog  $\mathbf{q}_{\mathbf{c}}$ , the test depth, the signal amplification range selected and the corresponding load factor.



jesta.

WCC ELECTRICAL CONE PENETROMETER

斯马克黎人 共产生



CONE PENETRATION TEST - APPARATUS SCHEMATIC

## LOG OF BORING No. 504-1 CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: AS DIRECTED

SURFACE ELEVATION: 502.2 (WATER)

DATE STARTED: 9/22/84

DATE COMPLETED: 9/22/84

STRATUM	DESCRIPTION OF MATE	RIAL	SAMPLE NO. & TYPE	SAMPL DEPTH		BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
				÷				
0'	BARGE					<u>.</u>		
					ł	. •	1	
2.0	TOP OF WATER	2						
	•							1
9.5	FLYASH							
							ŀ	
80.0	BOTTOM OF S	BORING						
				ŀ .				
	NOTE! CONE PENE	TEATION	7257	80	e,	15		
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			1.					
DRILLING ME	ETHOD: HOLLOW STEM AUGER	WATER OBSE	RVATIONS				SAMPLER:	
	IN IN	IITIAL DEPTH:	<i>2.0</i> H.2.0	······		☐ A. ☐ B.	SPLIT SPOO	N
		EPTH AFTER		1		_	SHELBY TU	3 E

BOWSER-MORNER
TESTING LABORATORIES, INC.

Woodward-Clyde	SUBJECT Table	Cone Resistance	PROJECT BOUSET	-Morrer
Consultants	is Depth		FILE No. 84630	92
CHICAGO	COMPUTED UN	CHECKED FW)	DATE 9-30-84 PAGE	1 or 2 pag
depth, ft	Lesistance 16/462	depth, ft	Lesistane	Boring 50-4-/
12.7	2788	38.3	18588	
13.7	37/7	39,/	50187	
14.8	9294	39,6	213762	
15,B		40,1	250933	
16.8	0	40,5	250958	
17.8	2323	40,75	315-996	
18.8	37/7	41,0	24629.	
19.8	14870	42,0	246291	
20,3	23235	43,0	250938	
20.6	57622	43,4	358748	
20.8	48328	44,0	330866	
2110	87363	45,0	277703	
Z2,3	18 588	46.0	204463	
23.3	37176	47,0	232250	
24.3	2/376	47,8	315976	
24.8	C9705	49,4	288NY	
25,5	176536	49,6	130116	
25,8	106831	49.9	171939	
26.8	18588	51,0	111528	
27.8	6041/	52.0	94798	
z8.8	30205	53.0	2 23056	
29.8	84575	54,0	297408	
3111	264879	55,0	269526	
31.4	195174	55,5	130116	
32,3	51117	56.0	83646	
33,3	7435-2	56.9	223056	••
34.1	55764	57.3	148704	
3416	63199	58.7	358748	
3510	66916	58.8	97587	
35,3	55764	59.6	386630	
36.Z	78069	60,0	456335	
36.9	44611	40,3	428453	
37.6	65058			

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Table 1

Woodward-Clyde

Consultants

CMICAGO

COMPUTED JUH CHECKED FW DATE 9-9-84 PAGE Z OF Z PAGES

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CMECKED FW DATE 9-9-84 PAGE Z OF Z PAGES

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61.0 130116 493511 67,2 63.0 111528 64,0 83646 65,0 260232 66,0 74352 67,0 428453 680 409865 2,80 465529 69,0 92 940 167292 69,3 20.0 55764 71,0 111528 297408 72,0 73.0 204468 745 148704 157998 745 750 55764 437747 75,9 126822 77.0 335513 フブラ 78,5 111528 78,8 316925 エタ/277 フダフ 80,Z 130116 80,5 45058 81.4 37176 81,9 116175 82,3 5/1/7

£9.3

CONE DENETRATION TEST

- :-

4-22-1984 1-4-05 BORING

### LOG OF BORING No. 50 4.5-/

CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: AS DIRECTED

DATE STARTED:

9/6/84

SURFACE ELEVATION: 502,2 (WATER)

DATE COMPLETED:

9/6/34

STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" on Sampler	"N" SLOWS/ FT.	SAMPLE RECOVERY IN.
o'	BARGE					
_						
2.0'	WATER					
		}				
16.0'	FLYASH	1A	18.5-20.0	0-0-0	0	0"
		2.4	23,5-25,0	0-0-0	0	0"
		3 <i>A</i>	28.5-30,0	1-1-0	1	14"
		10	33:5-3 <i>5.</i> 5	_	-	0"
				,		
		4A	38.5-40,0	1-2-3	5	18
	HARD GRAY SILT, TRACE					
42.0	OF SAND - MOIST	5A	43,5-45,0	15-26-31	57	16"
48.5	VERY STIFF BROWN SILT, SOME SAND - WET	6A	485-500	3-6-12	18	16.
				4-8-11		16
DRILLING ME	THOD: HOLLOW STEM WATER OBSER		3.5 5 5.5	TYPE SA		
	AUGER INITIAL DEPTH:	2.01		. 🔀 A. SPL	IT SPOON	
	8089 DEPTH AFTER	<del></del>		∐ <b>B</b> .		_

LOG OF BORING No. SO 4.5-1

### CLIFTY CREEK FLYASH IMPOUNIOMENT

BORING LOCATION: AS DIRECTED

DATE STARTED:

SURFACE ELEVATION: 502.2 (WATER)

DATE COMPLETED: 9/6/84

					•	
STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	BLOWS/ FT.	SAMPLE RECOVERY IN.
59.0	MEDIUM STIFF GRAY SILT TRACE OF SAND- MOIST	ВА	58,5-60,0	4-4-5	9	16"
	(BECOMES HARD AT 63.5')			14-19-24		16"
68.5	HARD GRAY SILT SOME SAND - WET	10A	68,5-7 <i>00</i>	12-20-29	49	17"
		IIA	73,5-75,0	/1-22-30	52	18"
79.0	HARD GRAYSILT SOME CLAY, TRACE OF SAND-MOIST	12A	78.5° 800	13-21-32	53	16"
		134	83,5-85,0	11-20-29	49	18"
		14A	88.5-90.0	14-21-30	51	16"
<del></del>		15,4	90.5-92.0	13-/9-26	45	16"
92.0	BOTTOM OF BORING					
	ALALA STONE	. 7. 0.11		TYPE SA	MBI ED	
DRILLER: 7	AUGER  B.C B.C.MK.G.  WATER OBSERV  INITIAL DEPTH:	0'	- 1	⊠ A. SPI □ B.	LIT SPÕO	N
JOB NO.: 2	8089 DEPTH AFTER	. HRS			ELBY TU	3E

FORM CE-5 REV. 5/8!

## AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY

	No					AEP		_	BORING
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PROJ	ict <u>C</u>	IFTY	1 CRC	CK	F. A.	<i>D</i>			Type of Borine Rig
		F BORI							CASING USED SIZE DRILLING MUD USED
									BORING BEGUN BORING COMPLETED
WAT	ER LEV	/EL							GROUND ELEVATION REFERRED TO
TIM	£								DATUM
DAT	E								FIELD PARTY:
SAMPLE NO.	DE	MPLE PTH FEET	PEN RES	CANDA ETRAT SISTAM	RD TION NCE '6"	TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	DESCRIPTION  SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
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ENGINEER

### AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY

I OG	OF	BORING	

MPAI	NY									50 4.5-/ BORING NO DATE SHEET 2 _ OF
ROJEC	т									TYPE OF BORING RIG
LOGAT	TION O	F Boni	NG:							CASING USED SIZE DRILLING MUD USED
										BORING BEGUN BORING COMPLETED
WATER LEVEL										GROUND ELEVATION REFERRED TO
TIME			· .							OA
DATE										FIELD PARTY:
ĝ	CAL	APLE	81	ANDA	RD.	≿			7	DESCRIPTION
<b>W</b>		PTH		ETRAT		TOTAL LENGTH RECOVERY	: .	DEP	TH	
		FEET	1	SISTAR		15×5	ELEVATION			BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVE
SAMPL	FD 014	TO		.ows /	/ e <sup>16</sup>			FEE	τ.	FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
3	PROM	10	- 81	. <del>UWS /</del>	T	<u> </u>		├	_	PEGGIGATIONS IN WAISH CEVEL, HOTES ON BRISEINS ENEX, ETC.
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								٦	3	SIT- GRAY BR MOIST TO WET-TRA
5	43.5	450	15	26	3/	16			_	OF 1- Fine Sand
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6	48.5	50.0	3	6	12	16		9	$\exists$	GRARED- 100% Fine GRAIN
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JOB NO.\_

## AMERICAN ELECTRIC POWER SERVICE CORPORATION

#### LOG OF BORING

AEP CIVIL ENGINEERING LABORATORY

_									SO 4.5-1 Boring No Date Sheet _3 of 5		
}									Type of Boring Rig		
_		r Bor						_	TYPE OF BORING RIG  CASING USED SIZE ORILLING MUD USED		
					_,				BORING BEGUN BORING COMPLETED		
WATER LEVEL									GROUND ELEVATION REFERRED TO		
DAT	<u> </u>				<u> </u>				FIELD PARTY:		
ģ		484 E		ran Da	. B.O.	<b> </b>			DESCRIPTION		
PLEN	U DEPTH		STANDARD PENETRATION RESISTANCE			TOTAL LENGTH RECOVERY	ELEVATION	DEPTH IN FEET	SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES		
₹	FROM	то	BL	ows .	/ 6"	-75			FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.		
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								3	Clarer SilT - rellowish BR- moist		
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a	۔. وع	60.0	4	4	ہے ا	16"		-	Clayey Sist - GRAY BR - WET TO SATURAT Slight plasticity - TRACE OF SAND		
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									SAWD- SIT- GRAY BR MoisT- W/		
9	63.5	65.0	14	19	24	16"		4	Fine GRAIN SAND LENS - Slight		
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FORM: C5-5 REV. 5/81

## AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

WPANY .											
,										BORING NO DATE SHEET 4 OF 5	
PROJECT										TYPE OF BORING RIG	
LOCATION	N OF	Borin	1 <b>6</b> i						ļ.	CASING USED SIZE DRILLING MUD USED	
344	1			• ,						BORING BEGUN BORING COMPLETED	
WATER L	LEVE	-								GROUND ELEVATION REFERRED TO DATUS	
DATE		-				····	·····			FIELD PARTY:	
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🙎   s	SAMF	PLE		ANDA		JE 25				DESCRIPTION	
	DEP.			ETRAT		LENGT	ELEVATION	DEP		SOIL TIPE, COMP, TEXTORE, CONSTRUENCY, SAMPLER BRITING HOTES	
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NY FR	OM	то	BL	ows /	6"	~~		$ldsymbol{ldsymbol{ldsymbol{eta}}}$		PLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.	
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11 73.	. 5	75.0	11	22	30	18		۱ 4	_	Slight Reaction to HCL- mois!	
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12 78.	.5   8	80.0	13	21	32	16"		9	<u>.</u>	Same As Sample No. 11	
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FORM CENS

#### AMERICAN ELECTRIC POWER SERVICE CORPORATION

# AEP CIVIL ENGINEERING LABORATORY

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								_	TYPE OF BORING RIG RIG CASING USED DRILLING MUD USED
Loc	TION O	F BORI	184						BORING BEGUN BORING COMPLETED
WAT	ER LEV	EL	····						GROUND ELEVATION REFERRED TO
TIM			_		••-			$\neg$	DAT
DAT									FIELD PARTY:
SAMPLE NO.	DE	APLE PTH FEET	PEN	TANDA IETRA SISTAI	ARD TION NCE	OTAL NGTH COVERY	ELEVATION	DEPTH IN	DESCRIPTION  Soil type, color, texture, consistency, sampler driving notes  Blows per foot on casing, depths wash water lost, osserves
SAM	FROM		81	ows /	/ 6"	RECT		FEET	FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
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	70.3	72.0	/3_	177		76		4 -	SILERY REACTION TO ACK FOOD PHASTILL
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$\dashv$		· 		<u> </u>	ļ				·
						ļ			ENGINEER

(PAGE 1 OFG)

## LOG OF BORING No. SO 4.5-2

# CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: AS DIRECTED

DATE STARTED:

9/5/84

SURFACE ELEVATION: 502.2 (WATER)

STRATUM	DESCRIPTION OF MAT	TERIAL	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
o'	BARGE					-	
2.0'	WATER						
				· ~			
7.5'	FLYASH		IA	7.5-9.0	1-1-0	,	12"
			2A	9.0 - 10.5	0-0-0	0	0"
					0-0-0	0	10'
					2-1-2	3	10
			5A	13.5-15.2	0-0-2	2	10'
			6A	15,0-16.5	0-0-0	0	12
			7A	16.5-18.	2-0-1	,	10"
	:			ı	0-2-1		8"
	ETHOD: HOLLOW STEM AUGER 3. CB.C.MK.G.	WATER OBSERVINITIAL DEPTH:	ATIONS		TYPE SA  A. SPI	MPLER:	<u> </u>
ЈОВ НО.: 2	8089	DEPTH AFTER	. HRS		🔀 C. SH	ELBY TU	ВЕ

# LOG OF BORING No. SO 4.5-2

# LIETY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: AS DIRECTED

SURFACE ELEVATION: 502.2 (WATER)

	Control College (WA)					
STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
		9A	19.5-21.0	1-1-1	2	17"
		IOA	21.0-22.5	3-2-/	3	13"
_	LOOSE BROWN SAND, SOME	ŀ				
23.0	SILT, TRACE OF CLAY-MOIST	IIA	22,5-24.0	1-2-7	9	9"
<u> </u>		10	24.0-260		_	20"
					]	1
26.0	VERY STIFF BROWN SILT, SOME CLAY, TRACE OF SAND-MOIST	12A	26.0-27.5	5-7-13	20	14"
•	VERY STIFF BROWN & GRAY SILT					
27.5	SOME CLAY, TRACE OF SAND-MOIST	13A	27.5-29.0	9-10-17	27	16"
		144	29.0-305	6-11-15	26	12"
	VERY STIFF BROWN SILT, SOME		•			
31.0'	SAND, TRACE OF CLAY- MOIST	15A	30.5-32.0	7-14-16	30	14"
		1				
		16A	32.0-33,5	8-13-15	28	14"
33,5	MEDIUM DENSE BROWN SAND, SOME SILT, TRACE OF CLAY-1401ST	1	1	7-12-16	1	17"
DRILLING M	ETHOD: HOLLOW STEN WATER OBSERV	_ /	, .	TYPE SA	MPLER:	
	B.C B.C.MK.G. INITIAL DEPTH:2		<i>y</i>	[∑] A. SPI ☐ 8.	IT SPOOP	1
JOB NO.: 2	28089 DEPTH AFTER	. HRS		⊠ с. sн	ELBY TUE	E

# LOG OF BORING No. 50 4,5-2

# CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: AS DIRECTED

DATE STARTED: 9/5/84

SURFACE ELEVATION: 502.2 (WATER)

	5 02,2 Common			COMPLETED:		J
STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
35.0'	MEDIUM DENSE BROWN & GRAY SA TRACE SILT, TRACE OF CLAY-1401	20 st 184	35.0-365	8-11-13	24	16"
36.5'	VERY STIFF BROWN SILT, SON SAND, TRACE OF CLAY-MOIST	12	<b>\</b>	9-13-14	i .	
38.0'	VIRY STIFF GRAY SILT, SOME SAND, TRACE OF CLAY-MOIST	- 20A	3 <i>8.0-</i> 39.5	10-11-17	28	16"
39.5	STIFF GRAY SILT, SOME CL. SOME ORGANIC, TRACE OF SAND-,	115 21A	39.5-41.0	4-6-7	73	18"
41.0'	VERY STIFF BROWN & GRAY CZ SOME SILT, TRAFE OF SAND-DAM	اميد	1	5-10-13	l .	1
	(BECOME STIFF AT 42.5')	]	1	4-6-8		
		24A	44,0-45,5	4-7-8	15	16"
	(BECOMES WERY STIFF AT 45	.s') 25A	45,5-47.0	5-8-10	18	17"
47.0	STIFF BROWN & GRAY SILT SOME SAND, TRACE OF CLAY-W.	26A	47.0-48.	3-4-8	12	16"
DRILLER: /	WATER OBSI AUGER  INITIAL DEPTH:  COMPLETION DEP  1.8089  DEPTH AFTER	Z.0'		TYPÉ SA A. SPU B. C. SHU	LIT SPOOF	

(PAGA 4 01 6)

# LOG OF BORING No. SO 4.5-2

# CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: AS DIRECTED

DATE STARTED: 9/5-/84

SURFACE ELEVATION: 502.2 (WATER)

					1/3/	07-
STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
	(BECOMES VERY STIRE AT 48,5')	27A	48,5.500	12-13-17	30	15"
	(BECOMES HARD AT 50.0')			15-16-16		
	(BECOMES STIRE ATSIS')			3-5-8		
	(BECOMES VERY STUF AT 63.0')			8-9-12		
<u></u>	(BECOMES HARD AT 54.5')	31A	54,5-56,0	19-19-35	54	18"
56.0	VERY STIFF GRAY SILT, TRACE OF CLAY- DAMP	324	560-575	9-10-19	29	18"
	(BECOMES HARD AT 57.5')	33.4	57,5-59,0	19-27-29	56	18"
		34.A	<i>59.0-60.</i> 5	27-21-20	41	12"
		35A	60.5-620	11-13-19	32	16"
DRILLING M	ETHOD: HOLLOW STEM WATER OBSERV	•		TYPE SA	_	
DRILLER:	B.CB.C.MK.G. COMPLETION DEPTHE	2.0		8. □ 8.		
10B NO:: 2	8089 DEPTH AFTER	HRS		∑ C. SHE	LBY TUB	ε <b>.</b>

(PAGE 5 016)

## LOG OF BORING No. SO 4.5-2

## CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: AS DIRECTED

DATE STARTED: 9/5-/84

SURFACE ELEVATION: 502.2 (WATER)

	ODZIZ CWANZ)					<u> </u>
STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. å Type	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
		ľ				
		36A	62.0-63,5	11-19-40	59	16"
		3 <i>74</i>	L3.5-65,0	17-21-40	61	12"
	VERY DENSE GRAY SAND					
65.0	AND SILT, TRACE OF CLAY-MOIST	38,4	65.0-66.5	16-23-30	53	16"
		i				
			Ì	•		
		39A	665-68.0	22-25-27	52	16"
		1			ļ.	İ
		1		<b>10</b> 5. 5.		14"
		40A	68,0-69,5	19-21-23	44	14
					ļ	
		411	10-710	17 18 22		16"
		712	69,3-11,0	17-19-22	41	-
		424	715-730	15-20-19	3 9	14
		7 2271	77,3-73,0	75 25 11	-	
		1		ļ		
		43.4	730-745	19-24-26	50	15"
		<del>                                     </del>				
					ļ	
		44A	745-760	18-23-24	47	16"
		<u> </u>				
	<u> </u>		<u>L, , ,</u>			<u> </u>
DRILLING ME	THOO: HOLLOW STEM WATER OBSERVA	_		TYPE SA		
	AUGSE INITIAL DEPTH:	ر 2.0	·····	X A. SPL	IT SPOON	
				☐ B.		
JUB NO.: 2	8089 DEPTH AFTER	HR\$.'	············	C. SHE	LBT IUB	

## LOG OF BORING No. 50 4.5-2

# CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: AS DIRECTED

DATE STARTED: 9/5/84

SURFACE ELEVATION: 502.2 (WATER)

				OMPCETED:	,, -,	<del></del>
STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON Sampler	#N## BLOWS/ FT.	SAMPLE RECOVERY IN.
·	(BECOMES MEDIUM DENSE AT 76.0)	45A	76,0-77,5	8-12-14	26	18"
	(BECOMES DENSE AT 77.5')	46A	77,5-79,0	12-16-19	35	18"
	(GECOMES MEDIUM DENSEAT 79.0')	47A	790-80s	10-15-11	26	16"
	(BECOMES DENSE AT BO.5')	48A	805-820	9-13-17	30	17"
82.0	BOTTOM OF BORING					
	; ;					
						·
DRILLER: /	WATER OBSERVE INITIAL DEPTH:	.0'	1	TYPE SA	LIT SPOOP	•
10B NO: 2	8089 DEPTH AFTER	HRS		<b>∑</b> c. sh	ELBY TUE	E

ORM CE-5

#### AMERICAN ELECTRIC POWER SERVICE CORPORATION

# AEP CIVIL ENGINEERING LABORATORY

୍ଦ୍ର ।	No						L	og o	F BORING
		IKE			•				504.5-2 Boring No Date Sheet _/_ of _5_
	ANY	<u> </u>	1 6 8	ceK	F		· · · · · · · · · · · · · · · · · · ·		Type of Borine Rig
`				CEN				_	CASING USED SIZE DRILLING MUD USED
Loc	ATION O	F BORE	NG:						BORING BEGUN BORING COMPLETED
WAT	ER LEV	/EL							GROUND ELEVATION REFERRED TO
TIM									Datus
DAT									FIELD PARTY:
			7					_	·
Ž	SAI	APLE	L	TANDA		ן אַבּגַר			DESCRIPTION
w		PTH		IETRA		<b> </b> ₹5₹	ELEVATION	DEPT	Self itre, Com, issiste, consistenci, samples salvine notes
, <u>₹</u>	IN	FEET		SISTA		TOT LENG RECOV		FEE	BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED
SAMPLE	FROM	TO	8L	.ows /	<b>6</b> "	1, 75			FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
)							Ì		
·	l			<u> </u>				. م ا	Boring TAKEN by BOWSER-MORNER  7.5 FT OF WATER
	-							֓֓֞֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֡֓֓֓֓֡֓	<u> </u>
				ļ	<u> </u>			<u>ا</u> ا	7.5 FT OF WATER
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,	ار ہوا	9.0		,	0	/2"			7 7 5 21 0 5 11 0 5 1
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2	9.0	10.5	0	0	٥			۰.	
	ľ					7		]	<b></b>
13	10.5	12.0	0	0	0	10		١.,	<u> </u>
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۱ <u>.</u>								] ~ ·	
¥	120	13.5	2	1	2	10"		3 -	<del></del>
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5	13.5	15.0	0	0	2	10 "	l	4 -	<u></u>
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از ا	,	14.5	0	0	0	13"			<del></del>
<u> </u>	13.0	/ <b>@/ 3</b>				100		6 -	<b>_</b>
17	16.5	18.00	2	0	,	10"		l _ '	-
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			'			1			
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	18.0	19.5	0	2	1	8		۔ و	
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	19.5	21.0	1	1	1	11"		-0	
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J	<u> </u>		ļ			<del> </del>		<b>!</b> .	
			I .	Ι .				l .	

FORM CE-5 REV. 5/61,

# AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY

1	OG.	OF	BO	91	MG
_	uu	VF	80	ĸп	

	MY						·	_		BORING NO DATE SHEET 2 OF 5
	CT								١.	CASING USED SIZE DRILLING MUD USED
Loca	ATION O	F BORII	<b>16</b> i						l	BORING BEGUN BORING COMPLETED
WAT	ER LEV	EL							]	GROUND ELEVATION REFERRED TO
TIM	£									DATUM
DAT	<u> </u>			· , · · · · · ·					J	FIELD PARTY:
SAMPLE NO.	DE	APLE PTH FEET	PER RE	TANDA NETRAT SISTAP LOWS /	RD FION NGE ' 6"	TOTAL LENGTH RECOVERY	ELEVATION	DEF II FE		DESCRIPTION  SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
_	<del></del>	-		├		<u> </u>		z	<u> </u>	
10	21,0	22.5	3	2	1	13"		Ļ.,		FLY ASH TOP OF SAMPLE
								] '	<u> </u>	CINYCY SIT - GRAY GREEN - WET . TRACE
1/_	22.5	24.0		2	2	9"		_2	? -	os sand ML
1								. ~		CIRYLY SANd- BR. WET - 100% Fine GRAIN
-				+	<del> </del>			3	3 —	Quartz SC
· .	מ.טנ	26,0	ļ			20"		۱.	_ =	
	7/.5							1 1	Έ	3" Shelby Tube
		Ļ		ļ	<u> </u>			٠ ا	s —	
					l .			'	=	
(	260	27.5	5	2	/3	1400		-	<u>,  </u>	SANDY SIT- BR MOIST TO WET- SlighT Plasticity - Trace of erg. mat. ML
					l .			l	=	Plasticity - Trace of org. mat. ML
								1 7	<u> </u>	Sandy SiT - GRAY + BR. MOIST TO WET
13	275	22,0	9	10	17	16"		▎.		ORG. MAT. ML
								l `	Έ	<u> </u>
14	29,0	30,5	6	11	15	12"		<u> </u>	2=	
				<u> </u>	ļ	<u> </u>		3 (		Sandy Sit - Be moist - slight Plasticity
۱		<b> </b>		1	١.,			<b> </b>	<b>&gt;</b> _	Same AS SAMPLE NO. 14
/5	30.5	32.0	7	14	16	14"		<b>∤</b> ≀	ـــا	
1/2	80.0	33.5	9	13	15	14"		<u> </u>	<u> </u>	Same as Sample No. 14
7.0		0.0,0		1		1		1 ²	_	DANCE AS SHIPPING TO THE
								] ,	Ξ.	
								<del>  -                                   </del>	<b>5</b>	SAND - BR. MOIST 100% Fine GRAIN
17	33.5	35.0	7	/2	16	17*		4	, <u> </u>	QUARTZ - TRACE OF SILT
۰,	2	36.5	o.	//	13	16"			=	SC-SP
-		-1643	<u>a</u>	+-//	13	/-		13	- K	Clayey Silf- GRAY BR. MoisT- TRACE
								_	Ξ	OF SAND ML
								"	´ =	
19	36,5	38.0	9	/3	14	18"		7	<del>-</del>	CIAY SIIT - BR - MOIST - TRACE OF SAND
	• • •			<b> </b>	ا	,,,,			Ξ	mL mL
40	38.0	39.5	10	11	17	16"		<b>\</b> *		Clayer SilT - GRAY BR. MOIST - TRACE
									>=	OF ORG MAT. TRACE OF SAND ML
								]	Έ.	
						<del> </del>		40	, _	
								[		
<b> </b>		· · ·		<del>                                     </del>		<del>                                     </del>		1		
										ENGINEER
_						<u> </u>				

PEV. 5/81

# AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY

701									'	504.5-2
/	ANY			<u>.</u>						BORING NO DATE SHEET 3 OF 5
	ECT									CASING USED SIZE DRILLING MUD USED
L∞	ATION O	F BORI	M <b>G</b> :							BORING BEGUN BORING COMPLETED
WAT	ER LEV	/EL	=				<del>_</del>			GROUND ELEVATION REFERRED TO
TIM	E						<u>.</u>			DATU
DAT	ŧ .									FIELD PARTY:
ź	SAI	MPLE	S	TANDA	ŔD	_ <u>&gt;</u>		Γ.		DESCRIPTION
W		PTH		NET RA		TOTAL LENGTH RECOVERY		DEP.	ТН	
3	_	FEET	RE	SISTA	NCE	528	ELEVATION	FEE		BLOWS PER FOOT ON CABING, DEPTHS WASH WATER LOST, OBSERVED
₹	FROM	TO	BI	LOWS	/ 6"			" " "	• •	FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
					T		_			
21	39.5	41.0	4	6	7	187		40	Ż	Clayer SilT - GRAY Be - MOIST - TRACE
								'	_	ORG. MAT. TRACE OF SAND ML
<u> </u>			ļ	ļ	<u> </u>	<del>                                     </del>		١,	1	
				١.,	١.٣					Silty Clay- BR moist - how to med
22	41.0	42.5	5_	10	/3	18"		2		PINSTICITY- CL
22	42.5	440	4	1	8	18	-		41	Clayey SilT - BR - WET TO SATURATED.
		1 110		1	1.			1 3	-	Tame or Sand. Mh
L	<u> </u>				<u> </u>				Ξ	
									Ľ	Same as Sample No. 23
24	44,0	45,5	4	7	8	16"		5	-	<u> </u>
	l		_			J			Ţ	
<u>عر</u>	45.5	47.0	5	8	10	17"		6	<b>2</b>	SAME AS SAMPLE NO. 23
			ļ		1.			ľ	7	
-			<u> </u>		•	7	: .	7	7	Same as sample No. 22
24	47.0	48.5	3	4	8	16"			٠,	· · · · · · · · · · · · · · · · · · ·
									ļ	SANdy SiT - BR. WET TO SATURATED
27	485	50.0	/2	13	17	15"		و ا	4	100 % Fine GRAIN ML
!								·	7	·
			<del> </del>		+-	<del> </del>		<i>5</i> . 0	$\exists$	
- 28 - 28	50.0	51.5	مع ر	16	16	18"			1	Same AS Sample NO. 27
<u> </u>	30,0	5//3	/3	. 76	16	100-		1	٦	Same as Sample HO, XT
29	51.5	53.0	3	5	8	16 "		_	ᅺ	SAME AS SAMPLE NO. 27 (SATURATED)
								2	Ε	
	ļ							3	ᅼ	
									ᅿ	Same AS Sample No. 27
30	53,0	54.5	8	9	/2	164		4	4	<u> </u>
2,	C11. E	540	10	19	35	18"	<u> </u>	$\rightarrow$	3	Same ne Samala de 27 (6-1 0a)
1	37.3	<u> </u>	1	1	<u> </u>	70		5	뒥	Same As Sample No. 27 (GRAY BR)
			<u> </u>						月	
:								<del></del> څ	$\exists$	SiTTY Clay- GRAY MoisT - Low To med.
32	540	57.5	9	10	19	18"	·	7	ᅼ	QlASTICITY - STRONG REACTION TO NOL-
									ュ	<u> </u>
33	575	59,0	19	27	29	18"		8	긬	SIIT - GRAY BEMOIST TO WET- TRACE OF SAND - MODERATE REACTION TO HILL - ML
)					1				7	sand-moderate Kenetion to Heb - ML
								9 .	┪	Same as Sample No. 33
34	59,0	40.5	27	21	20	/2*		0	寸	777
								J.	丁	
<u> </u>				ļ					ļ	
:										Engineen
		·			L	1				ENGINEER

FORM CE-5 REV. 5/81

# AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY

\										
OMP	ANY									SO 4.5 - 2 DATE SHEET _ 4 OF _ 5
Proji	ECT									TYPE OF BORING RIG
Loc	ATION O	r Boni	HG:							CASING USED SIZE DRILLING MUD USED
<u> </u>						_				BORING BEGUN BORING COMPLETED
	ER LEV	EL								GROUND ELEVATION REFERRED TO
DAT										FIELD PARTY:
UAI						,				PIECO PARTY:
ŝ	SAN	APLE	S	TANDA	RD	≿	•			DESCRIPTION
W	DE	PTH	PEN	IET RAT	TION	<b> ₹</b> 53	EI EVATION	DEPT	TH	SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER DRIVING NOTES
츁	IN I	FEET	RE	SISTA	NCE		CTE ANI IOM	FEE		BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVE
3	FROM	TO	81	.ows /	′ 6" ·	. ⊃ <b>£</b>	ELEVATION			FLUCTUATIONS IN WATER LEVEL, NOTES ON BRILLING EASE, ETC.
35	60, 5	62.0		13	19	16"		60	_	
:	-		•					1	_	SiT- GRAY & moisT- TRACE OF SAND
				ļ	<u> </u>	<u> </u>			_	MODERATE REACTION TO NEL ML
۵.	<b> </b> ,			١.,	١		-		_	
36	620	<i>63.</i> 5	//:	19	40	160		2	7	Clayer SilT - GRAY BE-mais T- Low Plasticity-moderate Reaction To HCL
27	63.5	18 A	177	21	40	/2"		1	-	· / / / / / / / / / / / / / / / / / / /
<u> </u>	3,3	65,0	1		1 70	1.		<del>  -3</del> ,	<b>y</b> -	SIT- GRAY & mais T. STRONG REACTION
	ŀ			] .			-	١ .	_	TO HEL- TRACE OF SAND. MI
			`					1 •	_	
J8	63,0	66,5	16	23	30	16"			Ξ	· · <u> </u>
	ŀ	1.1				1			7	Sit - GRAYEMOIST. Slight PLASTICITY
79	66.5	68.0	22	25	27	16"				STRONG REACTION TO HEL - W/ Fine GRAIN SAND LEWS QUARTE
									_	
	<u> </u>			<del> </del>	-	<del> </del> -		1 7	>	ML
	68.0			انما	-			l	_	SAMERS AS SAMPLE NO. 38
40	68.0	64,3	17	al_	23	74		-	7	Same A > Sample NO 38
u i	69.5	760	17	19	22	1600		] _	_	Dame we sumple for se
								-	5	SAME AS SAMPLE NO 38
	ļ							70	_	
	1							] ′ ັ	_	
42	71.5	73.0	15	20	19	14"			_	
_									Z	SIT- GRAY Semoist To wet- Slight
43	73.0	74.5	19	24	26	15"		- 2	-	PLASTICITY : Slight Repetion To HeL
								<u> </u>	<u>,                                    </u>	(m)
	<del> </del>	<u> </u>	<del> </del>	<b></b>		+		3	_	Same as Sample NO 42
رين	74.5	74.0	18	23	24	16"	-		٦	
T								1	Z	Same 45 Sample NO. 42 (MoisT)
45	76.0	77.5	8	12	14	18"			Ξ	
								<b>├</b> ~	1	
						<u> </u>		6	2	Chayey Silt - Gony Bemaist . Slight
								•	7	Placticity. Slight REMUTION TO HEL
46	77.5	79.0	12	16	19	18"	-	7	5	M
	L.C	م به	,_	ا ـ ا	١.,	, , ,,			11	Same As Sample NO. 45
47	790	80.5	10	15	11	16"	7	8	$\exists$	
)						' '			7	SIT - GRAYER MOIST. Slight Plasticity
<u> </u>	<u> </u>	<u> </u>	<del> </del>	<u> </u>		<del>   </del>		9	ᆿ	Slight Reaction TO HCL
L				L				_	7	ML ML
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										<b>6</b>

TORM CE-5

# AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY

MP	AHY								BORING NO DATE SHEET 5 OF 5
`	ст								TYPE OF BORING RIG RIG SIZE DRILLING MUD USED
Loc	TION O	F BORI	N 6 1				:		BORING BEGUN BORING COMPLETED
WAT	ER LEV	EL							GROUND ELEVATION REFERRED TO
TIM			· 	-		<u> </u>			DATUM
DATE									FIELD PARTY:
ĝ	SAN	APLE	1	ANDA		J±&	·. ·	]	DESCRIPTION
W		PTH :		ETRAT SISTAN	_	TOTAL LENGTH RECOVERY	ELEVATION	DEPT	Sole fire, come, fexture, consistenci, sampler briving hores
SAMPLE						500		FEET	
<u> </u>	FROM	10	8L	ows /	<u>•                                      </u>			<u> </u>	FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
						]		80-	
	-							180-	Sill-GRAY - mois T TO WET- Sligh T REACTION TO HOL- TRACE OF ORG. MAT.
48	80,5	82.0	7	13	17	17"		<u>ا</u> ا	REACTION TO HOL- TRACE OF ORG. MAT.
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## LOG OF BORING No. 505-/

# CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: AS DIRECTED

DATE STARTED: 9/21/84

SURFACE ELEVATION: 502.2' (WATER)

DATE COMPLETED: 9/21/84

STRATUM	DESCRIPTION OF MAT	TERIAL -	SAMPLE NO. & TYPE	SAMPI DEPT	-	BLOWS PER 6" ON SAMPLER	BLOWS/ FT.	SAMPLE RECOVERY IN.
	. •							
0'	BARGE							
2.0'	WATER				-			
4,5'	FLYASH			•				
				•				
80.0'	BOTTOM OF BOX	e/NG						
	NOTE: CONE PE	NETENTION	TE.	ST.	BOE	ING		
•								
					.			1
		•						
		·						
RILLING M	ETHOD: HOLLOW STEM	WATER OBSERV			1	TYPE	SAMPLER:	_1
RILLER:	B.CK.G.	COMPLETION DEPTH:2	. 0' 2.0'	,		☐ A. ☐ 8.	SPLIT SPOO	N
		DEPTH AFTER				c.	SHELBY TU	ΒE

BOWSER-MORNER TESTING LABORATORIES, INC.

Woodward-Clyde		Cone Resistance	
Consultants 😂	VS Depth		— FILE NO. 84C309Z
CHICAGO	COMPUTED JE	CHECKED FW)	DATE 9-30-84 PAGE 1 OF 2 PAGE
	Resistance		, Lesistance Boring
Depth fo	16/1/2	Ochth, fr	16/ JE 2 50-5-1
10.0	16729	27.8	120822
_	0	28,6	134765
10.3	0	29.0	0 0
11.0	26023	30.0	27235 1309 psf
12,0	9294	31,0	0794
13.0	37/7	32.0	9294 367 psf
140	27417	_33.0	5576 107psf
14.5	78493	34,0	20446
15,3	17658	34,4	¥8328
16.0	5576	34.8	21376
16.5	3717	35,0	37964
17.0	23 235	34,0	20446
17.5	10223	37,0	188590
18.0	23735	37,3	111528
18.5	14870	38,0	278820
19,0	4647	38,5	102234
19.5	5576	39.0	97587
20,0	13941	40.0	88293
20,7	13941	40.25	83293
20.8	288114	41.25	/8 <i>58</i> 8
21.4	52046	42.25	46470
22./	24164	43.25	65058
22.2	53 905	44,25	97587
22,4	44611	45,0	46470
2.2.8	64128	45,5	60411
23.2	28814	46.0	144057
23.8	64128	44.5	264468
24,2	55764	47.0	139410
24.8	1. 12011	47,5	167292
26.0 prep	37/7	48.0	223056
27.0	20446	48,5	144657
27.5	53905	49.0	162645
			Table 2

- :-

Table 2

Woodward-Clyde Subject Table I cont. Cone
Consultants & Resistance vs Depth COMPUTED JUST CHECKED FW)

PROJECT BOWET-MOTRET FILE NO. 84C309Z DATE 9-30-84 PAGE Z OF Z PAGE

depth, f	Resistance 16/At	depta. He	Resistance 16/ft
49,5	144057	76.75	78999
50.0	88 293	77.75	218409
51.2	185880	78.75	83646
52,2	171939	79.3	74352
53.2	120822	79.6	27887
53.7	23735	80,6	46470
54,2	116175	80.9	35529
55.0	92940		
56.0	107234		
57.0	227703		
58.0	116175		
59.0	46470		
60.0	55764		
25.03	37176		
01.25	37176		
62.25	88293		
63.25	92940		
4.25	55764		
65.5	125469		•
66,5	97587		
675	37/76		
68.5	41823		:
69.5	65058		
69.9	157998		
70.9	78 999		
71,9	88293		
72.9	66411		
73,9	97587		
74,3	60411		
74.75	2325		•
75,75	82646		

CLIFY CREEK POWER RANT BOWSER-MORNER CONE PENETRATION TEST

HCE SHC3088

Cons Tip Absilaas: 16/12

#### 50 5.5-1 LOG OF BORING No.

# CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: AS DIRECTED

DATE STARTED:

SURFACE ELEVATION: 502.2 (WATER)

					•	
STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	TN" BLOWS/ FT.	SAMPLE RECOVERY IN.
0	BARGE	ļ				
,			·			
2.0	WATER	ļ				
			0 - 10 0	1-1-1	9	18"
8.0	FIYASH	IA	8,5-10,0	7-7-7		10
		2.4	13.5-15.0	0-0-0	0	18*
			, 5, 5			
	HARD BROWN SILT, TRACE SAND					
16.5	TRACE OF CLAY - DAMP	3 <i>A</i>	18,5-20.0	14-23-31	54	18"
	VERY STIFF BROWN SILT, SUME SAND, TRACE OF CLAY - MUIST (1"706" CLAY LAYERS)	4 A	23,5-25.0	7-14-16	30	14"
	(BECOMES HARD AT 28.5')			19-28-36		
32.0	HARD BEDWN SILT, SOME SAND, TRACE CLAY, TRACE OF ROCK FRAGMENTS - PAMP			50/3"		
-×.0	L'ENGLIENTS - PASSOF				-	
		74	38.5-39.0	100/4"	100/4	4"
DRILLING M	ETHOD: HOLLOW STEAT WATER OBSERV			TYPE SA		
DRILLER: /	B. C B. C. A. R. G. COMPLETION DEPTH:	2.0		<b>⊠</b> A. SPI <b>⊠</b> B. <i>NX</i>	WL RO	CK GORE
JOB NO.: 2	2.8089 DEPTH AFTER	HRS		C. SH	ELBY TUE	3E 

# LOG OF BORING No. SO 5.5-/

# CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: AS DIRECTED

DATE STARTED:

7/1/84

SURFACE ELEVATION: 502,2 (WA)TER)

STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	GLOWS PER 6" ON SAMPLER	"N" SLOWS/ FT.	SAMPLE RECOVERY IN.
41.0'	WEATHERED LIMESTONE SHALE	84	43,5-44.	47-53/4"	10%	· 8"
	(LOST WATER REFURN AT 50.5')	·				
55.5	BOTTOM OF BORING	1B	45.5-55.5	-	_	1190
					r	
					-	<del> </del>
DRILLER: /	ETHOD: HOLLOW STEM WATER OBSER AUGER INITIAL DEPTH:	2 . o'		A. SP		
JOB NO.: 2	28089 DEPTH AFTER	HRS		C. SH	ELBY TU	3E

FORM CE-5

#### AMERICAN ELECTRIC POWER SERVICE CORPORATION

# AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

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- PROJ	ECT C	LETY	ICR	CCK	رسر	AD	·			BORING NO DATE SHEET _/ OF STEET OF
	ATION O								1	CASING USED SIZE DRILLING MUD USED
										BORING BEGUN BORING COMPLETED
	TER LEV	EL							ļ	GROUND ELEVATION REFERRED TO
TIN									ł	DATE -
DAT	1								J	FIELD PARTY:
ģ	CAN	PLE	6.	TANDA	BD .	<b></b>	Ĭ	1		DESCRIPTION
"	DE		_	ETRA	-	TOTAL LENGTH RECOVERY		DEP	TH	
		EET	RE	SISTA	NCE	528	ELEVATION	┨_"	N	BLOWS PER FOOT ON CASING, DEPTHS WASH WATER LOST, OBSERVED
SAMPL	FROM	TO	81	.ows /	/ <b>a</b> "	E SE			ET	FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
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2	15.5	15.0	٥	۵	٥	18"		١	-	17
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/3	18.5	20.0	14	23	31	18"		9		100% V-Fine GRAIN
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FC W CE-5

#### AMERICAN ELECTRIC POWER SERVICE CORPORATION

# AEP CIVIL ENGINEERING LABORATORY

	or Boni							TYPE OF BORING RIG  CASING USED SIZE DRILLING MUD USED BORING BEGUN BORING COMPLETED
ER L	VEL							GROUND ELEVATION REFERRED TO
E								0
E								FIELD PARTY:
1								DESCRIPTION
IN	MPLE EPTH FEET	PEN RES	TANDA ETRAT SISTAP OWS /	TION NCE	TOTAL LENGTH RECOVER	ELEVATION	DEPTH IN FEET	SOIL TYPE, COLOR, TEXTURE, CONSISTENCY, SAMPLER-DRIVING NOT
PRO	M TO	BL	OWS /	<u> </u>	<del>-</del>			Programma in water programma on smilling that, etc.
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23.	25.0	-/	14	16	17		3 –	Slight Plasticity.
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							8	Sitty Clay - LT. BR moist. Steoma
28.5	30.0	19	28	36	18"			Sinty Clay - LT. BR moist. Steams Reaction to Hel- TRACE OF Lime
								STONE FRAG.
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),	38.9	100/			4"			MOIST- POORIN GRAded- 1"max Size
- 13 B. S	38.7	/.4_					9 –	Sand = MORRY CRAded - 1"MAX SIZO
								GANGE GREATET FIMESTONE
$\overline{}$						T	40-	

FORM CE-5 REV. 5/81

### AMERICAN ELECTRIC POWER SERVICE CORPORATION

# AEP CIVIL ENGINEERING LABORATORY

Jos.	No			<del></del> .			L	JG U	· BORING
HP	YHA						<u></u>		SO 5. 5- 1 BORING NO DATE SHEET 3 OF 3
PROJ	ECT								TYPE OF BORING RIG
Loc	ATION O	F Bori	M4.						CASING USED SIZE DRILLING MUD USED
									BORING BEGUN BORING COMPLETED
WA	TER LEY	/EL							GROUND ELEVATION REFERRED TO
TIM	E								DATU
DAT	Έ				· ·	109 10 10 10			FIELD PARTY:
_	T		T				ELEVATION		DESCRIPTION
g		MPLE		TANDA IETRAT	ND NON	12.5		DEPT	DESCRIPTION
<b>"</b>		PTH FEET	1	SISTAN	106	1500	ELEVATION	IN	Sair life, com, lexione, consistency, samples skilling house
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₹,	FROM	TO.	BL	ows /	6"	_~			FLUCTUATIONS IN WATER LEVEL, NOTES ON DRILLING EASE, ETC.
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			11.03	57.		8			GRAY CALCARCOUS Shale + hime STONE
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# LOG OF BORING No. SO 6-1

# CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: AS DIRECTED

SURFACE ELEVATION: 502,2 (WATER) DATE COMPLETED: 9/8/84

STRATUM	DESCRIPTION OF MATERIAL	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
0'	BARGA					
2.0	WATER			• No.		
6.0'	FLYASH	IA	8,5-10.0	0-1-0	,	14"
		ZA	13.5-15.0	0-1-1	2	16"
		3 <i>A</i>	18.5-20.	0-1-0	1	16"
		4.4	23.5.25.0	0-0-1	1	15"
•						
		5A	28.5-30,0	2-3-2	5	18"
	,					
32.0'	TRACE OF CLAY - MOIST	6A	33.2-320	3-4-9	13	16"
30 -	STIFF GRAY SILT, TRACE SAND	24	38500	1.50	/2	12"
39.0	TRACE OF CLAY - MOIST	77	70,3-70,	4-5-8	1/5	
		BA	43.5-45.0	5-7-11		18'
DRILLING M	ETHOD: HOLLOW STEM WATER OBSERV			TYPE SA		
ı	B.C IC. G B.C. M, COMPLETION DEPTH:	2.0	·y	Ø A. SPI	LIT SPOO	N
l			ŀ	∐ В.	ELBY TUI	3E
705 NU.:	28089 DEPTH AFTER	. HRS				DVED

# LOG OF BORING No. 50 6-1

## CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: AS DIRECTED

DATE STARTED: 9/8

9/8/84

SURFACE ELEVATION: 502.2 (WATER)

DATE COMPLETED: 9/8/84

		_					7 - 7 - 7				
STRATUM	DESCRIPTION OF MA-	TERIAL -	SAMPLE HO. & Type	SAMPLI DEPTH	ELOWS 6" OF SAMPL		"N" BLOWS/ FT.	SAMPLE RECOVERY IN.			
	(BECOMES HARD	AT 48.5')	9A	48.5-	50.0 9-13	-/8	3/	16"			
			10A	53,5-5	508-14	1-19	33	18"			
	BECOMES NERY ST	(FF AT 58.5')	II.A	58,5-6	0,0 6-9	-/3	22	18"			
63.5	VERY STIFF GRAY SAND, TRACE CLA ORGANICS - IM D	SILT, TRAGE Y, TRAGE IST	JZA	63.5-	15,07-8	- /2	20	18"			
•	VERY STIFF GRAY	CLAY WITH		68.5-7	0.0 19-1	1-8	19	18*			
			4A	73.5-7	150 17-1	3-16	29	4"			
			15A	78.5·	30.0 /1-1	4-17	31	8"			
81.5	AUGER REFUSAL	L CAPPARENT RO	<b>(</b> *)			_					
DRILLER: /	ETHOD: HOLLOW STEM AUGER B. C K. G. · B. C.M.	WATER OBSERV INITIAL DEPTH:2 COMPLETION DEPTH: #	2.0			A. SPL	MPLER:	N .			

\* ROCK IN DRIVE SHOE

BOWSER-MORNER TESTING LABORATORIES, INC.

# LOG OF BORING No. SO6-2

### CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: AS DIRECTED

DATE STARTED: 9/20/84

SURFACE ELEVATION: 502.2 (WATER)

					<u> </u>	
STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
		l.				
	·	1	1	ONE PEN		
0'	BARGE	l	7	EST BOR	WG !	70
			2	8'		
2.0	WATER					
7.0	FLYASH					
	NEATHERED SHALE LIMESTONS	'		100/,	1001	<b></b>
28.0	WEATHERED SHALE/LIMISTONS FRAGMENTS	IA	28-28,5	//"	11.	/"
		1	]			
				1	1	-"
		ZA	34-34.5	100/3"	100/3	3
		1				
39.0	GRAY SHALE	3A	39-39.5	100/4"	100/4.	4"
				,		
		4A	44-44.5	100/3"	100%.	3"
		+			1	<u> </u>
ı.					١.,	
50.0	LIMESTONE SHALE	SA	49-49.5	100/1"	100%	0"
1 .			'	· .		
		10	15		-	110
60.0	BOTTOM OF BORING	110	50-60		ļ	110
DRILLING M	ETHOD: HOLLOW STEM WATER OBSERV	ATIONS		TYPE SA	MPLER:	
	AUGEIS INITIAL DEPTH:		• 1	A. SPI	LIT SPOOI	, N
DRILLER:	B. C K. G. COMPLETION DEPTH:			B.NX	W ROC	K CORE
JOB NO.: 3	28089 DEPTH AFTER	HRS		C. SH	ELBY TUE	3E

Cone Resistance Woodward-Clyde Suspect Table 3 PROJECT BOUSET - MOTHER Consultants 4 FILE No. 84C309Z Desth CHECKED FWJ COMPUTED UNT DATE 9-30-84 PAGE / OF / PAGES CHICAGO Resistance Resistance depth, ft depth, ft 324580 ZG 10 10.5 3345 **ミア/760** *32343* 26,25 10.75 345750 26.5 11.0 17286 27,0 ナチられつ 11.25 20075 223050 11.5 13383 27,5 11,8 23420 28,0 557070 12,3 9479 13.0 ZZ30 13.5 557 Boring 50-6-2 13,8 557 11152 14,0 16724 14,5 15.0 3345 7806 15,5 0 15.75 16,0 0 16.5 2230 31600 16.8 24165 17.5 18,5 13010 19.0 10223 19.5 15800 14870 20,1 9795 21.0 4460 ZZ.0 13940 22,25 4460 23.0 27325 23.75 24,5 4460 46840 25 25.5 66916

25.75

255580

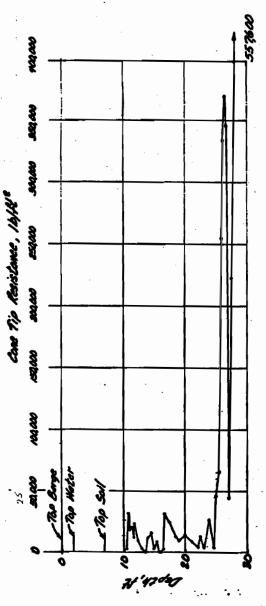
**强新工作 "我们**" "一个一个一个

Table 3

CLIFTY CREEK POWER PLANT BOWSER-NORWER CONE PENETRATION TEST

Wee Syksor?

Fig. S



787-02-P BORING : 50-6-2

(3د) س۲

# LOG OF BORING No. 50 6,5-/

# CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: AS DIRECTED

DATE STARTED: 9/9/84

SURFACE ELEVATION: 502.2 (WATER)

STRATUM	DESCRIPTION OF MA SOIL CLASSIFICATION SYSTEM:	TERIAL	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	**N** BLOWS/ FT.	SAMPLE RECOVERY IN.
0'	BARGE						
2.0'	WATER						
4.5'	FLYASH		IA	4.5-6.0	0-0-0	0	18"
		,	2 <i>A</i>	6.0-7.5	0-0-0	0	18"
			10	7.5-9.5	1		0"
····			34	9.5-11.0	0-0-0	0	18"
<u> </u>			4 A	11.0-12,5	0-0-0	0	18"
		. •	5A	12.5-14.0	0-0-0	0	18"
			6A	14.0-15.5	0-1-1	2	16"
			1		0-0-1	1	18"
	AUGER B. CFBCM - K. G.	WATER OBSERVA INITIAL DEPTH:	TIONS	,	TYPE SA  A. SPL	.IT SP001	٠.
JOB NO.: 2	8089	DEPTH AFTER	HRS		Z C. SHE		

# LOG OF BORING No. 50 6.5-1

# CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: AS DIRECTED

SURFACE ELEVATION: 502.2 (WATER)

DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	"N" BLOWS/ FT.	SAMPLE RECOVERY IN.
•					
	8A	17.0-18,5	0-1-1	2	18"
•					
	20	18,5-20,5	-		2/"
HARD BROWN SILT, SOME SAND			٠.		
TRACE OF CLAY- MOIST		20.5.22.0	15-17-16	33	16"
			-		
	3 <i>c</i>	22,0-24,0			/3"
VERY SOFT BROWN SILT					
AND SAND -WET	IOA	240-25.5	3-1-2	3	16"
,					
		25,5- 27,0	3-4-4	8	18"
(BECOMES SOFT AT 27.0')				5	14"
(BECOMES MEDIUM STIFF AT 28.5')					18"
	13A	28,5-30,0	1-2-3		10
VIERY STIFF BROWN CLAY, SOME SILT, TRACE OF SAND-MOIST	14A	30.0-315	6-8-10	18	17"
BECOMES HARD AT 31,5')			İ		16"
DRILLER: B.C B.C. 14 K. S.			X A. SPLIT SPOON		
					_
	HARD BROWN SILT, SOME SAND TRACE OF CLAY- MOIST  VERY SOFT BROWN SILT, AND SAND - WET  (BECOMES MEDIUM STIFF AT 25.5')  (BECOMES SOFT AT 27.0')  (BECOMES MEDIUM STIFF AT 28.5')  VERY STIFF BROWN CLAY, SOME SILT, TRACE OF SAND-MOIST  (BECOMES HARD AT 31.5')  ETHOD: HOLLOW STEM AUGER  B.CB.C.MK. G.  COMPLETION DEPTH:	BECOMES MEDIUM STIFF AT 28.5')  WERY STIFF BROWN CLAY, SOME  SILT, TRACE OF SAND—MOIST  BECOMES HARD AT 31.5')  ETHOD: HOLLOW STEM  RA  2C  HARD BROWN SILT, SOME SAND,  7A  3C  VERY SOFT BROWN SILT,  AND SAND—WET  10A  (BECOMES MEDIUM STIFF AT 28.5')  11A  (BECOMES MEDIUM STIFF AT 28.5')  12A  WERY STIFF BROWN CLAY, SOME  SILT, TRACE OF SAND—MOIST  (BECOMES HARD AT 31.5')  ETHOD: HOLLOW STEM  AUGER  10.6  WATER OBSERVATIONS  INITIAL DEPTH:	BOIL CLASSIFICATION SYSTEM:  8A 17.0-18,5  2C 18,5-20,5  HARD BROWN SILT, SOME SAND, TRACE OF CLAY-MOIST  3C 22,0-24,0  VERY SOFT BROWN SILT, AND SAND - WET  (BECOMES MEDIUM STIFF AT 25.5')  (BECOMES MEDIUM STIFF AT 28.5')  (BECOMES MEDIUM STIFF AT 28.5')  (BECOMES MEDIUM STIFF AT 28.5')  (BECOMES MEDIUM STIFF AT 28.5')  (BECOMES MEDIUM STIFF AT 28.5')  (BECOMES MEDIUM STIFF AT 28.5')  (BECOMES MEDIUM STIFF AT 28.5')  (BECOMES MEDIUM STIFF AT 28.5')  (BECOMES MEDIUM STIFF AT 28.5')  (BECOMES MEDIUM STIFF AT 28.5')  (BECOMES MEDIUM STIFF AT 28.5')  (BECOMES MEDIUM STIFF AT 28.5')  (BECOMES MEDIUM STIFF AT 28.5')  (BECOMES HARD AT 31.5')	BECOMES MEDIUM STIFF AT 28.5')  BECOMES HARD AT 31.5')  ETHOD: HOLD WORK SAND AT SILT, SOME SAND AND SAMPLER  10.0 10.5 20.5 20.5 20.5 20.5 20.5 20.5 20.5 2	DESCRIPTION OF MATERIAL  SOUL CLASSIFICATION SYSTEM:  8A 17,0-10,5 0-1-1 2  2C 18,5-20,5  HARD BRDWN SILT, SOME SAND, TRACE OF CLAY-MOIST 9A 20,5-22,5 15-17-16 33  3C 22,0-240  VERY SOFT BRDWN SILT, AND SAND - WET 10A 240-25,5 3-1-2 3  (BECOMES MEDIUM STIFF AT 25.5')  1/A 25,5-27,0 3-4-4 8  (BECOMES MEDIUM STIFF AT 28.5')  1/A 25,5-27,0 3-4-4 8  (BECOMES M

# LOG OF BORING No. 50 6.5-/

# CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: AS DIRECTED

DATE STARTED:

9/9/84

SURFACE ELEVATION: 502,2 (NATER)

	LEVATION: 5 02.2 Correctly				7.7	<b>5</b>
STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. & TYPE	SAMPLE DEPTH	SLOWS PER 6" ON SAMPLER	BLOWS/ FT.	SAMPLE RECOVERY IN.
34.0	NERY STIFF BROWN SILT, SOME SAND, TRACE DICLAY-MOIST	16 A	33,0-345	11-13-16	29	18"
		17A	345-360	6-12-16	28	14"
	(BECOMES HARD AT 44.0')	18A	360-375	21-21-23	44	16"
37.0	HARD GRAY CLAY, SOME SILT. TRACE OF SAND-MOIST			11-14-18		18"
	(BECOMES VERY STIFF AT 39.0')			<i>5-8-</i> /3		18"
		214	40,5-42,0	11-12-13	25	18"
				6-7-13		
		23A	435-450	6-9-14	23	18"
		24.A	450-46,5	6-9-11	20	18"
	ETHOD: HOLLOW STEM WATER OBSERV.  AUGER  B.C. B.C.M. K.G. COMPLETION DEPTH:			TYPE SA		N
JOB NO.: 2	28089 DEPTH AFTER	HRS		<b>∑</b> c. sн	ELBY TUI	3E

(PAGE 4 0 14)

# LOG OF BORING No. SO 6.5-/

# CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: AS DIRECTED

DATE STARTED:

7/9/84

SURFACE ELEVATION: 502.2 (WATER)

STRATUM	DESCRIPTION OF MATERIAL SOIL CLASSIFICATION SYSTEM:	SAMPLE NO. &	SAMPLE DEPTH	BLOWS PER 6" ON Sampler	BLOWS/ FT.	SAMPLE RECOVERY IN.
			·	`		
	· · · · · · · · · · · · · · · · · · ·	2 <i>5A</i>	465-480	4-10-10	20	18
48.0'	VERY STIFF GRAY CLAY, SOME SILT, ROCK FRAGMENTS - MOIST	264	480 -435	9-/0-/3	23	15
	(BECOMES HARD AT 49.5')				<b>.</b>	į
		27A	49,5-51,0	13-16-15	31	17
		2 <i>8A</i>	510-520	14-70/3"	84/9.	9.
		1		40-60/2.		
<i>55.0</i>	LIMESTONE SHALE	IB	55,0-65,0		-	110
65.0	BOTTOM OF BODING					
	•					
	ETHOO: HOLLOW STEM WATER OBSERVANCE S. CB.C.M K. G. COMPLETION DEPTH:	2.0	,	TYPE SA	.IT SP001	4
	28089 DEPTH AFTER			<b>⊠</b> 8. <b>Λ</b> Χ <b>⋈</b> c. shi		

## LOG OF BORING No. 507-/

# CLIFTY CREEK FLYASH IMPOUNDIMENT

BORING LOCATION: AS DIRECTED

SURFACE ELEVATION: 577

DATE COMPLETED: 9/19/84

STRATUM	DESCRIPTION OF MA	TERIAL	NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	BLOWS/ FT.	SAMPLE RECOVERY IN.
o'	BARGE	· · · · · · · · · · · · · · · · · · ·					
2.0'	TOP OF WATE						
	FLYASH						
	1" TO 1" WEATHER	*					
	LIMESTONE SH	· ·	1B	59-69		-	120'
,	BOTTOM OF BOR						
	NOTE: CONE PEN	CTRATION TE	ST A	80RING			
DRILLER: <u>Æ</u>	ETHOD: HOLLOW STEM AUGER B.CK.G.	WATER OBSERVINITIAL DEPTH:	2.0	· I	☐ A. SF ② B. A	AMPLER: PLIT SPOOM	0CK < 0
108 NO.: Z	8089	DEPTH AFTER	HRS		c. sh	ELBY TUE	SE

BOWSER-MORNER TESTING LABORATORIES, INC.

Woodward-Clyde	SUBJECT Toble 4	Cone Kesista	FILE No. 84C	SER-MOTHER
Consultants Chicago	COMPUTED 5775	CHECKED Fu)	FILE NO	
	Resistance		Resistance	
when the state	4	10 th 14	16/ft	
deth, ft	10/70	depth, ft	10/70	_ Boring 507-1
7.0	1115	35,0	43681	507-7
7.5	5576	35,5	55764	
8.0	8364	36.5	55764	
9.0	16729	37.5	61340	
10,5	2/747	38.5	41823	
11.5	13841	39,5	55764	
12,5	22305	40.0	50187	
13,5	22305	41.0	33458	
14.5	27882	46.0	36246	
15.0	72493	43.0	39634	
14,0	16729	44,0	19517	
17.0	16729	45.0	5576	
18.0	0	46.0	16 724	
19.0	3345,8	47.0	19517	
20.0	16729	48.0	33458	
20.5	21376	49.0	22305	•
21.0	26023	50.0	16729	
22.0	25093	51,0	446111	
23,0	557	52.0	19517	•
244	0	53,0	19517	
24.9	0	54.0	22305	
25,9	3345	55.0	61340	
26.9	24164	55,3	66916	
27.9	58051	55.5	178444	
28,9	11150	560	78069	
29,4	6691	56,3	557640	
30.0	43681			
31.0	2730			
32,0	39034			
33,0	41823			
33,5	39034			
33,8	33458			
34,0	11152			Table 4

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#10-7-49

4-18-1884 BORING SO-74
ORILLED 9-19-193 CONE PENETRATION TEST CLIFTY CREEK POWER RANT BOWSER-NORMER

MCC DECEMB

# LOG OF BORING No. 50 7-2

# CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: AS DIRECTED

DATE STARTED: 9/10/84

SURFACE ELEVATION: 502,2 (WATER)

STRATUM	DESCRIPTION OF MA	TERIAL	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON SAMPLER	BLOWS/	SAMPLE RECOVERY IN.
		. ·					
0'	BARGE						
2.0'	WATER			· ·	·		
		•					
5.0'	FLYASH		14	50-6.5	0-1-1	2	12"
							"
		· <del>·</del>	2.A	8.5-100	1-2-2	4	14"
			3,4	13,5-15,0	0-0-0	0	10"
			4 <i>A</i>	185.200	0-0-0	0	10"
				230-250		3	16"
28.0	MEDIUM STIFF GE SILT, TRACE OF DRO		6A	7 <i>8,5-30,0</i>	1-2-4	6	10"
,	VERY STIFF BROW						
34.0	SOME SILT - DA		IA	3 <b>3,5-</b> 35,0	4-9-17	26	14"
	ETHOD: HOLLOW STEM AUGER 3.CB.C.MK.G.	WATER OBSERVA	. 0'		TYPE SA  A. SPI	.IT SP001	4
	28089	DEPTH AFTER			<del></del>	LBY TUE	

#### 507-2 LOG OF BORING No.

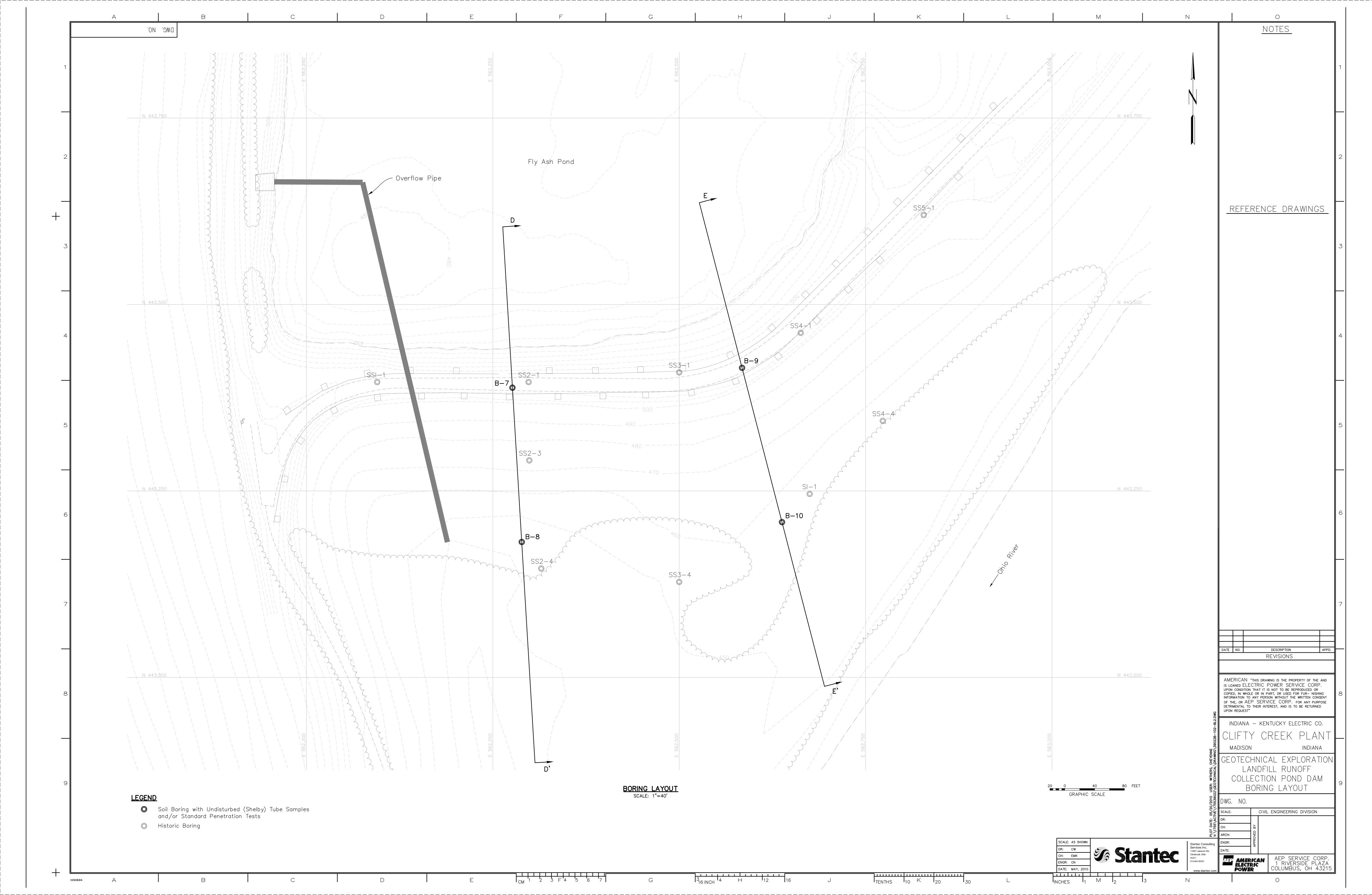
# CLIFTY CREEK FLYASH IMPOUNDMENT

BORING LOCATION: AS DIRECTED

SURFACE ELEVATION: 502.2 (WATER)

		_				•	- 1
STRATUM	DESCRIPTION OF MAT	ERIAL	SAMPLE NO. & TYPE	SAMPLE DEPTH	BLOWS PER 6" ON Sampler	TLOWS/ FT.	SAMPLE RECOVERY IN.
3 <i>8.0</i>	VERY STIFF BROW		8A	38.5-10.0	5-8-13	2/	16*
44.0	HARD BROWN SILL CLAY, TRACE OF ROO	K FRAGMENTS DAMP	9 <sub>A</sub>	43,5-45,0	15-21-30	51	12"
48.5'	HARD GREEN CLAY	, SOME VIS-DAMP	JOA	485-50p	9-18-6%	78/18	. 10 "
		· · · · · · · · · · · · · · · · · · ·	11 A	53,5-54,0	100/5"	109/	5"
55.5	LIMESTONE /S	HALE	1B	55,5-65,5			120"
65.5	BOTTOM OF BO	RING					
	•			,			
			<u></u>				
DRILLING METHOD: 10220W STEM WATER OBSERVATIONS  AUGER  INITIAL DEPTH: 2.0'  COMPLETION DEPTH: 2.0'  JOB NO.: 28089  DEPTH AFTER						CK COEL	

Stantec (2016)



# LANDFILL RUNOFF COLLECTION POND: 2009 GEOTECHNICAL EXPLORATION



Project Name   AEP Clifty Creek / Ash Ponds   Jufferson, IN   Jufferson, IN   Project Type   Geotechnical Exploration   Date Started   11/12/09   Completed   11/12/09   Completed   11/12/09   Date / Time   11/12/09   Date / Time   11/12/09   Date / Time   11/12/09   Date / Time   11/12/09   Date / Time   11/12/09   Date / Time   11/12/09   Date / Time   11/12/09   Date / Time   11/12/09   Date / Time   N/A   Date / Time / Time   N/A   Date / Time   N/A   Date / Time / Time / Time	Project	Number	175539022			Location	С	rest: LRC	P Dam	
Project Type	Project	Name	AEP Clifty Creek /	Ash Ponds		Boring No.	B-7		Total Depth	29.0 ft
Supervisor   C. Nisingizwe   Driller   M. Wethington   Depth to Water   Dry   Date/Time   11/12/09   N/A	County		Jefferson, IN			Surface Ele	vation	50	3.4 ft	
Logged By   C. Nisingizwe   Depth to Water   N/A   Date/Time   N/A	Project	Туре	Geotechnical Explo	oration		Date Started	d 1	1/12/09	Completed	11/12/09
Lithology	Supervi	sor	C. Nisingizwe Dri	ller M. Wet	hington	Depth to Wa	ater D	ry	Date/Time	11/12/09
Elevation   Depth   Description   Rock Core   RQD   Run   Rec. Ft.   Rec. %   Run Depth   Remarks	Logged	Ву	C. Nisingizwe			Depth to Wa	ater N	/A	Date/Time	N/A
ST-1   23.0 - 25.0   2.0	Lithol	ogy		Overburden	Sample #	Depth	Rec. Ft.	Blows	Mois.Cont. %	
ST-1 23.0 - 25.0 2.0  ST-2 25.0 - 27.0 2.0 20  No Refusal /	Elevation	Depth		Rock Core	RQD	Run	Rec. Ft.	Rec. %	Run Depth	Remarks
ST-1   23.0 - 25.0   2.0			•							
Lean Clay, yellow and light gray, moist, stiff  ST-1 23.0 - 25.0 2.0 ST-2 25.0 - 27.0 2.0 20 ST-3 27.0 - 29.0 2.0 20  No Refusal /	502.9'	0.5'	Asphalt pavement	and						-
ST-1   23.0 - 25.0   2.0     ST-2   25.0 - 27.0   2.0   20   ST-3   27.0 - 29.0   2.0   20   No Refusal /	_		(-	and light						-
ST-2   25.0 - 27.0   2.0   20   20   ST-3   27.0 - 29.0   2.0   20   No Refusal /				and ngm						_
ST-2   25.0 - 27.0   2.0   20   20   ST-3   27.0 - 29.0   2.0   20   No Refusal /	-									_
ST-2   25.0 - 27.0   2.0   20   20   ST-3   27.0 - 29.0   2.0   20   No Refusal /	-									-
ST-2   25.0 - 27.0   2.0   20   20   ST-3   27.0 - 29.0   2.0   20   No Refusal /										_
ST-2   25.0 - 27.0   2.0   20   20   ST-3   27.0 - 29.0   2.0   20   No Refusal /	-									-
ST-2   25.0 - 27.0   2.0   20   20   ST-3   27.0 - 29.0   2.0   20   No Refusal /	-									-
ST-2   25.0 - 27.0   2.0   20   20   ST-3   27.0 - 29.0   2.0   20   No Refusal /										_
ST-2   25.0 - 27.0   2.0   20   20   ST-3   27.0 - 29.0   2.0   20   No Refusal /										_
ST-2   25.0 - 27.0   2.0   20   20   ST-3   27.0 - 29.0   2.0   20   No Refusal /	-									-
ST-2   25.0 - 27.0   2.0   20   20   ST-3   27.0 - 29.0   2.0   20   No Refusal /	-									_
ST-2   25.0 - 27.0   2.0   20   20   ST-3   27.0 - 29.0   2.0   20   No Refusal /										_
ST-2   25.0 - 27.0   2.0   20   20   ST-3   27.0 - 29.0   2.0   20   No Refusal /	-									-
ST-2   25.0 - 27.0   2.0   20   20   ST-3   27.0 - 29.0   2.0   20   No Refusal /	-									_
ST-2   25.0 - 27.0   2.0   20   20   ST-3   27.0 - 29.0   2.0   20   No Refusal /	<b>-</b>									_
ST-2   25.0 - 27.0   2.0   20   20   ST-3   27.0 - 29.0   2.0   20   No Refusal /										_
ST-2   25.0 - 27.0   2.0   20   20   ST-3   27.0 - 29.0   2.0   20   No Refusal /	_									-
ST-3 27.0 - 29.0 2.0 20 No Refusal /	-				ST-1	23.0 - 25.0	2.0			-
ST-3 27.0 - 29.0 2.0 20 No Refusal /					ST 2	25.0 27.0	2.0		20	_
474.4'   29.0'   No Refusal /	_				31-2	20.0 - 21.0	2.0		20	_ _
No Refusal /		29 0'			ST-3	27.0 - 29.0	2.0		20	-
	7/7.4	20.0	No Refusal /		I .	<u> </u>	I	<u> </u>		_
										=
5										-
										_
										-
										-
										-
Stantec Consulting Services Inc.										4/16/10



Project I	Number	175539022			Location		Toe: LRCF	Dam	
Project I	Name	AEP Clifty Creek /	Ash Ponds		Boring No.	B-	8	Total Depth	a 31.0 ft
County	_	Jefferson, IN			Surface Ele	vation_	44	1.5 ft	
Project <sup>-</sup>	Туре	Geotechnical Explo	oration		Date Started	d	11/19/09	Completed	11/19/09
Supervis	sor	C. Nisingizwe Dri	ller Danny	Jessie	Depth to Wa	aterl	Dry	Date/Time	11/19/09
Logged	Ву	C. Nisingizwe			Depth to Wa	aterl	N/A	Date/Time	N/A
Litholo	ogy		Overburden	Sample #	Depth	Rec. Ft	. Blows	Mois.Cont. %	
Elevation	Depth	Description	Rock Core	RQD	Run	Rec. Ft	. Rec. %	Run Depth	Remarks
441.5'	0.0'	Top of Hole							
<b> </b>		Silty Clay, yellow a gray, damp to moi							-
		3 77 1							-
-									-
<b>-</b>									_
									-
-									-
-									_
-									-
-									-
L									
425.5'	16.0'								-
-		Lean Clay, yellowi and light gray, mo	sh brown ist						_
		3 3 7							- -
-									_
-									_
									- -
-									_
-									_
02				ST-1	25.0 - 27.0	2.0		25	-
100   -  -  -   412.5'				ST-2	27.0 - 29.0	2.0		26	-
412.5'	29.0'	Lean Clay With Sa	and	_					-
호 - 410.5'	31.0'	yellowish brown a	nd light	ST-3	29.0 - 31.0	2.0		23	
		∖gray, moist	/						-
Y CREEK		No Refusal / Bottom of Hole							-
77553002 CLIFTY CREE									
									-
									-
ANTEC/FMSM_LEGACY									_
TANT									4/16/10
			Stanted	Consul	ting Services	Inc.			4/10/10



Project Number	er 175539022			Location		Crest: LRC	P Dam	
Project Name	AEP Clifty Creek /	Ash Ponds		Boring No.	B	-9	Total Depth	22.0 ft
County	Jefferson, IN			Surface Ele	vation	50	4.3 ft	
Project Type	Geotechnical Explo	oration		Date Started	- b	11/12/09	Completed	11/12/09
Supervisor	C. Nisingizwe Dr	iller M. We	thington	Depth to Wa	ater	Dry	Date/Time	11/12/09
Logged By	C. Nisingizwe			Depth to Water N/A		Date/Time	N/A	
Lithology		Overburden	Sample #	Depth	Rec. F	t. Blows	Mois.Cont. %	
Elevation Depth	Description	Rock Core	RQD	Run	Rec. F	t. Rec. %	Run Depth	Remarks
504.3' 0.0'	Top of Hole							_
503.8' 0.5'	Asphalt pavement	t and						-
- - - - - - - - - - -	Lean Clay, yellow and light gray, dar moist		ST-1	16.0 - 18.0	2.0		22	- - - - - - - - - - - - - -
_			ST-2	18.0 - 20.0	2.0		19	_
482.3' 22.0'			ST-3	20.0 - 22.0	2.0		20	-
	No Refusal / Bottom of Hole							- - - - - - - - - - - - - -



Project	Number	175539022			Location	Т	oe: LRCF	Dam	
Project	Name	AEP Clifty Creek /	Ash Ponds		Boring No.	B-1	0	Total Depth	n 18.0 ft
County	-	Jefferson, IN			Surface Ele	vation	45	7.3 ft	
Project <sup>-</sup>	Туре	Geotechnical Explo	ration		Date Started	 d 1	1/19/09	Completed	11/19/07
Supervi	sor	C. Nisingizwe Dri	ller Danny	Jessie	Depth to WaterDry			Date/Time	11/19/07
Logged	Ву	C. Nisingizwe			Depth to Water N/A			Date/Time	N/A
Litholo	ogy		Overburden	Sample #	Depth	Rec. Ft.	Blows	Mois.Cont. %	
Elevation	Depth	Description	Rock Core	RQD	Run	Rec. Ft.	Rec. %	Run Depth	Remarks
457.3'	0.0'	Top of Hole							
- - -		Silty Clay With Sa yellow and light gr to moist	nd, ay, damp						- - -
- - -									- - -
-									<del>-</del>
- - 444.1'	13.2'			ST-1	12.0 - 14.0	1.5		17	_
-		Silty Sand, gray to	brown,						=
441.3'	16.0'	damp to moist		ST-2	14.0 - 16.0	2.0		10	_
439.3'	18.0'	Silty Clay With Sa yellow and light gr \to moist	nd, ay, damp	ST-3	16.0 - 18.0	2.0		25	_
- - - -		No Refusal / Bottom of Hole	/						_ _ _ _
- - -									
HICLOS.401 4718									
- TANSMICKARI									- - -
TI Y CKEEK'G									_
175538022.0									
M_LEGAC)									=
- Lec'twis									=
N N			Ctarata	. O	tina Services	lna			4/16/10

# LANDFILL RUNOFF COLLECTION POND: 2015 GEOTECHNICAL EXPLORATION



Page: 1 of 3

Project Nur	mber	175553022			Location	L	andfill Rur	noff Collection	on Pond Dam
Project Nar	me	CCR Rule - AEP CI	ifty Creek		Boring No.	B-1	2	Total Depth	n 101.5 ft
County		Jefferson, IN			Surface Elev	ation/	503	3.9 (estimate	ed)
Project Typ	pe	Geotechnical Explo	ration		Date Started	7.	/6/15	Completed	7/7/15
Supervisor		C. Nisingizwe Dri	ller E. Cau	dill	Depth to Water 60.0 ft			Date/Time	7/7/15
Logged By	- '	C. Nisingizwe			Depth to Wa	iter N	/A	Date/Time	N/A
Lithology		-	Overburden	Sample #	Depth	Rec. Ft.	Blows	Mois.Cont. %	
Elevation D	Depth	Description	Rock Core	RQD	Run	Rec. Ft.	Rec. %	Run Depth	Remarks
	0.0	Top of Hole							
(estimated) \( \square{1} \)	0.4_/	Asphalt and base							Pocket -
-		Lean Clay With Sa damp, medium stif	nd, gray, f to stiff	SPT-1	1.0 - 2.5	1.5	1-2-5	21	Penetrometer (PP) = 2.50 tsf
									-
				SPT-2	5.0 - 6.5	1.5	3-3-4	20	PP = 2.50 tsf
-									-
									]
									PP = 3.50 tsf
-				SPT-3	10.0 - 11.5	1.2	3-4-5	23	-
-									-
									]
									PP = 2.50 tsf
-				SPT-4	15.0 - 16.5	1.0	3-3-5	19	_
									-
									PP = 2.50 tsf
-				SPT-5	20.0 - 21.5	0.9	4-6-9	18	-
-									-
									-
									PP = 4.25 tsf
-				SPT-6	25.0 - 26.5	1.1	3-5-7	18	- 4.20 (3)
8678									_
<u>-</u>									-
DIAMETER STANDARD									PP = 4.50 tsf -
- HWSW				SPT-7	30.0 - 31.5	1.3	2-5-8	19	-4.50 (5)
-									-
									+
- LEG									DD = 4.00 t-f
				SPT-8	35.0 - 36.5	0.9	WOH-3-4	18	PP = 4.00 tsf –
40				Cta	ıntec				8/6/15

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Page: 2 of 3

Project	Number	175553022			Location	L	andfill Ru	noff Collection	n Pond Dam
Project	Name	CCR Rule - AEP C	Clifty Creek		Boring No.	<u>B-1</u>	2	Total Depth	101.5 ft
Litholo	ogy		Overburden	Sample #	Depth	Rec. Ft.	Blows	Mois.Cont. %	
Elevation	Depth	Description	Rock Core	RQD	Run	Rec. Ft.	Rec. %	Run Depth	Remarks
	40.0	Lean Clay With Sadamp, medium sti	and, gray, iff to stiff						_
		Silty Clay With Sa brown, moist, med to very stiff		SPT-9	40.0 - 41.5	1.5	6-8-8	16	
-				SPT-10	45.0 - 46.5	1.5	1-3-5	19	-
_	50.0	Silt With Sand, gra	avish light	SPT-11	50.0 - 51.5	1.5	2-3-3	22	-
		brown, moist, med to stiff	dium stiff	SF I-II	30.0 - 31.3	1.5	2-3-3	22	
<b>-</b>	58.0			SPT-12	55.0 - 56.5	1.0	2-5-8	20	-
- -	00.0	Silty Sand, grayish brown, damp, very		SPT-13	60.0 - 61.5	1.4	3-11-17	15	-
	63.5	O'll Wills Occasion	- Jak Baka						
<del>-</del>		Silt With Sand, grabrown, wet, stiff	ayısıı ligili	SPT-14	65.0 - 66.5	1.5	2-3-8	28	-
	70.0	Sand, mottled gra brown, moist to we medium stiff to stif	et,	SPT-15	70.0 - 71.5	1.5	3-5-5	22	-
_				SPT-16	75.0 - 76.5	1.3	2-3-5	28	-
	78.0								8/6/1

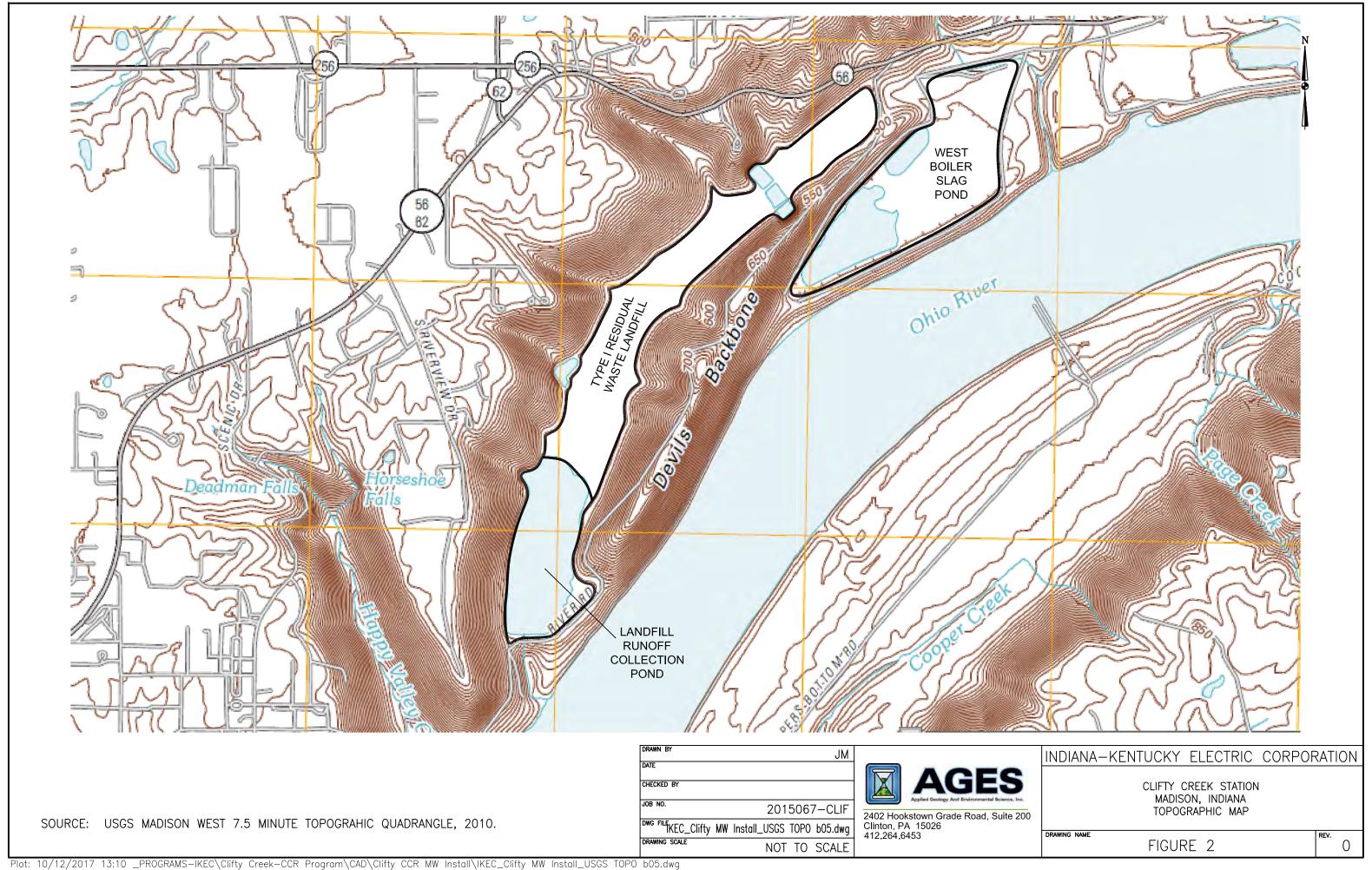
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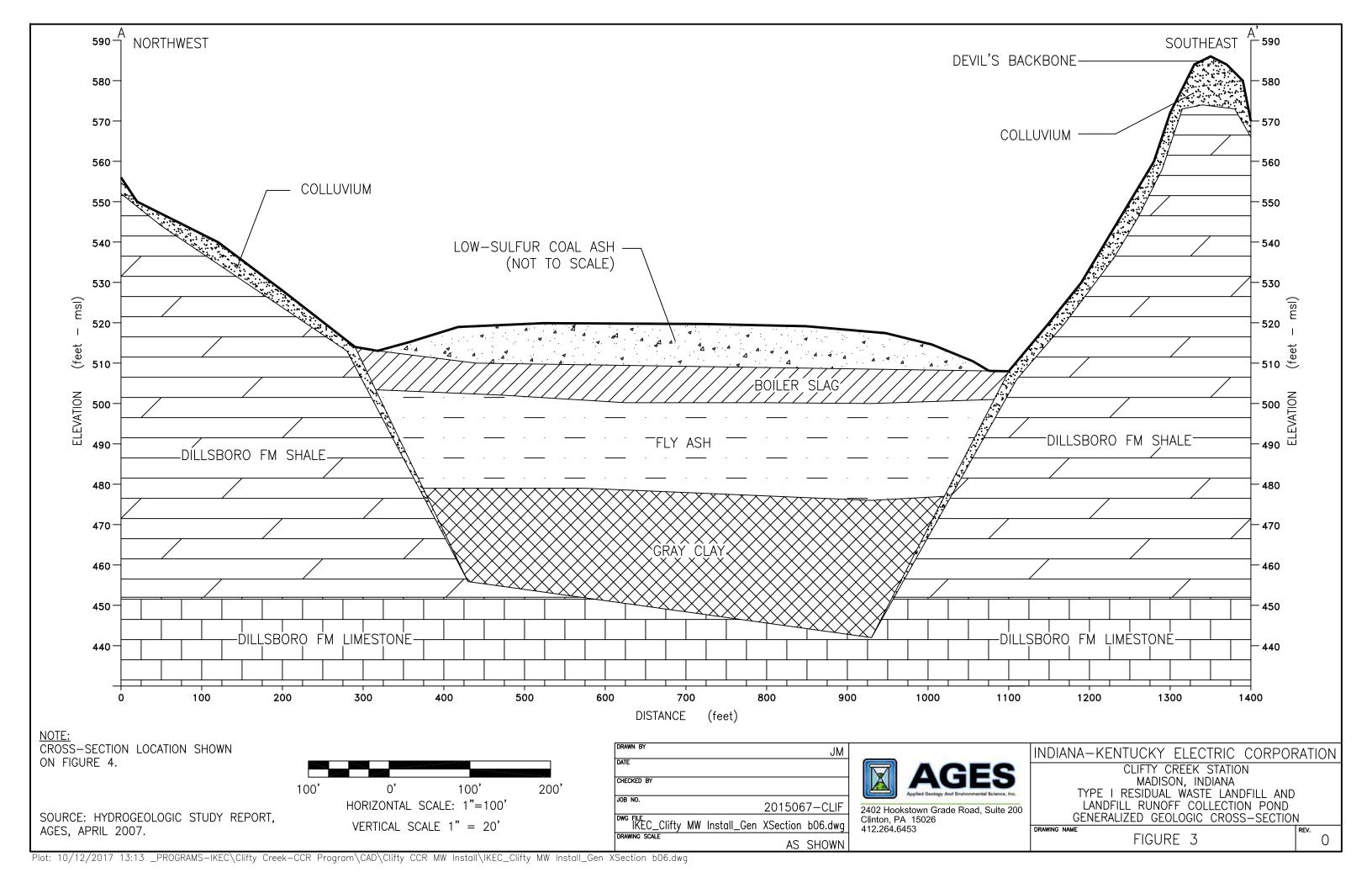


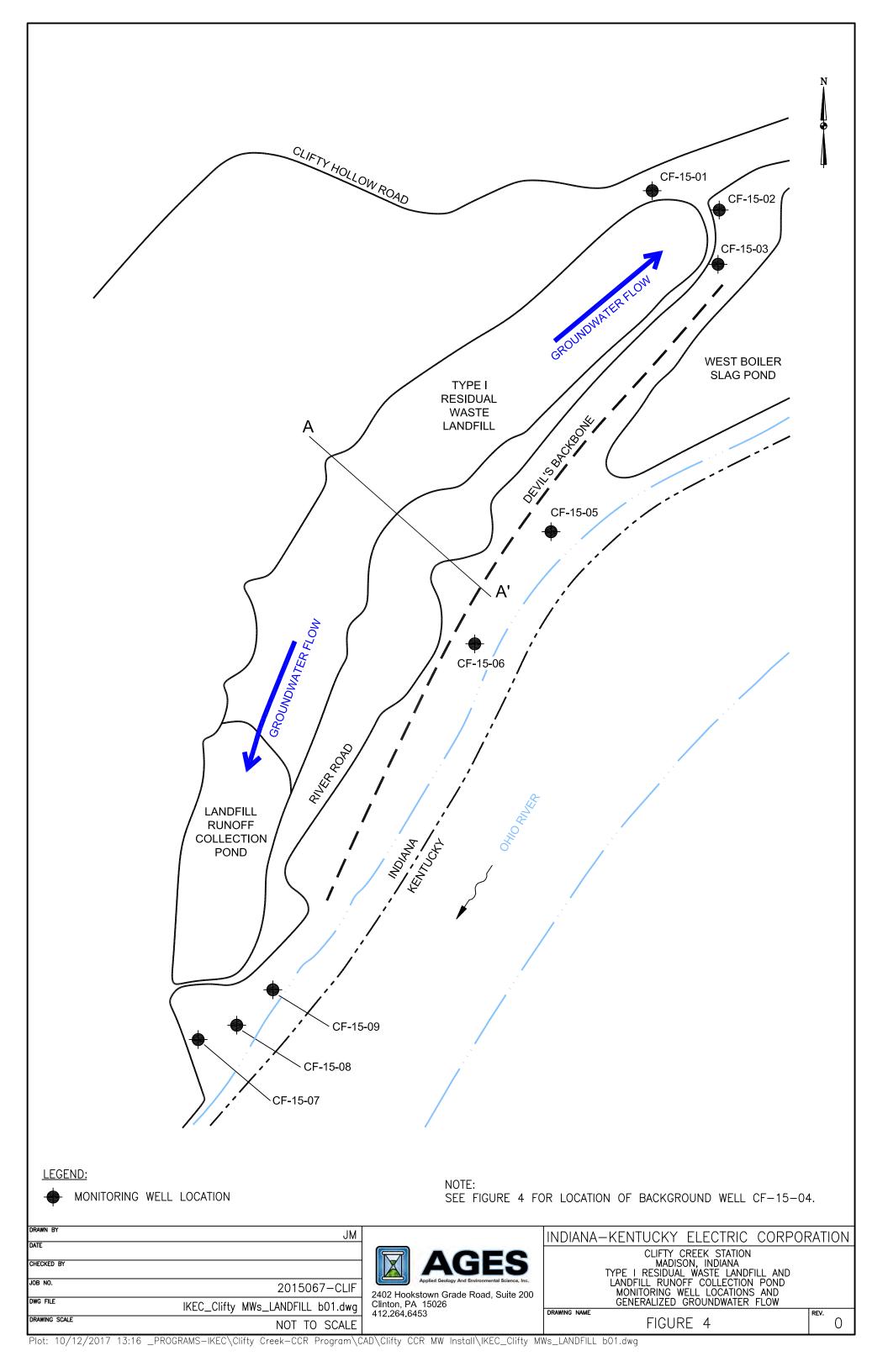
Page: 3 of 3

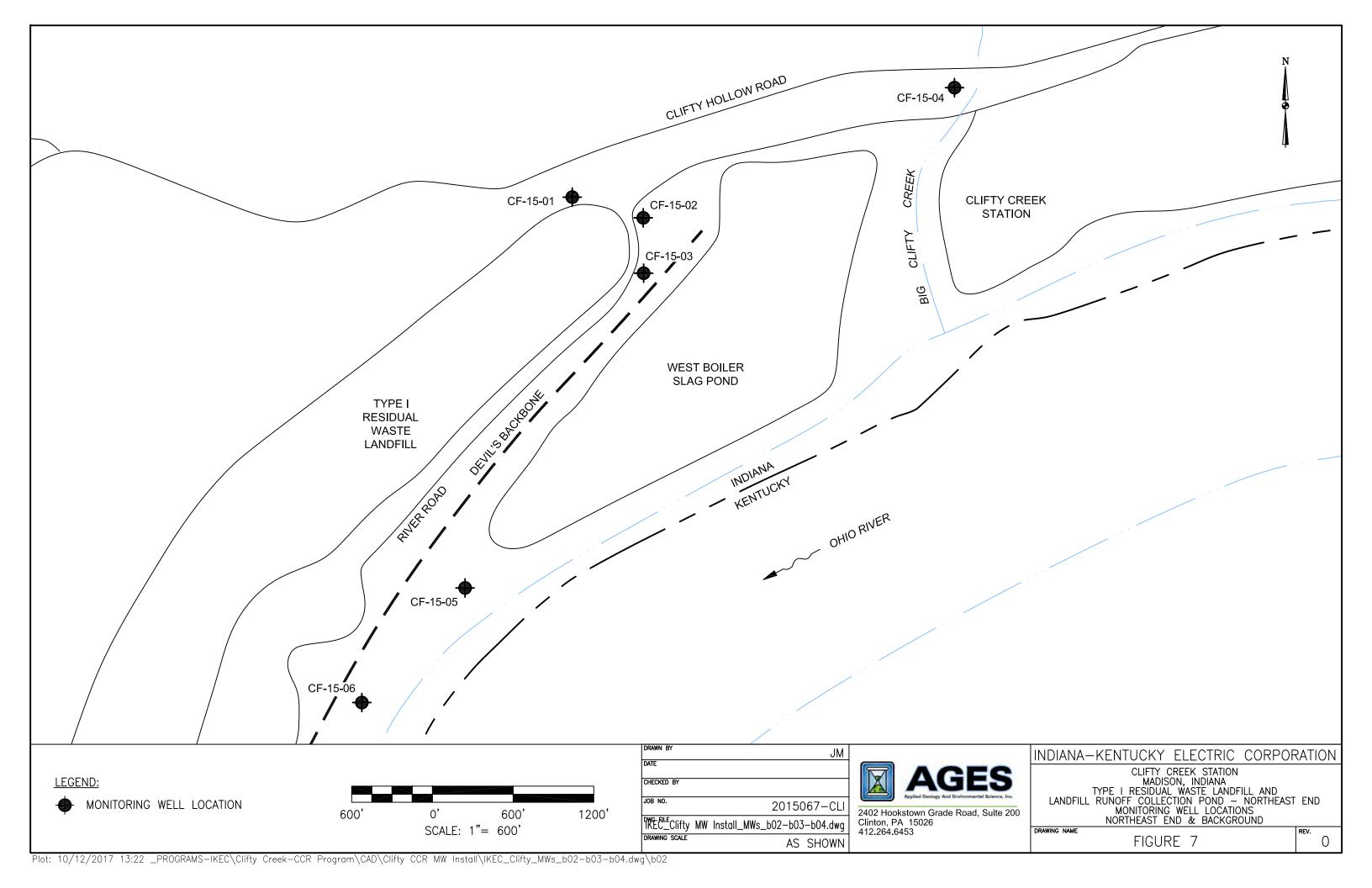
Project I	Number	175553022		Location	La	andfill Rui	noff Collection	on Pond Dam	
Project I	Name	CCR Rule - AEP C	lifty Creek		Boring No.	<b>B-1</b> 2	2	Total Dept	h 101.5 ft
Litholo	ogy		Overburden	Sample #	Depth	Rec. Ft.	Blows	Mois.Cont. %	
Elevation	Depth	Description	Rock Core	RQD	Run	Rec. Ft.	Rec. %	Run Depth	Remarks
- - - -		Silt, gray, moist to medium stiff to stif	wet, f	SPT-17	80.0 - 81.5	1.5	6-9-6	26	-
- - -				SPT-18	85.0 - 86.5	1.5	2-3-5	28	-
-	90.0	Lean Clay, gray, m medium stiff to ver	noist, y stiff	SPT-19	90.0 - 91.5	1.5	2-4-4	25	PP = 2.25 tsf -
- - - -				SPT-20	95.0 - 96.5	1.5	5-8-11	23	PP = 3.75 tsf -
- - -	101.5			SPT-21	100.0 - 101.5	1.5	4-6-8	27	PP = 3.50 tsf -
		No Refusal / Bottom of Hole							-
					intec				8/6/15

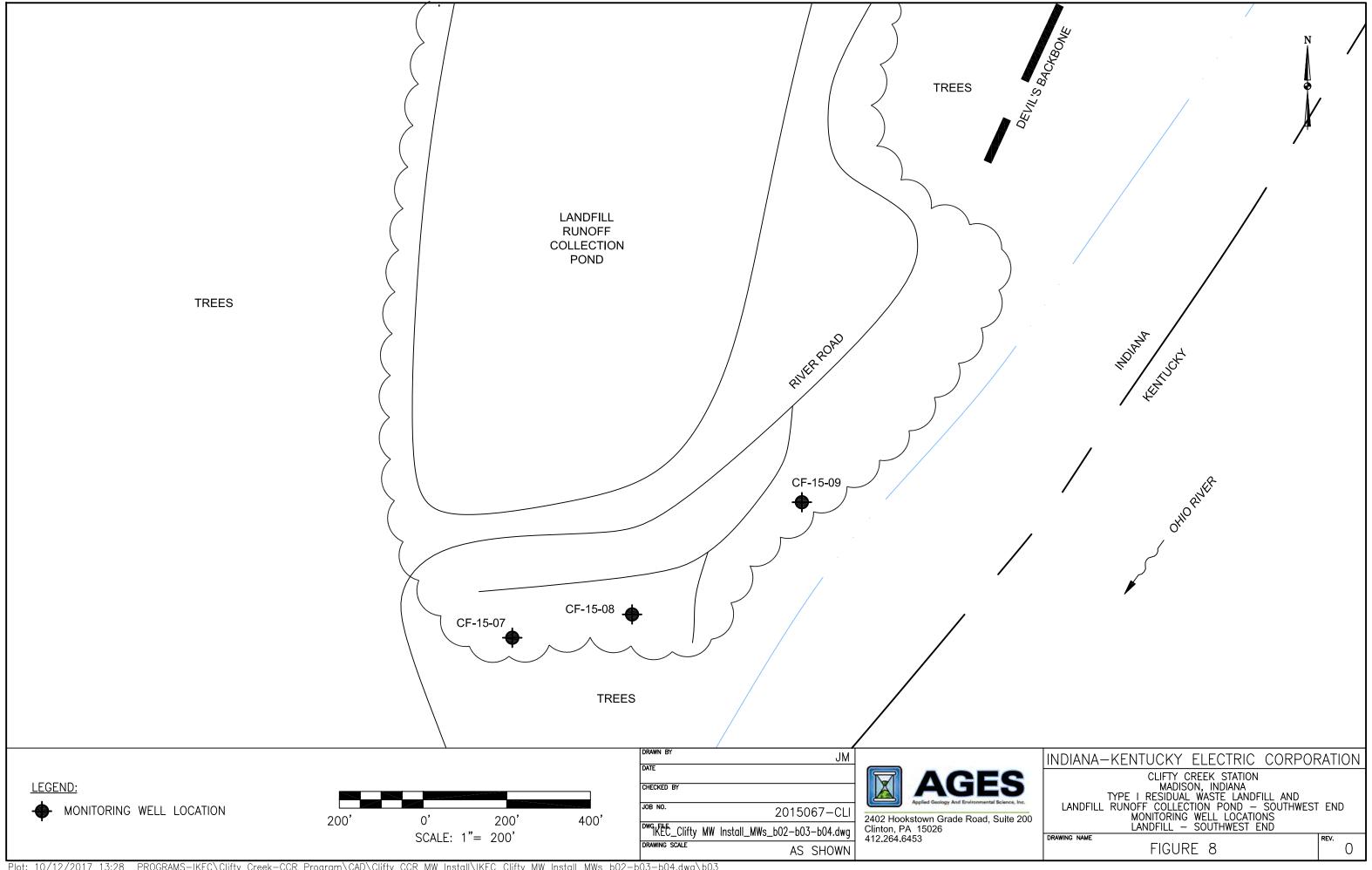
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# APPENDIX B BORING & WELL LOGS

#### BORING NO. <u>CF-15-01</u> SAMPLE/CORE LOG

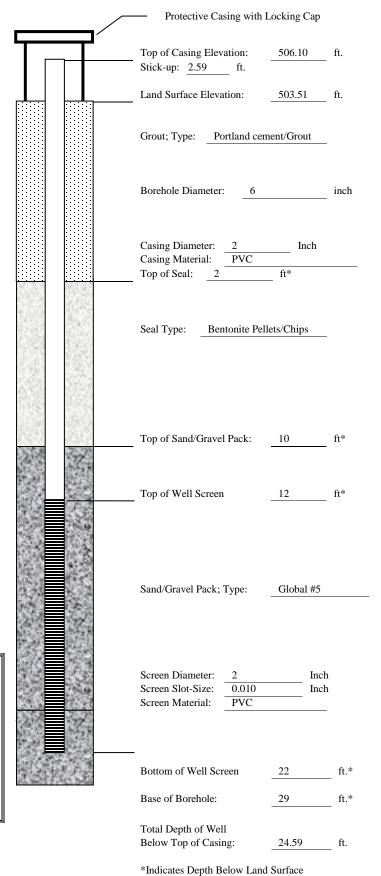
Project Number:	2015067 Clifty Creek Plant		Log Page	1	of	1	
Project Location:	Landfill Northeast End		Drilling Co	ntractor:	Bowser Morn	er	
Drilling Date(s):	12/1/15		AGES Geo	logist:	Mike Gelles		
Drilling Method:	Roto-Sonic	Coring Device Size:	NA	Hammer	Wt. NA	and Drop 1	NA
Sampling Method:	NA	Borehole Diameter:	6"	Drilling	Fluid Used:	Water	
Sampling Interval:	NA	Borehole Depth:	29'	Surface	Elevation:	503.51' MSL	
NOTES/COMMI	ENTS:						

Depth Interval (feet)	Sample Recovery (feet)	Penetration (Hyd. Pres. or Blow Counts)	Sample/Core Description	PID (PPM)
0-10	8	NA	Yellow brown silty clay, stiff, plastic, moist	N/A
10-20	8	NA	10'-15' Gray brown silty clay, stiff, plastic, moist, wet; 15'-20' light gray limestone, dry	N/A
20-29	6	NA	20'-27' Orange brown silty clay, limestone fragments, stiff, moist; 27'-29' light gray limestone, hard, dry	N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A

#### WELL CONSTRUCTION LOG WELL NO. CF-15-01

2015067 Project Number: Clifty Creek Plant -Landfill Northeast End Project Location: Installation Date(s): 12/1/2015 Drilling Method: Roto-Sonic Drilling Contractor: Bowser Morner Development Date(s): 12/14/15 Submersible Pump, Development Method: Peristaltic Pump, Bailer Field parameters stabilized. Turbidity = 3.12 NTUs Volume Purged: 33 Static Water-Level\* 15.35' Top of Well Casing Elevation: 506.10' Well Purpose: Groundwater Monitoring Northing (Y): 450793.03 Easting (X): 566812.11 Comments/Notes: 2 inch PVC riser and screen 10 ft of 0.010 pre-packed well screen with an inner filter pack of 0.40 mm clean quartz sand and an outer layer of food-grade nylon mesh. Inspector: Michael Gelles

CC	ONSTRUCTION MATERIALS USED:
6	Bags of Sand
5	Bags/Buckets Bentonite Pellets
	Bags Portland for Grout
	Bags Concrete/Sakrete



#### BORING NO. <u>CF-15-02</u> SAMPLE/CORE LOG

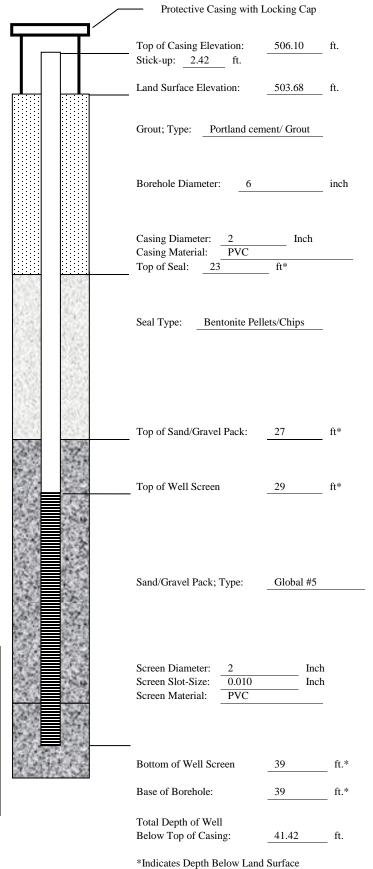
Project Number:	2015067 Clifty Creek Plant		Log Page	1	of	1	<u>l</u>	
Project Location:	Landfill Northeast End		Drilling Co	ntractor:	Bowser	Morn	er	
Drilling Date(s):	12/2/15-12/3/15		AGES Geol	logist:	Mike G	lelles		
Drilling Method:	Roto-Sonic	Coring Device Size:	NA	Hamme	r Wt.	NA	and Drop	NA
Sampling Method:	NA	Borehole Diameter:	6"	Drilling	Fluid Us	ed:	Water	
Sampling Interval:	NA	Borehole Depth:	39'	Surface	Elevation	n:	503.68' MS	L
NOTES/COMMI	ENTS:							
								-

Depth Interval (feet)	Sample Recovery (feet)	Penetration (Hyd. Pres. or Blow Counts)	Sample/Core Description	PID (PPM)
0-10	4	NA	Brown silty clay, limestone fragments, stiff, plastic, moist	N/A
10-20	6	NA	10'-17' Brown silty clay, limestone fragments, stiff, plastic, moist; 17'-20' brown silty clay, limestone fragments, stiff, plastic, wet	N/A
20-30	4	NA	Gray brown silty clay, gravel, stiff, plastic, moist	N/A
30-39	8	NA	30'-38' Brown sandy clay, silt, wet; 38'-39' gray limestone	N/A
				N/A

#### WELL CONSTRUCTION LOG WELL NO. CF-15-02

2015067 Project Number: Clifty Creek Plant -Landfill Northeast End Project Location: Installation Date(s): 12/1/15 Drilling Method: Roto-Sonic Drilling Contractor: Bowser Morner Development Date(s): 12/7/15 Submersible Pump, Development Method: Peristaltic Pump, Bailer Field parameters stabilized. Turbidity = 3.69 NTUs Volume Purged: 114.5 gallons Static Water-Level\* 15.89' Top of Well Casing Elevation: 506.10' Well Purpose: Groundawter Monitoring Northing (Y): 450449.42 Easting (X): 566908.15 Comments/Notes: 2 inch PVC riser and screen 10 ft of 0.010 pre-packed well screen with an inner filter pack of 0.40 mm clean quartz sand and an outer layer of food-grade nylon mesh. Inspector: Michael Gelles

СО	NSTRUCTION MATERIALS USED:
6	Bags of Sand
	Bags/Buckets Bentonite Pellets
6	Bags Portland for Grout
	Bags Concrete/Sakrete



#### BORING NO. <u>CF-15-03</u> SAMPLE/CORE LOG

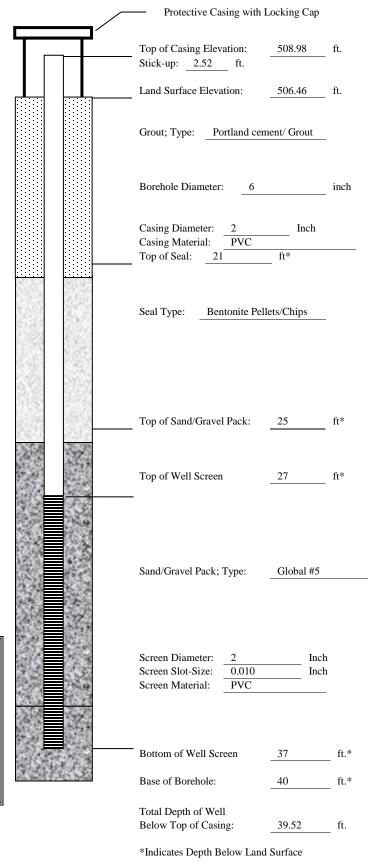
Project Number:	2015067 Clifty Creek Plant		Log Page	1	of	1	
Project Location:	Landfill Northeast End		Drilling Co	ntractor:	Bowser Morn	er	
Drilling Date(s):	12/2/15-12/3/15		AGES Geo	logist:	Mike Gelles		
Drilling Method:	Roto-Sonic	Coring Device Size:	NA	Hammer	Wt. NA	and Drop	NA
Sampling Method:	NA	Borehole Diameter:	6"	Drilling	Fluid Used:	Water	
Sampling Interval:	NA	Borehole Depth:	40'	Surface	Elevation:	506.46' MSL	
NOTES/COMMI	ENTS:						
		·					

Depth Interval (feet)	Sample Recovery (feet)	Penetration (Hyd. Pres. or Blow Counts)	Sample/Core Description	PID (PPM)
0-10	5	NA	0'-1.5' Gravel; 1.5'-5' boiler slag, fill	N/A
10-20	6	NA	10'-12' Boiler slag; 12'-16' yellow brown silty clay, limestone fragments, stiff, plastic, moist	N/A
20-30	6	NA	20' -27' yellow brown silty clay, limestone fragments, stiff, plastic, moist; 27'-30' yellow brown silty clay, limestone fragments, stiff, plastic, wet	N/A
30-40	10	NA	30'-37' Yellow brown silty clay, limestone fragments, stiff, plastic, wet; 37'-40' Dark gray silty clay, trace gravel, stiff, plastic, moist, till	N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A

#### WELL CONSTRUCTION LOG WELL NO. CF-15-03

2015067 Project Number: Clifty Creek Plant -Landfill Northeast End Project Location: Installation Date(s): 12/2/15-12/3/15 Drilling Method: Roto-Sonic Drilling Contractor: Bowser Morner Development Date(s): 12/15/15 Development Method: Submersible Pump Field parameters stabilized. Turbidity = 3.99 NTUs Volume Purged: 32.25 gallons Static Water-Level\* 19.25 Top of Well Casing Elevation: 508.98' Well Purpose: Groundwater Monitoring Northing (Y): 450262.60 Easting (X): 566915.99 Comments/Notes: 2 inch PVC riser and screen 10 ft of 0.010 pre-packed well screen with an inner filter pack of 0.40 mm clean quartz sand and an outer layer of food-grade nylon mesh. Inspector: Michael Gelles

### CONSTRUCTION MATERIALS USED: 6.5 Bags of Sand Bags/Buckets Bentonite Pellets Bags Portland for Grout Bags Concrete/Sakrete



#### BORING NO. <u>CF-15-04</u> SAMPLE/CORE LOG

Project Number:	2015067 Clifty Creek Plant		Log Page	1	of	1	
Project Location:	Landfill Northeast End		Drilling Co	ntractor:	Bowser Mor	ner	
Drilling Date(s):	12/3/15		AGES Geo	logist:	Mike Gelles		
Drilling Method:	Roto-Sonic	Coring Device Size:	NA	Hammer	Wt. NA	and Drop	NA
Sampling Method:	NA	Borehole Diameter:	6"	Drilling	Fluid Used:	Water	
Sampling Interval:	NA	Borehole Depth:	40'	Surface	Elevation:	465.55' MSL	·
NOTES/COMMI	ENTS:						

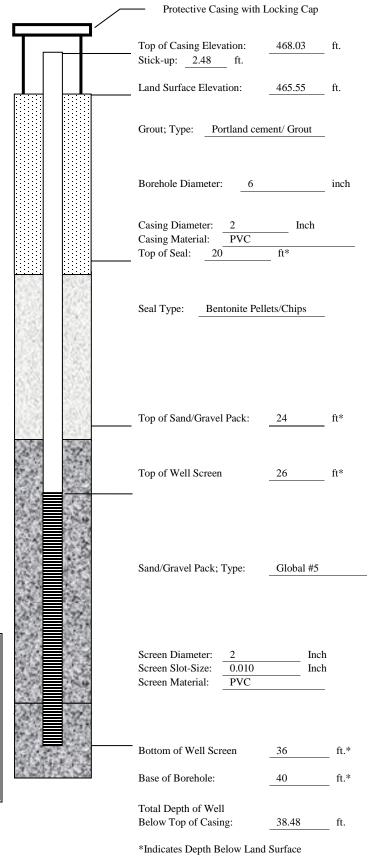
Depth Interval (feet)	Sample Recovery (feet)	Penetration (Hyd. Pres. or Blow Counts)	Sample/Core Description	PID (PPM)
0-10	7	NA	0-4' Boiler slag, clay, fine sand, moist, fill; 4'-7' gray silty clay, trace gravel, stiff, plastic, moist	N/A
10-20	9	NA	Orange brown silty clay, fine sand, gray mottling, stiff, plastic, moist	N/A
20-30	10	NA	20'-24' Orange brown silty clay, fine sand, gray mottling, stiff, plastic, moist; 24'-29' gray brown silty clay, fine sand, stiff, plastic, wet; 29'-30' gray brown silty clay, fine sand, stiff, plastic, moist	N/A
30-40	10	NA	30'-36' Orange brown silty clay, fine and medium sand, gravel, stiff, plastic, wet; 36'-40' brown gray silty clay, trace gravel, stiff, plastic, moist, till	N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A

#### WELL CONSTRUCTION LOG **WELL NO. CF-15-04**

Projec Projec Install Drillin Drillin Develo Devel Field Turbic Volun Static Top of Well P Groun North Eastin Comm 2 inch 10 ft filter p layer o Inspec

et Number:	2015067
	Clifty Creek Plant –
et Location:	Landfill Northeast End
lation Date(s):	12/3/15
ng Method:	Roto-Sonic
ng Contractor:	Bowser Morner
opment Date(s):	12/9/15
opment Method: parameters stabilize	Submersible Pump
dity = 0.91  NTUs	-
ne Purged:	65 gallons
Water-Level*	28.53'
f Well Casing Eleva	ation: 468.03'
Purpose:	
dawter Monitoring	
ing (Y): 451482.81	
ig (X): 569307.19	
nents/Notes:	
PVC riser and scre	ed well screen with an inner
	ean quartz sand and an outer
of food-grade nylon	
<u> </u>	-
ctor: Michael Ge	alles
Michael Ge	

### CONSTRUCTION MATERIALS USED: Bags of Sand Bags/Buckets Bentonite Pellets Bags Portland for Grout Bags Concrete/Sakrete



#### BORING NO. <u>CF-15-05</u> SAMPLE/CORE LOG

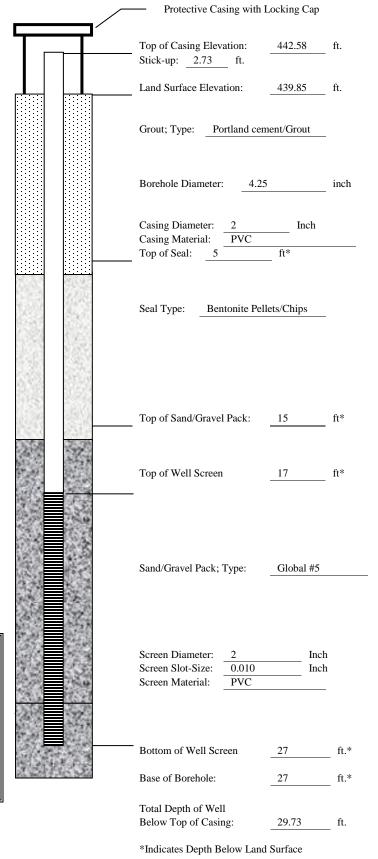
Project Number:	2015067 Clifty Creek Plant		Log Page	1	of	1
Project Location:	Landfill South End		Drilling Co	ntractor:	Bowser Morr	ner
Drilling Date(s):	11/29/15-11/30/15		AGES Geo	logist:	Joe Webster	
Drilling Method:	HSA	Coring Device Size:	NA	Hammer	Wt. 160lb	and Drop 2ft
Sampling Method:	NA	Borehole Diameter:	4.25"	Drilling	Fluid Used:	Water
Sampling Interval:	NA	Borehole Depth:	27'	Surface	Elevation:	439.85' MSL
NOTES/COMME	ENTS:					

Depth Interval (feet)	Sample Recovery (feet)	Penetration (Hyd. Pres. or Blow Counts)	Sample/Core Description	PID (PPM)
0-10		NA	Advance augers – no samples	N/A
10-12	2	2-2-2-2	Brown clay, little silt, very moist to wet	N/A
12-14	2	1-2-2-3	Brown clay, little silt, wet.	N/A
14-16	2	2-2-2-2	Brown clay, little silt, very moist to wet	N/A
16-18	2	2-3-2-2	Brown to olive gray clay, little silt, trace sand, very moist to wet	N/A
18-20	1.33	1-1-2-1	Olive gray clay, some silt, wet	N/A
20-22	2	2-2-3-2	Olive gray clay, some silt, wet	N/A
22-24	2	WH-WH-2-2	Gray clay, some silt, trace fine sand, moist to wet	N/A
24-26	2	1-1-2-2	Gray clay, some silt, trace fine sand, moist	N/A
26-27	0.1	10-50/1	Brown to gray weathered shale with limestone	N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A

### WELL NO. CF-15-05

Project Number:	2015067
	Clifty Creek Plant –
Project Location:	Landfill South End
3	
Installation Date(s):	11/29/15-12/1/15
Drilling Method:	Hollow Stem Auger
Drilling Contractor:	Bowser Morner
Development Date(s):	12/16/15
(-/·	
Development Method:	Peristaltic Pump, Bailer
Field parameters stabilize	ed.
Turbidity = 4.28 NTUs	
Volume Purged:	46 gallons
Static Water-Level*	11.23'
Top of Well Casing Elev	vation: 442.58'
Well Purpose:	
Groundawter Monitoring	
Northing (Y): 447491.9	
Easting (X): 565533.64	•
Comments/Notes:	
2 inch PVC riser and scre	een
	ed well screen with an inner
	lean quartz sand and an outer
layer of food-grade nylor	n mesh.
Inspector: Joe Webste	er
<del></del>	

со	NSTRUCTION MATERIALS USED:
	Bags of Sand
	Bags/Buckets Bentonite Pellets
	Bags Portland for Grout
	Bags Concrete/Sakrete



#### BORING NO. <u>CF-15-06</u> SAMPLE/CORE LOG

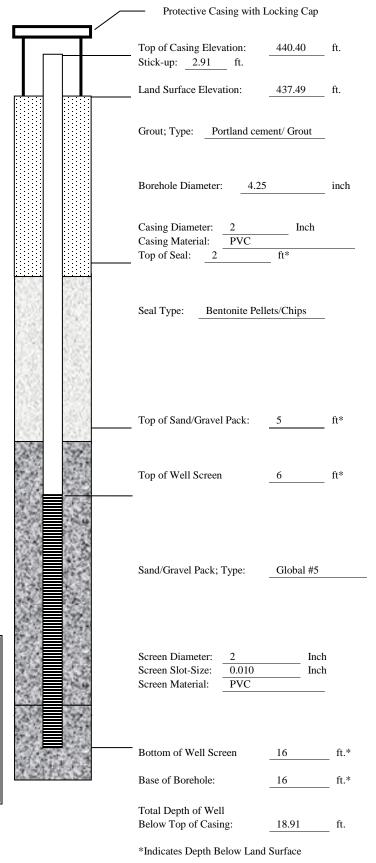
Project Number:	2015067 Clifty Creek Plant		Log Page	1	of	1
Project Location:	Landfill South End		Drilling Co	ntractor:	Bowser Morn	er
Drilling Date(s):	11/29/15-11/30/15		AGES Geo	logist:	Joe Webster	
Drilling Method:	HSA	Coring Device Size:	NA	Hammer	: Wt. 160lb	and Drop 2ft
Sampling Method:	NA	Borehole Diameter:	4.25"	Drilling	Fluid Used:	Water
Sampling Interval:	NA	Borehole Depth:	16'	Surface	Elevation:	437.49' MSL
NOTES/COMME	ENTS:					

Depth Interval (feet)	Sample Recovery (feet)	Penetration (Hyd. Pres. or Blow Counts)	Sample/Core Description	PID (PPM)
0-10		NA	Advance augers – no samples	N/A
10-12	1.5	3-3-3-5	Brown clay, some silt, soft, moist	N/A
12-14	1.7	3-3-4-3	Brown clay, little silt, soft, moist	N/A
14-16	0.8	4-7-46-50/4	Gray to brown, weathered shale with limestone, hard, dry	N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A

### WELL NO. CF-15-06

Project Number:	2015067			
Project Location:	Clifty Creek Plant – Landfill South End			
Installation Date(s):	11/29/15-11/30/15			
Drilling Method: Drilling Contractor:	Hollow Stem Auger Bowser Morner			
Development Date(s):	12/16/15			
Development Method: Field parameters stabilize Turbidity = 5.59 NTUs	Peristaltic Pump, Bailer			
Volume Purged:	6.95 gallons			
Static Water-Level*	17.65'			
Top of Well Casing Elev	ation: 440.40'			
Well Purpose: Groundwater Monitoring Northing (Y): 447026.93 Easting (X): 565190.31				
	ed well screen with an inner lean quartz sand and an outer			
Inspector:Joe Webste	er			

Inspector:	Joe Webster
co	NSTRUCTION MATERIALS USED:
	Bags of Sand
	Bags/Buckets Bentonite Pellets
	Bags Portland for Grout
	Bags Concrete/Sakrete



#### BORING NO. <u>CF-15-07</u> SAMPLE/CORE LOG

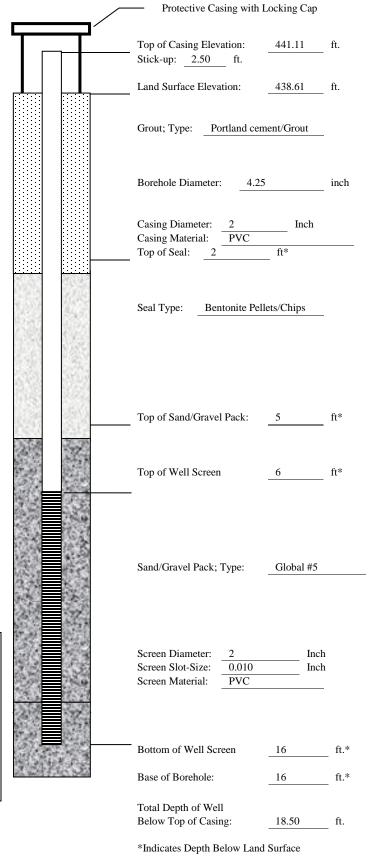
Project Number:	2015067 Clifty Creek Plant		Log Page	1	of1	·
Project Location:	Landfill South End		Drilling Contractor: Bowser Morner			er
Drilling Date(s):	11/19/15-11/23/15		AGES Geo	logist:	Joe Webster	
Drilling Method:	HSA	Coring Device Size:	NA	Hammer	Wt. 160lb.	and Drop 2ft
Sampling Method:	NA	Borehole Diameter:	4.25"	Drilling	Fluid Used:	Water
Sampling Interval:	NA	Borehole Depth:	16'	Surface 1	Elevation:	438.61' MSL
NOTES/COMMI	ENTS:					

Depth Interval (feet)	Sample Recovery (feet)	Penetration (Hyd. Pres. or Blow Counts)	Sample/Core Description	PID (PPM)
0-10		NA	Advance augers – no samples	N/A
10-12	1	1-1-3-5	Brown silty clay, stiff, plastic, wet	N/A
12-14	0	NA	No recovery	N/A
14-16	1.35	3-2-2-2	Brown gray silty clay with mottling, trace gravel, stiff, moist	N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A

### WELL NO. CF-15-07

Project Number:	2015067
	Clifty Creek Plant –
Project Location:	Landfill South End
<b>J</b>	
Installation Date(s):	11/23/15
Drilling Method:	Hollow Stem Auger
Drilling Contractor:	Bowser Morner
Development Date(s):	12/15/15
Development Date(s).	12/13/13
Development Method:	Peristaltic Pump, Bailer
Field parameters stabilize	ed.
Turbidity = 4.42 NTUs	
Volume Purged:	12.5 gallons
, oranie i argear	1210 ganono
Static Water-Level*	5.92'
Top of Well Casing Elev	vation: 441.11'
Well Purpose:	
Groundwater Monitoring	
Northing (Y): 443135.03	
Easting (X): 562259.25	<u>'</u>
Comments/Notes:	
2 inch PVC riser and scre	een
	ed well screen with an inner
filter pack of 0.40 mm c	lean quartz sand and an outer
layer of food-grade nylor	n mesh.
Inspector: Joe Webste	er

C	ONSTRUCTION MATERIALS USED:
	Bags of Sand
	Bags/Buckets Bentonite Pellets
	Bags Portland for Grout
	Bags Concrete/Sakrete



Project Number:	2015067 Clifty Creek Plant		Log Page	1	of	1
Project Location:	Landfill South End		Drilling Co	ntractor:	Bowser Mor	ner
Drilling Date(s):	11/17/15-11/19/15		AGES Geo	logist:	Mike Gelles	
Drilling Method:	HSA	Coring Device Size:	NA	Hammer	Wt. 160lb	and Drop 2ft
Sampling Method:	NA	Borehole Diameter:	4.25"	Drilling	Fluid Used:	Water
Sampling Interval:	NA	Borehole Depth:	40'	Surface	Elevation:	460.33' MSL
NOTES/COMME	ENTS:					

Depth Interval (feet)	Sample Recovery (feet)	Penetration (Hyd. Pres. or Blow Counts)	Sample/Core Description	PID (PPM)
0-10		NA	Advance augers – no samples	N/A
10-12	2	3-6-6-7	Orange brown silty clay, fine sand, slightly plastic, moist	N/A
12-14	1.4	5-7-10-10	Light brown silt, loose, moist	N/A
14-16	1.6	4-8-12-10	Light brown silt, loose, moist	N/A
16-18	1.6	7-6-9-7	Light brown silt, loose, moist	N/A
18-20	1.6	3-6-4-4	18'-19' Light brown silt, loose, moist; 19'20' Light brown silt, loose, wet	N/A
20-22	1.2	2-3-6-6	Light brown silt, trace clay, wet	N/A
22-24	0.1	2-3-3-3	Brown silt, clay, wet	N/A
24-26	2	2-4-6-7	Brown silt, clay, wet	N/A
26-28	2	3-5-5-5	Brown fine and medium sand, trace silt, trace clay, wet	N/A
28-30	2	3-5-9-12	Brown fine and medium sand, trace silt, trace clay, wet	N/A
30-32	1.2	1-2-2-2	Brown fine and medium sand, medium gravel, trace silt, trace clay, wet	N/A
32-34	2	4-5-5-9	Brown fine and medium sand, fine and medium gravel, trace silt, trace clay, wet	N/A
34-36	2	WH-3-6-8	Brown fine and medium sand, fine and medium gravel, trace silt, trace clay, wet	N/A
36-38	2	4-5-7-8	Brown fine and medium sand, fine and medium gravel, trace silt, trace clay, wet	N/A
38-40	2	3-5-5-11	38'-39.75' Brown fine and medium sand, fine and medium gravel, trace silt, trace clay, wet; 39.75'-40' gray fine and medium sand, silt, trace clay, wet	N/A

#### WELL CONSTRUCTION LOG WELL NO. CF-15-08

2015067 Project Number: Clifty Creek Plant -Landfill South End Project Location: Installation Date(s): 11/17/15-11/19/15 Drilling Method: Hollow stem Auger Drilling Contractor: Bowser Morner Development Date(s): 12/8/15 Development Method: Submersible Pump Field parameters stabilized. Turbidity = 2.16 NTUs Volume Purged: 100 gallons Static Water-Level\* 24.31' Top of Well Casing Elevation: 462.79' Well Purpose: Groundwater Monitoring Northing (Y): 443219.57 Easting (X): 562537.29 Comments/Notes: 2 inch PVC riser and screen 10 ft of 0.010 pre-packed well screen with an inner filter pack of 0.40 mm clean quartz sand and an outer layer of food-grade nylon mesh. Inspector: Michael Gelles

Protective Casing with Locking Cap Top of Casing Elevation: 462.79 ft. Stick-up: 2.46 ft. ft. Land Surface Elevation: 460.33 Grout; Type: Portland cement/ Grout Borehole Diameter: 4.25 inch Casing Diameter: Inch Casing Material: Top of Seal: Seal Type: Bentonite Pellets/Chips Top of Sand/Gravel Pack: Top of Well Screen Sand/Gravel Pack; Type: Global #5 Screen Diameter: Inch Screen Slot-Size: 0.010 Inch Screen Material: PVC Bottom of Well Screen 40 ft.\* ft.\* Base of Borehole: Total Depth of Well Below Top of Casing: 42.46 ft. \*Indicates Depth Below Land Surface

4.5 Bags of Sand

0.5 Bags/Buckets Bentonite Pellets

3 Bags Portland for Grout

Bags Concrete/Sakrete

#### BORING NO. <u>CF-15-09</u> SAMPLE/CORE LOG

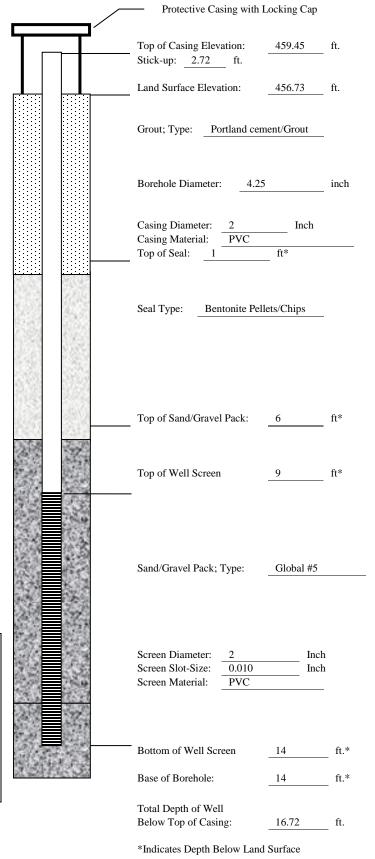
Project Number:	2015067 Clifty Creek Plant		Log Page	1	of	1
Project Location:	Landfill South End		Drilling Contractor: Bowser Morner			er
Drilling Date(s):	11/24/15-11/25/15		AGES Geo	logist:	Joe Webster	
Drilling Method:	HSA	Coring Device Size:	NA	Hammer	Wt. NA	and Drop NA
Sampling Method:	NA	Borehole Diameter:	4.25"	Drilling	Fluid Used:	Water
Sampling Interval:	NA	Borehole Depth:	14'	Surface 1	Elevation:	456.73' MSL
NOTES/COMMI	ENTS:					

Depth Interval (feet)	Sample Recovery (feet)	Penetration (Hyd. Pres. or Blow Counts)	Sample/Core Description	PID (PPM)
0-10		NA	Advance augers – no samples	N/A
10-12	1.8	5-9-6-9	Brown weathered shale wilt limestone, hard, dry	N/A
12-14	0.2	50/4	Brown weathered shale wilt limestone, hard, dry	N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A
				N/A

# WELL NO. CF-15-09

Project Number:	2015067
	Clifty Creek Plant –
Project Location:	Landfill South End
Installation Date(s):	11/24/15-11/25/15
Drilling Method:	Hollow Stem Auger
Drilling Contractor:	Bowser Morner
Development Date(s):	12/16/15
Development Method:	Peristaltic Pump, Bailer
Field parameters stabilize Turbidity = 3.21 NTUs	ed.
Turbidity = 3.21 NTUS	
Volume Purged:	6 gallons
Static Water-Level*	12.18'
Top of Well Casing Elev	ation: _459.45'
Well Purpose: Groundwater Monitoring	,
Northing (Y): 443445.96	
Easting (X): 562871.69	
Comments/Notes:	
2 inch PVC riser and screen	
	d well screen with an inner
	lean quartz sand and an outer
layer of food-grade nylor	ı mesn.
Inspector: Joe Webste	er
	<del></del>

C	CONSTRUCTION MATERIALS USED:
	_ Bags of Sand
	Bags/Buckets Bentonite Pellets
	Bags Portland for Grout
	Bags Concrete/Sakrete



# APPENDIX C WELL DEVELOPMENT DATA

#### TABLE C-1 SUMMARY OF WELL DEVELOPMENT DATA KYGER CREEK PLANT GALLIA COUNTY, OHIO

				Final Turbidity
Well/ Piezometer  Type I Residual Waste	Dates  e Landfill and Landfill Runoff	Method Collection Pone	Volume (gal)	(NTU)
CF-15-01	12/08/2015 - 12/14/2015	Pump/Bail	33	3.12
CF-15-02	12/04/2015 - 12/7/2015	Pump/Bail	115	3.69
CF-15-03	12/04/2015 - 12/15/2015	Pump/Bail	32	3.99
CF-15-04	12/9/2015	Pump	65	0.91
CF-15-05	12/9/2015 - 12/16/2015	Pump	46	4.28
		Î		
CF-15-06	12/09/2015 - 12/18/2016	Pump/Bail	21	9.59
CF-15-07	12/08/2015 - 12/15/2015	Pump/Bail	13	4.42
CF-15-08	12/8/2015	Pump	100	2.16
CF-15-09	12/08/2015 - 12/16/2015	Pump/Bail	6	3.21
West Boiler Slag Pond				
WBSP-15-01	12/03/2015 - 12/17/2015	Pump/Bail	23	70.8
WBSP-15-02	12/03/2015 - 12/15/2015	Pump	31.5	3.48
WBSP-15-03	12/09/2015 - 12/15/2015	Pump/Bail	15	2.42
WBSP-15-04	12/02/2015 - 12/08/2015	Pump	110	1.37
WBSP-15-05	12/02/2015 - 12/03/2015	Pump	130	1.87
WBSP-15-06	12/03/2015 - 12/09/2015	Pump	100	3.44
WBSP-15-07	12/02/2015 -12/16/2015	Pump/Bail	36	2.86
WBSP-15-08	12/02/2015 - 12/16/2015	Pump	90	4.96
WBSP-15-09	1/08/2016 - 1/19/2016	Pump	59	3.57
WBSP-15-10	1/07/2016 - 1/20/2016	Pump	33	3.59

#### APPENDIX D

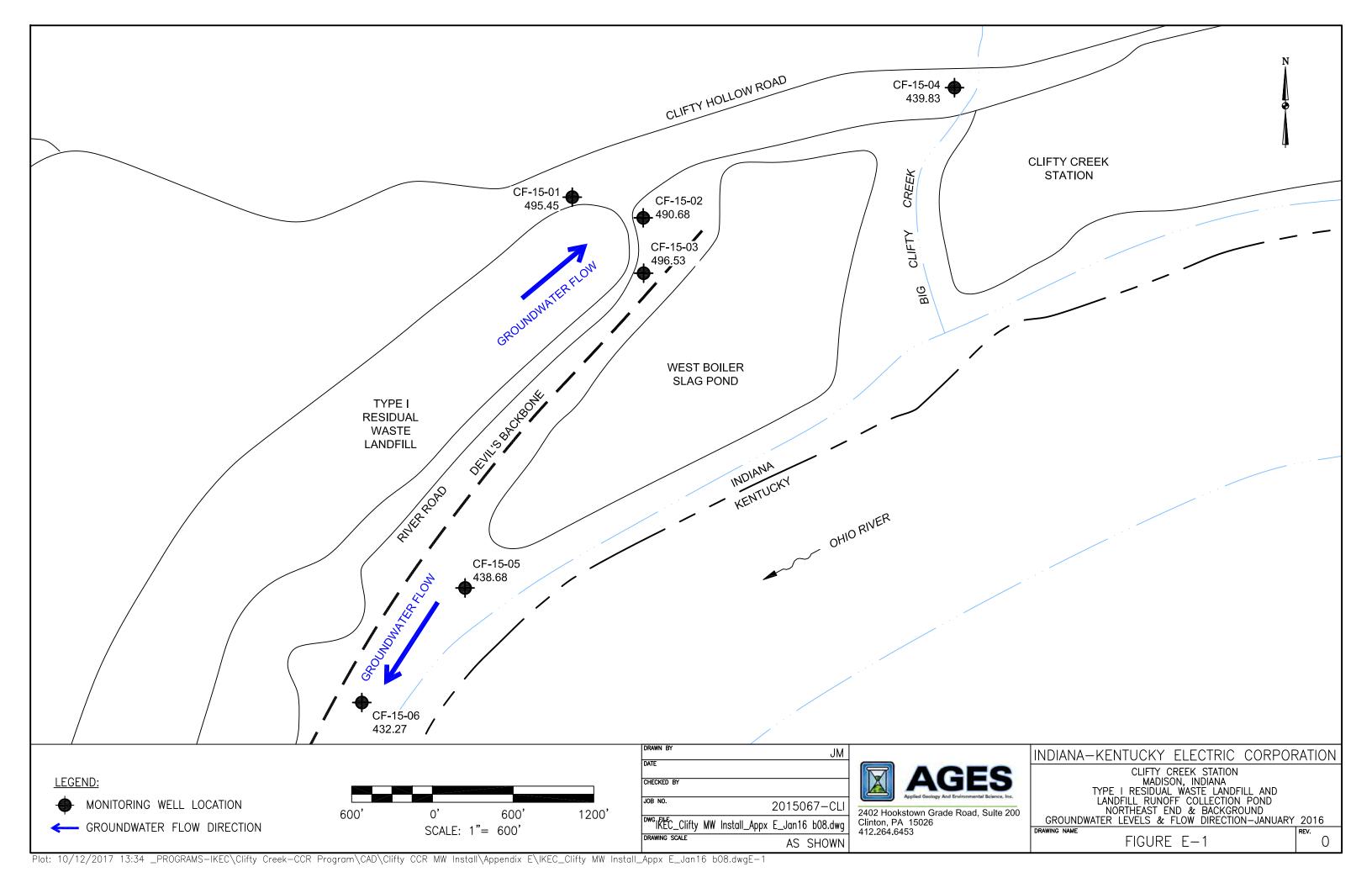
GROUNDWATER LEVELS January 2016 through May 2016

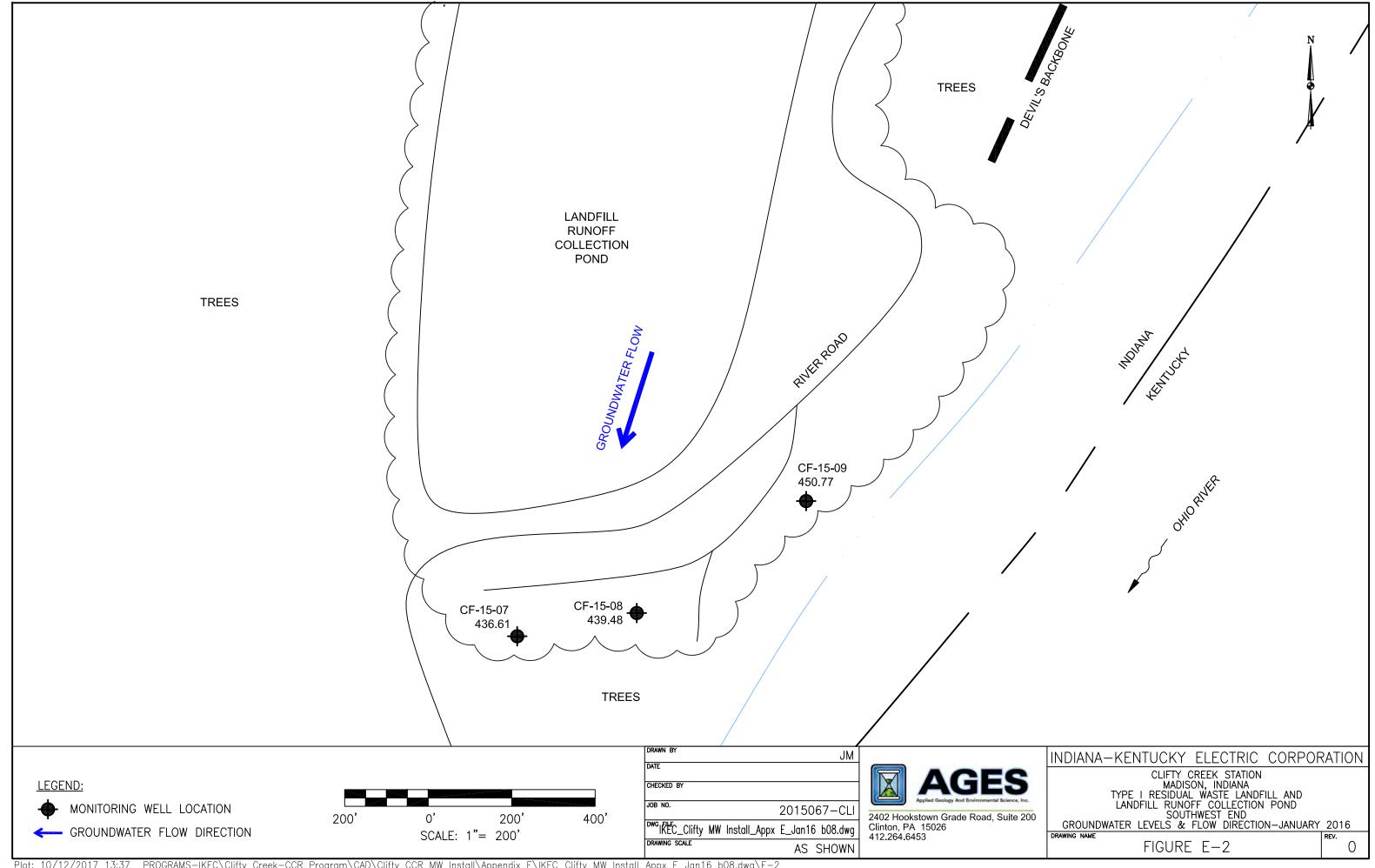
#### TABLE D-1 CLIFTY CREEK CREEK PLANT SUMMARY OF GROUNDWATER ELEVATION DATA JANUARY 2016 - MAY 2016

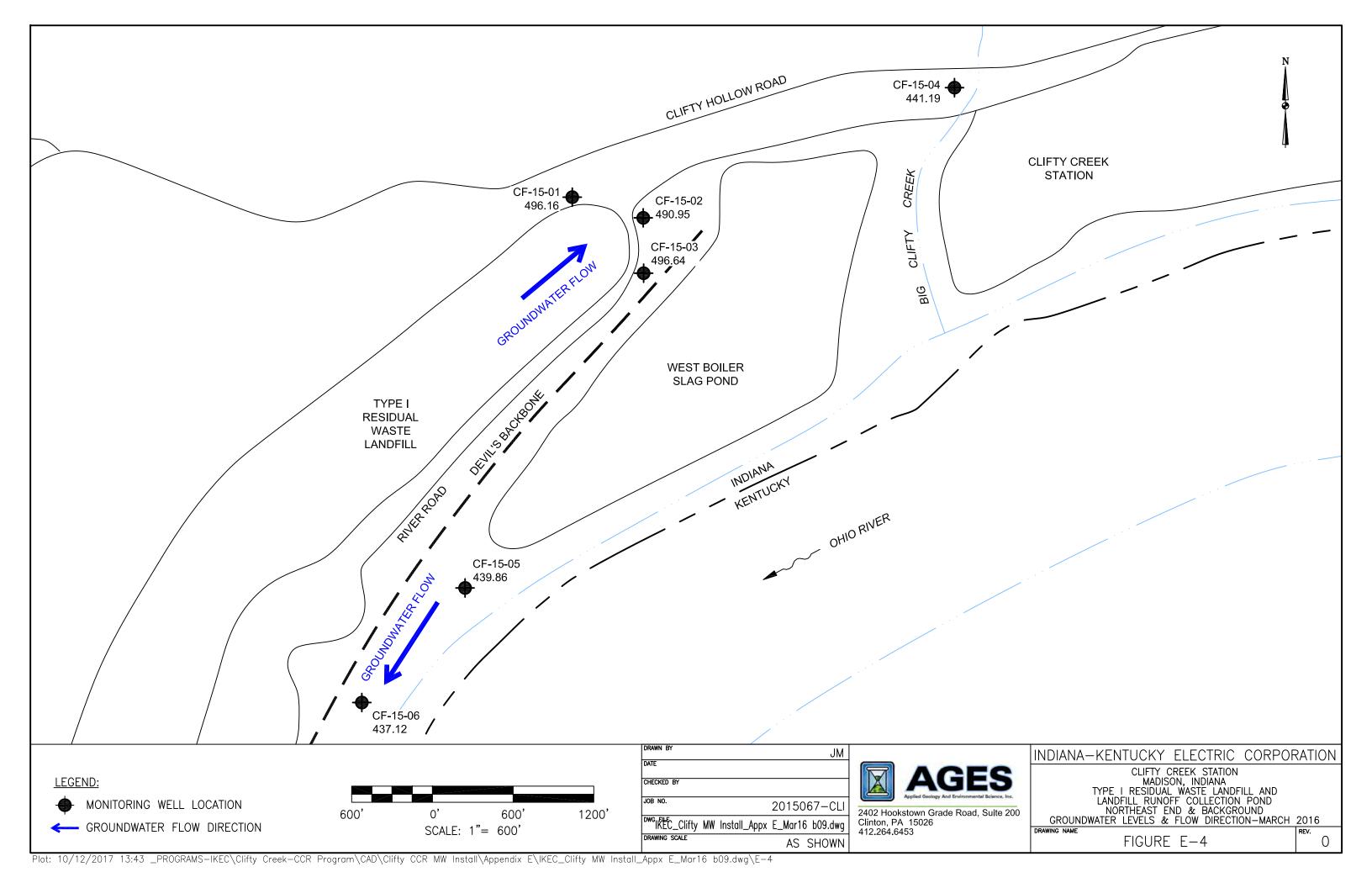
Monitoring Well Designation	Jan-16 Groundwater Elevation (ft)	Mar-16 Groundwater Elevation (ft)	May-16 Groundwater Elevation (ft)		
LANDFILL AND LAND	` ,	` '	Elevation (It)		
CF-15-01	495.45	496.16	496.35		
CF-15-02	490.68	490.95	490.97		
CF-15-03	496.53	496.64	496.38		
CF-15-04	439.83	441.19	441.27		
CF-15-05	438.68	439.86	436.25		
CF-15-06	432.27	437.12	429.22		
CF-15-07	436.61	438.08	437.48		
CF-15-08	439.48	440.54	440.88		
CF-15-09	450.77	451.58	450.69		
WEST BOILER SLAG	POND				
WBSP-15-01	451.72	453.01	453.27		
WBSP-15-02	468.31	472.52	471.52		
WBSP-15-03	477.03	477.11	477.62		
WBSP-15-04	429.22	436.25	424.96		
WBSP-15-05	428.95	436.12	424.84		
WBSP-15-06	428.82	436.06	424.77		
WBSP-15-07	429.72	430.41	430.88		
WBSP-15-08	434.03	434.62	434.81		
WBSP-15-09	432.17	430.39	432.21		
WBSP-15-10	431.41	433.28	432.58		

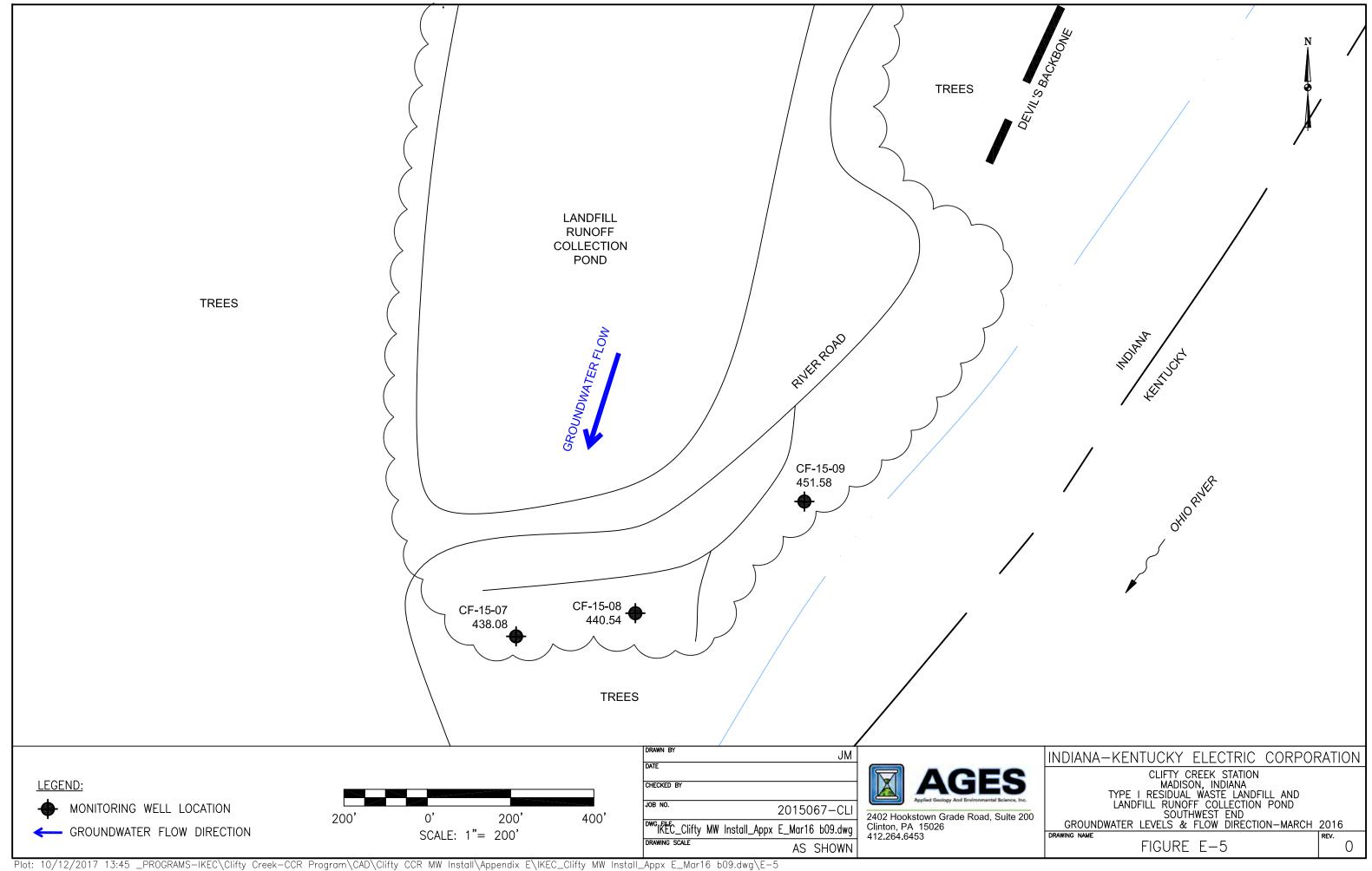
#### APPENDIX E

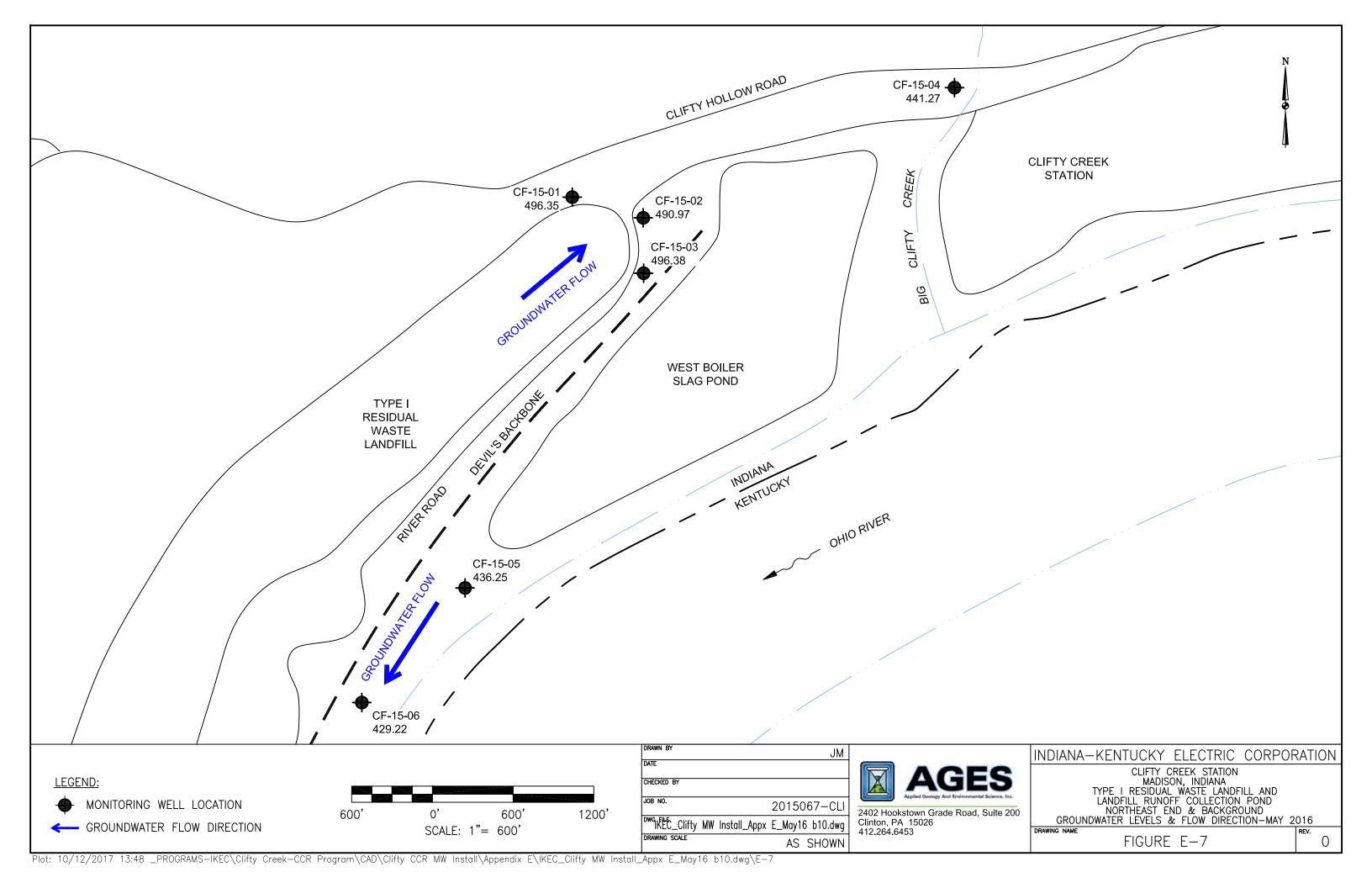
GROUNDWATER CONTOUR MAPS January 2016 through May 2016

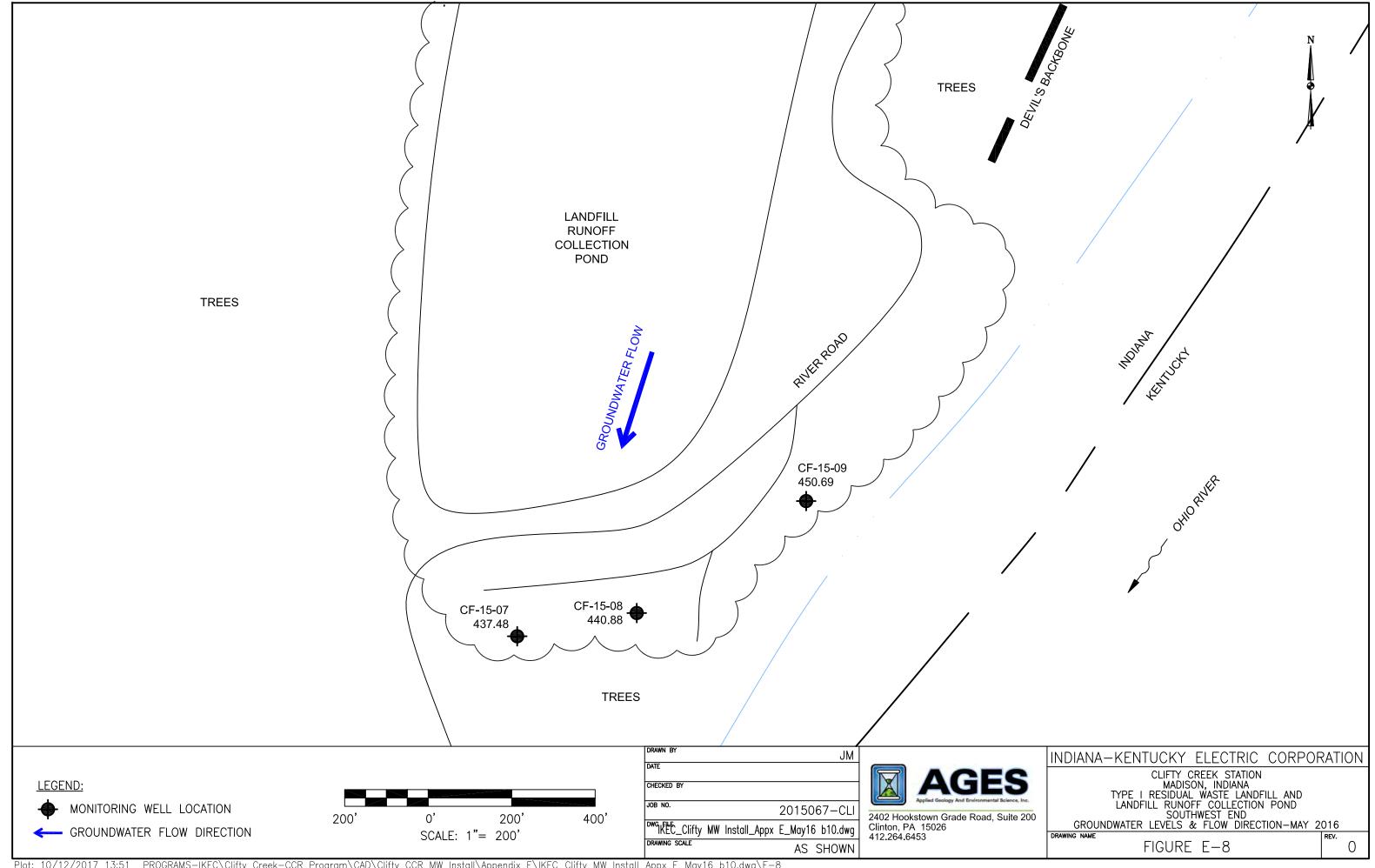




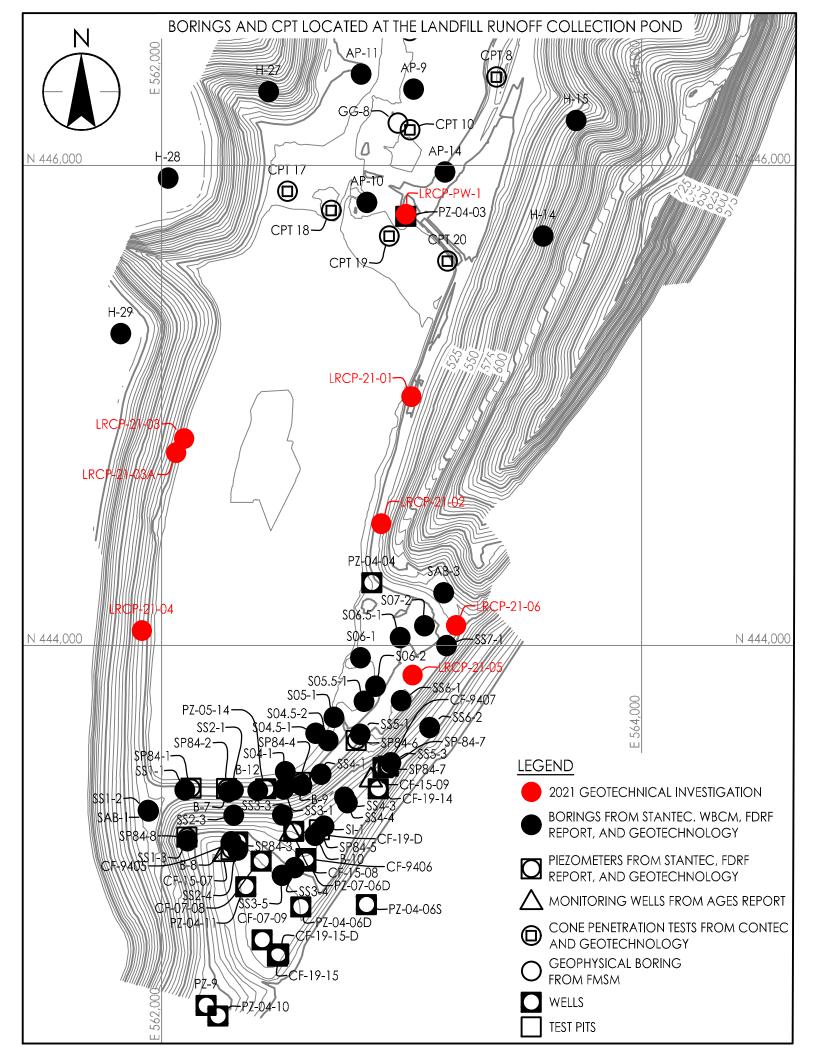








Stantec (2021)





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Oliana D	1	doublification I DCD 21	_ <b>_</b> 1				Otamba - D	dan as N.C.	DCD 24 04
	orenole I	dentification LRCP-21	1-U I		Doring Lagar			_	_RCP-21-01
Client		IKEC			Boring Locat	_	45036.29 N;		_
'		175539026			Surface Elev	_			Datum NAVD88
1	-	Clifty Creek WBSP a			Date Started	-		Complete	
	•	Clifty Creek Power Pla	ant, Madi	son, IN	Depth to Wa	-		ate/Time	
Logged	-				Depth to Wa			oate/Time	
Drilling (	Contracto	or Stantec Consultin	g Service	es Inc.	Drill Rig Typ	e and ID	CME 55 T	rack Rig	#711
Overbur	den Drill	ing and Sampling Tool	and Size)	4.25" HSA,	2" Split	Spoon w/o I	iners, 3" \$	Shelby Tubes	
Rock Dr	illing and	d Sampling Tools (Type	e and Siz	e) <u>N/A</u>					
Sampler	r Hamme	er Type Automatic	Weigh	nt <u>140</u>	lb Drop	30 in	Effic	iency _	88 % (Avg.)
Borehole	e Azimut	h N/A (Vertical)			Borehole In	clination	(from Vertic	cal)	Vertical
Litholo			Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %	
Elevation	Depth		Rock Core	RQD	Run	Rec. Ft.	Rec. %	Depth	Remarks
499.5	0.0	Top of Hole							_
-		SAND WITH GRAV (SW), CCR, dark br	own	SPT-1	0.0 - 1.5	1.3	4-8-10		_
		with black, moist, loo medium dense, with							
		chunks	Coal						
-									-
-									-
L									_
				SPT-2	5.0 - 6.5	1.5	4-3-3		_
-									-
-									-
-									-
489.5	10.0								_
		SILTY CLAY (CL-MI dark brownish gray,		SPT-3	10.0 - 11.5	1.5	WH-1-1		
		very soft	wci,		_				
									-
9									-
									_
484.5	15.0								
25		FAT CLAY (CH), ora		SPT-4	15.0 - 16.5	1.5	2-2-3		_
		brown mottled with t moist, medium stiff t		01 1-4	15.0 - 10.5	1.5	2-2-0		-
_		stiff, medium to high							-
		plasticity							_
S S S S S S S S S S S S S S S S S S S									
ā Q									
			Stant	oo Conou	Iting Services	Inc	1	1	4/12/2



Client Bo	orehole l	dentification_LRCP-2	21-01				Stantec Bori	ng No. <b>L</b>	RCP-21-01
Client		IKEC			Boring Locat	tion 4	45036.29 N;	-	
Project N	Number	175539026		Surface Elev	ation 4	99.5 ft E	levation [	Datum_NAVD88	
Litholo	gy		Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %	
Elevation	Depth	Description	Rock Core	RQD	Run	Rec. Ft.	Rec. %	Depth	Remarks
-		FAT CLAY (CH), or brown mottled with moist, medium stiff stiff, medium to high plasticity (Continual)	tan, f to very th	SPT-5	20.0 - 21.5	1.4	6-7-8		
- 473.6	25.9	-at 25.0', wet		SPT-6	25.0 - 26.3	1.2	11-11-50/4"		
473.2	26.3	SHALE, gray, mois wet, friable Auger Refusal / Bottom of Hole	st to						
Borehole was backfilled with a mixture of cement-bentonite grout from the bottom of hole to the ground surface using a tremie pipe.								of hole to the	
-		ground sandoe do	ng a tronii	о ріре.					
_									
_									
_									
-									
-									
-									
_									



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Client Borehole Identification LRCP-21-02 Stantec Boring No. LRCP-21-02										-02
Client		IKEC			Boring Local	tion 4	44507.211 <b>N</b>	N; 562914	.551 E	
Project I	Number	175539026			Surface Elev	ation 5	01.6 ft E	levation [	Datum N	AVD88
Project I	Name	Clifty Creek WBSP	and LRCP	Closure	Date Started	I 3	/14/21	Completed	d 3/	14/21
Project	Location	Clifty Creek Power P	lant, Madi	son, IN	Depth to Wa	iter N	N/A Date/Time		• N	/A
Logged	by B. F	Herries			Depth to Wa	iter N	I/A [	ate/Time	• N	/A
Drilling (	 Contract	or Stantec Consult	ing Service	es Inc.	Drill Rig Typ	e and ID	O CME 55 T	rack Rig i	#711	
Overbur	den Drill	ing and Sampling To	ols (Type a	and Size)	4.25" HSA,	2" Split	Spoon w/o I	iners, 3" S	Shelby Tu	bes
Rock Dr	illing and	d Sampling Tools (Ty	pe and Siz	e) N/A						
	•	er Type Automatic	Weigh	· -	lb Drop	30 in	Effic	iency	88 % (Av	g.)
Borehole					<u> </u>		—— ı (from Vertio	-	Vertical	<u> </u>
Litholo			Overburden	Sample #		Rec. Ft.	Blows/	NMC %		
Elevation	Depth	Description	Rock Core	RQD	Run	Rec. Ft.	Press.(psi) Rec. %	Depth	Re	marks
501.6	0.0	Top of Hole								
501.3	0.3	TOPSOIL		ODT 4	00.45	4.5	0.07			
-		SILTY SAND (SM) brown with black, r		SPT-1	0.0 - 1.5	1.5	2-6-7			
		medium dense, wit	th coal							
<u> </u>		pieces of about 3/1 3/4", possible CCR	6" to							
_		-clay from 1.3' to 1								
- 407.4	4.5									
497.1	4.5	GRAVEL (GP), wh	ito to							
<b>-</b>		gray, dry, dense, m								-
L		coarse to coarse		SPT-2	5.0 - 6.5	1.0	30-12-20			
495.1	6.5									
-		LEAN CLAY (CL), brown and tan, mo	light ist vorv							
		stiff, some gravel	ist, very							
-		-at 8.0', cobbles fo	r 6"	SPT-3	7.5 - 9.0	1.0	8-15-22			
L		at 0.0 , 0000.00 10	. •							
_										_
				SPT-4	10.0 - 11.5	1.4	5-11-16			
<u> </u>				01 1-4	10.0 - 11.0	1.4	0-11-10			
-										
<u> </u>										



	Client Bo	orehole l	dentification_LRCP-2	21-02				Stantec Bo	ring No. <b>L</b>	RCP-21-02	
	Client		IKEC			Boring Locat	tion 4	44507.211 I	N; 562914	.551 E	
	Project N	Number	175539026			Surface Elev	ation 5	01.6 ft E	Elevation [	Datum NAVD8	8
	Litholo	ogy		Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %		
F	Elevation	Depth	Description	Rock Core	RQD	Run	Rec. Ft.	Rec. %	Depth	Remarks	
-			LEAN CLAY (CL), brown and tan, mo stiff, some gravel (Continued) -at 15.0', medium		SPT-5	15.0 - 16.5	1.5	7-11-12			<u>-</u>
-			plasticity								_
	404.0	00.0									
	SHALE, light gray to gray, moist, soft rock, horizontal bedding, some fractures		SPT-6	20.0 - 20.3	0.3	50/4"			_		
-			bedding, some na	ciares							_
-											-
-											-
F	476.2	25.4			SPT-7	25.0 - 25.4	0.4	50/5"			
-			Auger Refusal / Bottom of Hole								-
-											-
от 4/12/21 I			Borehole was back ground surface us	kfilled with ing a tremi	a mixture e pipe.	of cement-be	entonite (	grout from th	ne bottom	of hole to the	_
HGRAPHIC LOG.GE										_	
LOGS.GPJ FMSN											_
CLIFTY_BORINGS											-
RO BORING LOG CLIFTY_BORINGS_LOGS GPJ FMSM-GRAPHIC LOG GDT 4/12/2											-
≸				<u> </u>		Iting Son <i>i</i> cos					4/12/21



Page: 1 of 2

Client Bo	orehole	dentification LRCP-	21-03				Stantec Bor	ring No. <b>I</b>	_RCP-21-03	
Client		IKEC			Boring Locat	tion 4	44860.837 <b>N</b>	N; 562093	3.44 E	_
Project I	Number	175539026			Surface Elev	ation 4	90.9 ft E	Elevation l	Datum_NAVD88	<u>.                                    </u>
Project I	Name	Clifty Creek WBSP	and LRCP	Closure	Date Started	I <u>3</u>	/9/21 (	Complete	d3/9/21	
Project	Location	Clifty Creek Power F	Plant, Madi	son, IN	Depth to Wa	ter N	I/A [	Date/Time	N/A	
Logged	by B. F	Herries			Depth to Wa	ter N	/A   [	Date/Time	e N/A	
Drilling (	Contract	or Stantec Consul	ting Service	es Inc.	Drill Rig Typ	e and ID	CME 55 T	rack Rig	#711	
Overbur	den Drill	ing and Sampling To	ools (Type a	and Size)	4.25" HSA,	2" Split	Spoon w/o I	iners, 3"	Shelby Tubes	
Rock Dr	illing and	d Sampling Tools (Ty	/pe and Siz	e) <u>N/A</u>						
Samplei	r Hamme	er Type _Automatic	Weigh	nt 140	lb Drop	30 in	Effic	iency _	88 % (Avg.)	
Borehole	e Azimut	th N/A (Vertica	al)		Borehole In	clination	(from Verti	cal)	Vertical	
Litholo	pgy	-	Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %		
Elevation	Depth	Description	Rock Core	RQD	Run	Rec. Ft.	Rec. %	Depth	Remarks	
490.9	0.0	Top of Hole								
		FAT CLAY (CH), I brown with brown		SPT-1	0.0 - 1.5	0.5	WH-WH-1			
-		little green, moist, soft to very stiff, tra								
-		gravel, roots from								
		1.5'								
_										
L										-
				SPT-2	5.0 - 6.5	1.2	4-4-6			
_		-at 5.7', roots and matter from 5.7' to		0 2	0.0 0.0	1.2	1 1 0			
_		matter from 6.7 te	0.0							
-										
_										
  -										_
				SPT-3	10.0 - 11.5	1.5	7-11-13			
_		-at 10.9', gravel ar	nd		10.0 11.0	1.0	7 11 10			
_		cobbles from 10.9 11.5'	' to							
		11.0								
_										
-										



Client Bo	orehole l	dentification LRCP-	21-03				Stantec Bori	na No. <b>L</b>	RCP-	21-03
Client		IKEC			Boring Locat	ion 4	44860.837 N			
Project N	Number	175539026			Surface Elev					NAVD88
Litholo	gy		Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %		
Elevation	Depth	Description	Rock Core	RQD	Run	Rec. Ft.	Rec. %	Depth		Remarks
-		FAT CLAY (CH), light brown with brown and little green, moist, very soft to very stiff, trace gravel, roots from 0' to 1.5' (Continued) -at 15.6', gray with brown		SPT-4	15.0 - 16.5	1.5	13-22-18			<del>-</del> -
- - 471.7	19.2	gravelly clay seam 15.6' to 16.5'	ı from	Grab-1	18.0 - 19.0		Grab-1-no SPT			-
_	10.2	Auger Refusal / Bottom of Hole				ı	1	<u>ı</u>		_
_										_
_										_
		Dorobolo was bas	kfill od with	a mistura	of coment he	ntonito :	arout from th	o hottom	of bolo	to the
		Borehole was bac ground surface us	ing a tremi	e pipe.	or cement-be	monite (	grout from th	e bollom (	oi noie	to trie -
_										-
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_										_
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_										



### **SUBSURFACE** LOG (DRAFT) Page: 1 of 2

Client Bo	orehole I	dentification LRCP-	21-03A				Stantec Bo	oring No. <b>L</b>	RCP-21-03A
Client IKEC					Boring Locat	tion 4	44802.654	N; 562060	.15 E
Project N	Number	175539026			Surface Elev	ation 4	97.1 ft	Elevation [	Datum_NAVD88_
Project N	Name	Clifty Creek WBSP	and LRCP	Closure	Date Started	3	/10/21	Completed	d3/10/21
Project	Location	Clifty Creek Power F	Plant, Madi	son, IN	Depth to Wa	ter N	/A	Date/Time	N/A
Logged	by <u>B</u> . F	lerries			Depth to Wa	ter N	/A	Date/Time	N/A
Drilling C	Contracto	or Stantec Consul	ting Service	es Inc.	Drill Rig Typ	e and ID	CME 55	Track Rig #	<del>#</del> 711
Overburden Drilling and Sampling Tools (Type and Size				and Size)	4.25" HSA,	2" Split	Spoon w/o	liners, 3" §	Shelby Tubes
Rock Dr	illing and	d Sampling Tools (Ty	pe and Siz	e) N/A					
Sampler	· Hamme	er Type _Automatic	Weigh	nt 140	lb Drop	30 in	Effi	ciency _	88 % (Avg.)
Borehole	e Azimut	h N/A (Vertica	al)		Borehole In	clination	(from Vert	ical)	Vertical
Litholo	gy		Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %	
Elevation	Depth	Description	Rock Core	RQD	Run	Rec. Ft.	Rec. %	Depth	Remarks
497.1	0.0	Top of Hole							_
		FAT CLAY (CH), the with dark brown, n		SPT-1	0.0 - 1.5	1.0	6-7-2		
-		stiff to very stiff, tra	ace to		0.0 1.0	1.0	0 / 2		-
_		some gravel, som from 0' to 1.5'	e roots						_
_									-
_									-
_									_
		-at 5.0', light brown	n and	SPT-2	5.0 - 6.5	1.1	2-3-14		
_		2.2		01 1-2	0.0 - 0.0	1	2-0-14		-
_									_
_									-
_									-
_									_
				SPT-3	10.0 - 11.5	1.0	8-9-13		
-					10.0 11.0	1.0			-
_									_
-									-
_									
									_



Client B	Client Borehole Identification LRCP-21-03A Stantec Boring No. LRCP-21-03A									
Client		IKEC			Boring Locat	tion 4	44802.654 N	; 562060	.15 E	
Project I	Number	175539026			Surface Elev	ation 49	97.1 ft E	levation [	Datum_NAVD88_	
Litholo	ogy		Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %		
Elevation	Depth	Description	Rock Core	RQD	Run	Rec. Ft.	Rec. %	Depth	Remarks	
481.8	15.3								_	
481.2	15.9	SHALE, light gray	with	SPT-4	15.0 - 15.9	0.9	41-50/5"			
		white, dry, hightly fractured, horizont bedding	al						_	
_		Auger Refusal / Bottom of Hole							=	
-									-	
_		Borehole was bac ground surface us	kfilled with ing a tremi	a mixture e pipe.	of cement-be	ntonite g	grout from the	e bottom	of hole to the	
-									_	
-									-	
-									-	
-									-	
-									_	
-									_	
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L									_	



### **SUBSURFACE** LOG (DRAFT) Page: 1 of 2

Client Bo	orehole I	dentification_LRCP-2	21-04				Stantec Bo	ring No. <b>L</b>	RCP-21-04
Client		IKEC			Boring Locat	ion 4	44061.327	N; 561916	5.533 E
Project I	Number	175539026			Surface Elev	ation 5	09.0 ft E	Elevation I	Datum NAVD88
Project N	Name	Clifty Creek WBSP	and LRCP	Closure	Date Started	3	/10/21 (	Complete	3/10/21
Project	Location	Clifty Creek Power F	Plant, Madi	son, IN	Depth to Wa	ter 1	5.5 ft [	Date/Time	3/10/21
Logged	by B.F	Herries			Depth to Water N/A Date/Time			Date/Time	N/A
Drilling (	Contracto	or Stantec Consult	ting Service	es Inc.	Drill Rig Typ	e and ID	CME 55 T	rack Rig	#711
Overbur	den Drill	ing and Sampling To	ols (Type a	and Size)	4.25" HSA,	2" Split	Spoon w/o	liners, 3" S	Shelby Tubes
Rock Dr	illing and	d Sampling Tools (Ty	pe and Siz	e) N/A		•	·		
	_	er Type Automatic	Weigh	· —	lb Drop	30 in	Effic	eiency	88 % (Avg.)
Borehole					Borehole In			_	Vertical
Litholo			Overburden	Sample #	Depth	Rec. Ft.	Blows/	NMC %	<u> </u>
Elevation	Depth	Description	Rock Core	RQD	Run	Rec. Ft.	Press.(psi) Rec. %	Depth	Remarks
509.0	0.0	Top of Hole							
508.5	0.5	TOPSOIL		SPT-1	0.0 - 1.5	0.5	2-4-6		
_		FAT CLAY (CH), li		01 1-1	0.0 - 1.0	0.0	2-4-0		
_		brown with tan, mo to very stiff, trace o							
_		angular to subang							
_									
_									-
_		. = 0.		SPT-2	5.0 - 6.5	1.5	4-4-8		
		-at 5.9', coarse gra	avel and						
_									
_									
_									
_									_
		-at 10.0', roots for	1"	SPT-3	10.0 - 11.5	1.1	4-8-8		
_		-at 11.0', coarse g	ravel	0 0	10.0 11.0				,
_		and cobbles for 6"	ı						
_									
495.0	14.0								
		GRAVEL WITH SA		SPT-4	14.0 - 15.5	0.6	25-7-10		
_		(GP), gray to dark moist to wet, dens		01 1 4	14.0 10.0	0.0	20710		_
_		medium dense, ar							
_		poorly graded							
_									
-									
489.0	20.0								



Client Bo	Client Borehole Identification_ LRCP-21-04 Stantec Boring No. LRCP-21-04								
Client		IKEC			Boring Locat	tion 4	44061.327 N	I; 561916	.533 E
Project I	Number	175539026			Surface Elevation 509.0 ft Elevation Datum NAVD88				Datum_NAVD88
Litholo	ogy		Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %	
Elevation	Depth	Description	Rock Core	RQD	Run	Rec. Ft.	Rec. %	Depth	Remarks
 - 487.0	22.0	FAT CLAY (CH), gray to olive, moist, very stiff, some gravel, trace sand, subangular to angular.		SPT-5	20.0 - 21.5	1.3	12-7-14		_ - -
- - 483.7	25.3	SHALE, light gray dry, highly weathe fractured, horizont bedding	red,	SPT-6	25.0 - 25.3	0.3	50/4"		_ _
_		Auger Refusal / Bottom of Hole		- OI 1-0		· 0.0			-
_									-
_									-
_									-
_		Borehole was bac ground surface us	kfilled with ing a tremi	a mixture e pipe.	of cement-be	ntonite (	grout from th	e bottom	of hole to the
									_
									_
_									_
									_
									_
_									-
  -									-
_									-
_									_
_									=
-									-
_									=
-									-
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### **SUBSURFACE** LOG (DRAFT) Page: 1 of 2

Client Bo	orehole I	dentification LRCP-2	21-05				Stantec Bo	ring No. <b>I</b>	_RCP-21-05
Client	2. 3. 1010 1	IKEC			Boring Locat	 tion 4	43875.965 N	_	
	——— Number	175539026			Surface Elev	_			Datum NAVD88
1 1	-	Clifty Creek WBSP	and LRCF	Closure					
	Project Location Clifty Creek Power Plant, Madison, IN					iter 1		Date/Time	
Logged	•	•	<u> </u>	Depth to Wa	-		Date/Time		
	-	or Stantec Consult	ing Service	es Inc.	Drill Rig Typ		O CME 55 T	rack Rig	#711
Overbur	den Drill	ing and Sampling To	ols (Type a	and Size)	4.25" HSA,	2" Split	Spoon w/o l	iners, 3"	Shelby Tubes
Rock Dr	illing and	d Sampling Tools (Ty	pe and Siz	e) N/A					
Sampler	r Hamme	er Type Automatic	Weigh	nt 140	lb Drop	30 in	Effic	eiency	88 % (Avg.)
Borehole	e Azimut	h N/A (Vertica	nl)		Borehole In	clination	(from Verti	cal)	Vertical
Litholo	ogy		Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %	
Elevation	Depth	Description	Rock Core	RQD	Run	Rec. Ft.	Rec. %	Depth	Remarks
504.8	0.0 \ 0.2 /r	Top of Hole							_
504.6	0.4	TOPSOIL	/ <u> </u>	SPT-1	0.0 - 1.5	0.7	2-4-4		_
		GRAVELLY CLAY	(CL)						
502.3	2.5	$\lnot$ with sand, light bro	wn with /-						
-		gray, moist, mediu		SPT-2	2.5 - 4.0	1.4	6-22-10		_
-		LEAN CLAY (CL), brown to tan, mois							-
L		stiff to hard, some subangular	gravel,						_
-		-at 3.6', coarse gra	avel for	SPT-3	5.0 - 6.5	1.5	13-16-26		_
		2" -at 5.0', some grav	⁄el						
		, C							
_		-at 8.0', clay is gray	y for	SPT-4	7.5 - 9.0	1.5	13-27-35		
-		1.8'							-
_									_
-				SPT-5	10.0 - 11.5	1.2	7-9-13		_
-									_
		-at 13.2, wet		SPT-6	12.5 - 14.0	1.5	11-14-24		
489.8	15.0								_
_ 409.0	15.0	SHALE, gray to da	rk grav.	SPT-7	15.0 - 15.6	0.6	43-50/1"		_
-		moist, hard, residu weathered							-
<b> </b> -		wealileleu							-
_									
	Stantec Consulting Services Inc 4/12/21								



Client B		IKEC	Boring Locat	tion 4	13875 065	N; 563044.9	050 ⊏			
					=					\/D00
	Number	175539026	T		Surface Elev		Blows/	Elevation Da	alum_NA	VD80
Litholo		5	Overburden	Sample #	Depth	Rec. Ft.	Press.(psi)	NMC %		
Elevation	Depth	Description	Rock Core	RQD	Run	Rec. Ft.	Rec. %	Depth	Rem	arks
		SHALE, gray to d moist, hard, resid weathered <i>(Con</i> -at 20.0', horizont bedding, less wea	ual soil, itinued) al	SPT-8	20.0 - 20.4 22.5 - 22.9	0.4	50/5" 50/5"			
				0. 1 0	22.0 22.0	0.1	33/3			
479.5	25.3	Auger Refusal /		SPT-10	25.0 - 25.3	0.3	50/4"			
		Borehole was bad ground surface u	ckfilled with sing a tremi	a mixture e pipe.	of cement-be	entonite (	grout from	the bottom o	of hole to t	he
		Borehole was bad ground surface u	ckfilled with sing a tremi	a mixture e pipe.	of cement-be	entonite (	grout from	the bottom o	of hole to t	he
		Borehole was bac ground surface u	ckfilled with sing a tremi	a mixture e pipe.	of cement-be	entonite (	grout from	the bottom o	of hole to t	he
		Borehole was bac ground surface us	ckfilled with sing a tremi	a mixture e pipe.	of cement-be	entonite (	grout from	the bottom o	of hole to t	he



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I Client Bo	orehole I	dentification LRCP-2	1-06				Stantec Bo	ring No. I	LRCP-21-06
Client		IKEC			Boring Locat	ion 4	44082.85 N	; 563226.	249 E
Project N	Number	175539026			Surface Elev	ation 5	06.2 ft I	Elevation	Datum NAVD88
Project N	Name	Clifty Creek WBSP a	and LRCP	Closure	Date Started	3	/14/21 (	Complete	d 3/14/21
Project	Location	Clifty Creek Power Pl	ant, Madi	son, IN	Depth to Wa	ter 2	2.5 ft I	Date/Time	3/14/21
Logged	by B.F	lerries			Depth to Wa	ter N	I/A I	Date/Time	e N/A
Drilling (	Contracto	or Stantec Consultin	ng Service	es Inc.	Drill Rig Typ	e and ID	CME 55 1	rack Rig	#711
Overbur	den Drilli	ing and Sampling Too	ols (Type a	and Size)	4.25" HSA,	2" Split	Spoon w/o	liners, 3"	Shelby Tubes
Rock Dr	illing and	d Sampling Tools (Typ	e and Siz	e) N/A		•	•		
	_	er Type Automatic		· —	lb Drop	30 in	Effic	eiency	88 % (Avg.)
Borehole					Borehole In			_	Vertical
Litholo	ogy		Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %	
Elevation	Depth	Description	Rock Core	RQD	Run	Rec. Ft.	Rec. %	Depth	Remarks
506.2	0.0	Top of Hole							_
505.6	0.6	CRUSHED STONE	E AND						
-		LEAN CLAY (CL) w							-
		gravel, tan with ligh brown, moist, stiff, a	t						
		to subangular	arigulai						
-				SPT-1	2.5 - 4.0	1.3	3-6-6		_
				SF 1-1	2.5 - 4.0	1.3	3-0-0		
-									=
									_
				SPT-2	5.0 - 6.5	1.2	3-7-5		
-		-at 5.9', gravel for 2	"	SP1-2	5.0 - 6.5	1.2	3-7-5		_
		, 3							
498.7	7.5								
_		FAT CLAY (CH), lig brown with gray to b		CDT 2	7.5 - 9.0	1.3	2-7-14		_
		gray, moist, stiff to h	nard,	SPT-3	7.5 - 9.0	1.3	2-7-14		
_		with gravel and san	d						-
									_
				SPT-4	10.0 - 11.5	1.2	6.5.0		
-		-at 10.7', olive gray	with	SP1-4	10.0 - 11.5	1.2	6-5-8		_
		blue gray							
									]
-				SPT-5	12.5 - 14.0	1 5	2-3-4		-
ł				371-3	12.5 - 14.0	1.5	∠-3-4		
<u> </u>									



Client B	orehole l	dentification_LRCP-2	21-06				Stantec Bo	ring No. <b>L</b>	RCP-21-06
Client		IKEC			Boring Locat	tion 4	44082.85 N	I; 563226.2	249 E
Project I	Number	175539026			Surface Elev	ation 5	06.2 ft	Elevation [	Datum_NAVD88_
Litholo	ogy		Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %	
Elevation	Depth	Description	Rock Core	RQD	Run	Rec. Ft.	Rec. %	Depth	Remarks
<del>-</del>	FAT CLAY (CH), light brown with gray to blueish gray, moist, stiff to hard, with gravel and sand (Continued)		SPT-6	15.0 - 16.5	0.0	6-7-8		- -	
- -		-at 17.5', olive to o gray for 1.5'	live	SPT-7	17.5 - 19.0	0.9	7-9-13		-
486.2 	20.0	LEAN CLAY (CL), brown to brown, so mottling with gray, stiff, low to mediun plasticity	ome moist,	SPT-8	20.0 - 21.5	1.5	3-4-8		- - -
-		-at 22.5', wet, soft		SPT-9	22.5 - 24.0	1.5	3-2-3		-
481.2 - -	25.0	FAT CLAY (CH), to orange with with light brown and gray, mostiff	ght	SPT-10	25.0 - 26.5	1.5	3-4-12		- - -
- - -		-at 30.2', coarse g 1" -at 31.3', coarse g 2"		SPT-11	30.0 - 31.6	1.5	8-9-11		- - - -



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Client Bo	Client Borehole Identification LRCP-21-06 Stantec Boring No. LRCP-21-06								
Client		IKEC			Boring Locat	tion 4	44082.85 N;	563226.2	249 E
Project N	Number	175539026			Surface Elev	ation 5	06.2 ft E	levation [	Datum_NAVD88_
Litholo	ogy		Overburden	Sample #	Depth	Rec. Ft.	Blows/ Press.(psi)	NMC %	
Elevation	Depth	Description	Rock Core	RQD	Run	Rec. Ft.	Rec. %	Depth	Remarks
-		FAT CLAY (CH), ke orange with with lip brown and gray, me stiff (Continued) -at 36.0', coarse ge 6"	ght noist,	SPT-12	35.0 - 36.5	1.0	12-10-15		- - -
-									-
466.2	40.0								_
466.0	<u>√40.2</u>	∖SHALE, gray, dry					ļ.		
_		Auger Refusal / Bottom of Hole							-
-									-
-									_
-		Borehole was bac ground surface us	kfilled with ing a tremi	a mixture e pipe.	of cement-be	ntonite (	grout from th	e bottom	of hole to the -
<u> </u>									_
-									-
									=
-									=
-									-
L									
_									_
-									=
<u> </u>									-
_									- -

#### **APPENDIX F**

Slope Stability Analysis



## Landfill Runoff Collection Pond Dam Stability

Closure Plan
Landfill Runoff Collection Pond
Clifty Creek Station
Madison, Jefferson County, Indiana

April 16, 2021

Prepared for:

Indiana-Kentucky Electric Corporation (IKEC)

Prepared by:

Stantec Consulting Services Inc.

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#### **LIST OF ATTACHMENTS**

ATTACHMENT A SLOPE STABILITY ANALYSES OUTPUTS

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INTRODUCTION

#### 1.0 INTRODUCTION

The Clifty Creek Generating Station's Landfill Runoff Collection Pond (LRCP), owned and operated by the Indiana Kentucky Electric Corporation (IKEC), is located in Jefferson County, Indiana. The facility is bordered in the south by the Ohio River, on the east a bedrock outcrop known as the Devil's Backbone and on the west by high ground. The LRCP served as a settling facility for sluiced fly ash produced by the generating plant. The LRCP will no longer receive fly ash and closure documentation is being prepared.

Stantec Consulting Services, Inc. (Stantec) was contracted by the IKEC to prepare construction design documents to support the closure of the LRCP. The purpose of this report is to present the results of slope stability analyses of the LRCP dam with the proposed closure configuration of closure. Closure will be executed by excavating, filling, and grading existing fly ash in the pond to create positive drainage and will include the installation of a cover system. The existing dam will be degraded as stormwater run-on will be captured by a diversion ditch and routed downstream. The cover system, from top to bottom, will consist of 6 inches of topsoil, 2.5 feet of cover soil, a geocomposite drainage layer, and a 40-mil thickness linear low-density polyethylene (LLDPE) flexible membrane liner (FML) overlying the in-place fly ash coal combustion residual (CCR) material.

This report documents the information reviewed from previous field explorations and laboratory testing and geotechnical engineering analyses performed by Stantec to support the closure requirements. The scope of work includes the following:

- Existing documentation review
- Slope stability analyses of the modified LRCP dam under long-term, 1/2 Probable Maximum Flood (PMF), and seismic loading conditions

#### 2.0 GLOBAL SLOPE STABILITY ANALYSES

The global slope stability was evaluated using conventional, limit equilibrium, method of slices analysis as implemented in the SLOPE/W module of GeoStudio 2021. Spencer's method was selected for the analyses as this method includes all interslice forces and satisfies both moment and force equilibrium. A slope stability analysis includes a search for the most critical slip surface, corresponding to the lowest factor of safety. Several options are available in SLOPE/W to facilitate the search for the critical failure surface. In the analyses presented in this report, potential circular failure surfaces were generated using the "entry and exit" method. Considering shallow, surficial failures pose little risk to the overall stability of the slopes, and are usually considered as a potential maintenance issue, a minimum slip surface depth of 3 feet was specified in the analyses to force the evaluation on the deeper potential failure surfaces.

When a soil at the entry of a potential failure surface is assigned with a cohesion value (c>0) in a slope stability analysis, tensile stresses are often computed between the slices in this area. In the field, tensile



#### LANDFILL RUNOFF COLLECTION POND DAM STABILITY

GLOBAL SLOPE STABILITY ANALYSES

stresses result in the opening of a tension crack, reducing the lateral stresses to zero. Because tension results in a stabilizing force at the head of the sliding mass, it is unconservative to have tensile stresses between the slices in a slope stability analysis. A tension crack line was used in the analysis to eliminate the tensile inter-slice forces. Multiple iterations were performed to define the appropriate tension crack line.

# 2.1 LOAD CASES, GROUNDWATER CONDITIONS, AND DESIGN CRITERIA

Global slope stability analyses were performed on the final grading of the LRCP closure. The evaluated load cases, along with the target factors of safety, are summarized in Table 2.1. Target factors of safety were selected per criteria presented in the Indiana Administrative Code (Table 1 of 329 IAC 10-15-8). If the slope were to fail, it would cause major environmental impact due to material being released into the Ohio River. There is small uncertainty of soil strengths with the laboratory testing performed (discussed in Stantec 2016). Therefore, the minimum factors of safety used for these analyses were 1.5 for static conditions and 1.3 for seismic, post-earthquake conditions.

Target Factor of Slope **Load Case Analysis** Safety Drained, effective 1.5 Long-term stress Drained, effective 1/2 PMF 1.5 stress Seismic (Pseudo-Undrained, total stress 1.0 static) LRCP Dam Residual strengths for liquefied CCR material Seismic (Post-1.3 Earthquake) Undrained, total stress reduced 20% for nonliquefied soils

Table 2.1: Evaluated Load Cases

#### 2.1.1 Long-Term

This analysis was performed to evaluate the stability of the final grading under the long-term, drained condition after excess pore pressures have dissipated. Drained, effective stress strength parameters were used in the analysis. A long-term piezometric line was applied to the slope stability models assumed to be five feet below the lowest point of the proposed closure liner.

#### 2.1.2 1/2 Probable Maximum Flood

This analysis was performed to evaluate the stability of the LRCP dam under a design storm event equivalent to 1/2 of the site's PMF event. Drained, effective strength parameters were used in the



#### LANDFILL RUNOFF COLLECTION POND DAM STABILITY

GLOBAL SLOPE STABILITY ANALYSES

analysis. The high water piezometric line was assigned to elevation 487 feet based on an anticipated backwater in the LRCP dam during this flood event.

#### 2.1.3 Seismic (Pseudo-static)

This analysis was performed to evaluate the stability of the embankment dike and final grading under seismic loading from a design earthquake event. Undrained, pseudo-static strength parameters were assigned to low permeability materials in the analysis. Groundwater was assumed to be at the long-term water level.

The US Geological Survey (USGS) Unified Hazard Tool was used to determine the site's peak ground acceleration (PGA) corresponding to a seismic event with a return period of 2,475 years and a peak rock acceleration at the site of 0.0882 g was obtained.

Haynes-Griffin and Franklin (1984) recommended that half of the peak rock acceleration be used as the seismic coefficient in the pseudo-static slope stability analysis. Based on that, a seismic coefficient of 0.0441 (=0.5 x 0.0882) was selected. The selected seismic coefficient was rounded and a horizontal seismic coefficient of 0.045 was used in SLOPE/W. The program calculates seismic force (=column weight x seismic coefficient) and applies to each column in the sliding mass.

Note that the pseudo-static stability analysis should only be considered as an index of the seismic resistance available in a structure not subject to build-up of pore pressure from shaking. A pseudo-static factor of safety greater than 1.0 is very strong evidence that there would be little or no damage to the dam from an earthquake (FEMA 2005).

Groundwater was assumed to be at the long-term water level which was assumed to be 5 feet below the proposed closure liner.

#### 2.1.4 Post-Earthquake

This analysis was performed to evaluate the stability of the LRCP dam assuming liquefaction of the LRCP fly ash to evaluate the potential for a release of CCR offsite. The assumption for performing this analysis is that the saturated fly ash (CCR) deposits will undergo pore pressure build-up and liquefy during the design seismic event. A post-earthquake, residual strength (in the form of Residual Shear Strength ratio as discussed in Section 2.3) was applied to the saturated fly ash (CCR) layers. Undrained, total stress strength parameters were reduced 20% and were applied to non-liquifiable soils to account for potential strength loss due to earthquake shaking.

Groundwater was assumed to be at the long-term water level which was assumed to be 5 feet below the proposed closure liner.



GLOBAL SLOPE STABILITY ANALYSES

#### 2.2 SECTION GEOMETRY

The section of maximum dam height was selected as the critical section for analyses. Locations of the selected cross sections are shown in the drawings. The subsurface profiles modeled in the analyses for each cross section were selected based on historical data from previous explorations. Soil borings selected for definition of the subsurface lithology for each cross section are shown in Table 2.2 below.

Table 2.2: Soil Borings used for Stratigraphy Definition

Cross Section	Soil Boring
LRCP Dam	Stantec B-7, Stantec B-8, Stantec B-9, and Stantec B-10

#### 2.3 MATERIAL PROPERTIES

Material strength parameters used in the stability analyses of this study are summarized in Table 2.3 below. These parameters are the same parameters that were used and selected in the Stantec 2016 Geotechnical Report, with the exception of the Cover Soil which is based on the field exploration results, reviewed historical geotechnical information, and engineering judgement.

**Table 2.3: Material Parameters for Slope Stability Analysis** 

	Unit	Drained Shear	Undrained Shear Strengths	Residual Strength	
Material	Weight	Effective Friction Angle φ'	Effective Cohesion c'	Su	Sr
	(pcf)	(deg.)	(psf)	(psf)	(psf)
Embankment	129	27.5	198	1400¹	-
Lean Clay with Sand	127	28	206	1200¹	-
Clayey Gravel with Sand	130	35	0	-	-
Fly Ash (CCR)	115	25	0	-	$S_r / \sigma_{vc}' = 0.04^{-6}$
Sandy Silt	125	30	0	-	-
Silty Sand	94	30	0	-	
Cover Soil	125	27	300	1000¹	-

1. For Seismic (Post Earthquake Analyses) use 80% of Undrained Shear Strength.



#### LANDFILL RUNOFF COLLECTION POND DAM STABILITY

GLOBAL SLOPE STABILITY ANALYSES

Soil strength parameters, with the exception of the Cover Soil, were derived in the Stantec February 2016 CCR Rule Stability Analyses report. The 2016 parameters were developed using CU triaxial test data for the Embankment Fill and Lean Clay with Sand soils. Strength parameters for the proposed Cover Soil were taken to be identical to the parameters of the Lean Clay with Sand soil and its unit weight was selected based on preliminary standard Proctor testing data of a potential borrow source soil. Undrained shear strength parameters for coarse-grained (or cohesionless) materials were taken to be identical to the drained shear strength parameters.

The residual strength of liquefied fly ash (CCR) layers for post-earthquake evaluations was estimated by applying a conservative Residual Shear Strength ratio,  $\tau/\sigma$  (Shear Strength/Effective Overburden Stress Ratio) of 0.04 using correlations presented by Idriss and Boulanger between corrected SPT blow counts and the Residual Shear Strength Ratio. SPT blowcounts were obtained from existing site data boring logs and a representative SPT N value of 3 was selected to obtain the selected Residual Shear Strength ratio of 0.04 for the fly ash materials.

#### 2.4 ANALYSIS RESULTS

The computed factors of safety of slope stability for the LRCP closure grading under the evaluated loading conditions are summarized in Table 2.4, along with the target factors of safety according to the established design criteria for the project. The computed output plots, depicting the predicted critical failure surfaces, are presented in Attachment A.

**Table 2.4: Computed Slope Stability Factors of Safety** 

Section	Location	Loading Condition	Computed Slope Stability Factor of Safety	Target Factor of Safety
	Dam – Downstream Slope	Long Term	2.15	1.5
	Dam – Upstream Slope Long Term		4.12	1.5
	Dam – Downstream Slope ½ PMF Pool		1.93	1.5
LRCP Dam	Dam – Upstream Slope	½ PMF Pool	4.49	1.5
LIXOF Daili	Dam – Downstream Slope	Seismic (Pseudo-Static)	1.50	1.0
	Dam – Upstream Slope	Seismic (Pseudo-Static)	3.05	1.0
	Dam – Downstream Slope	Seismic (Post-earthquake)	1.66	1.3
	Dam – Upstream Slope	Seismic (Post-earthquake)	4.15	1.3



CONCLUSION

#### 3.0 CONCLUSION

The calculated factors of safety for the analyzed cases meet the minimum factors of safety required by 329 IAC 10-15-8. This analysis is based on the information discussed in this report and the interpretation of the subsurface conditions encountered at the site. No warranties can be made regarding the continuity of conditions. If future design changes are made, Stantec should be notified so that such changes can be reviewed, and the analysis amended as necessary.

#### 4.0 LIMITATIONS

This report was prepared by Stantec Consulting Services, Inc. (Stantec) for IKEC. Stantec's professional services have been performed using a degree of care and skill ordinarily exercised, under similar circumstances, by reputable geotechnical consultants practicing in this or similar localities. No other warranty, express or implied, is made as to the professional advice included in this report. This geotechnical report has been prepared for IKEC and is to be used solely for the design of the proposed Closure for the Landfill Runoff Collection Pond at the Clifty Creek Station in Jefferson County, Indiana and may not contain sufficient information for use by other parties.

The recommendations provided in this geotechnical report are based upon our understanding of the described project information and our interpretation of available published information and previous field and laboratory investigations. We have made our recommendations based upon experience with similar subsurface conditions. The recommendations apply to the specific project discussed in this report; therefore, any change in the configuration of the proposed design or any change to the site grades should be provided to us so that we can review our conclusions and recommendations and make any necessary modifications.

The recommendations provided in this report are based upon the assumption that the necessary geotechnical observations and testing during construction will be performed by our firm during the entire duration of the construction. The field observation services are considered a continuation of the geotechnical investigation and are essential to verify that the actual soil conditions are as expected. This also provides for the procedure whereby IKEC may be advised of unexpected or changed conditions that would require modifications of our original recommendations.



#### **5.0 REFERENCES**

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- Stantec Consulting Services, Inc. (2016). Report of CCR Rule Stability Analyses, AEP Clifty Creek Power Plant Boiler Slag Pond Dam and Landfill Runoff Collection Pond, American Electric Power, Columbus, Ohio.
- U.S. Army Corps of Engineers (2003). Slope Stability. EM 1110-2-1902, October 31.

### ATTACHMENT A SLOPE STABILITY ANALYSES OUTPUTS

Factor of Safety: 2.15

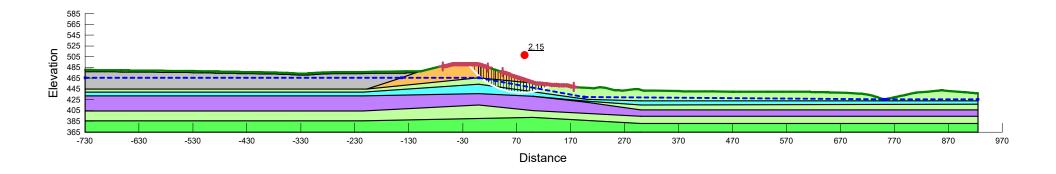
Long Term, Downstream Slope

**Method: Spencer** 

Slip Surface: Entry and Exit

Note: The results of the analysis shown here are based on available subsurface information, laboratory test results and approximate soil properties. The drawing depicts approximate subsurface conditions based on historical drawings or specific borings at the time of drilling.

Color	Name	Model	Unit Weight (pcf)	Effective Cohesion (psf)	Effective Friction Angle (°)
	Clayey Gravel with Sand (Drained)	Mohr-Coulomb	130	0	35
	Cover Soil (Drained)	Mohr-Coulomb	125	300	27
	Embankment (Drained)	Mohr-Coulomb	129	198	27.5
	Fly Ash (Drained)	Mohr-Coulomb	115	0	25
	Lean Clay with Sand (Drained)	Mohr-Coulomb	127	206	28
	Sandy Silt (Drained)	Mohr-Coulomb	125	0	30
	Silty Sand (Seismic Undrained)	Mohr-Coulomb	94	0	30



Factor of Safety: 4.12

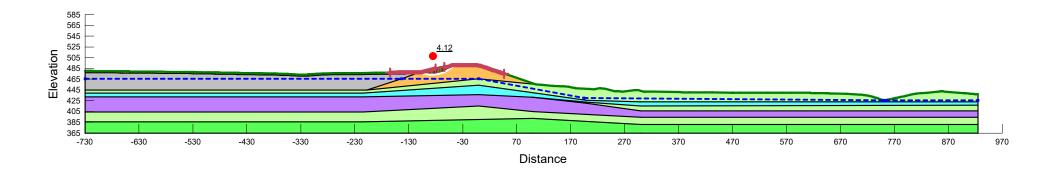
Long Term, Upstream Slope

**Method: Spencer** 

Slip Surface: Entry and Exit

Note: The results of the analysis shown here are based on available subsurface information, laboratory test results and approximate soil properties. The drawing depicts approximate subsurface conditions based on historical drawings or specific borings at the time of drilling.

Color	Name	Model	Unit Weight (pcf)	Effective Cohesion (psf)	Effective Friction Angle (°)
	Clayey Gravel with Sand (Drained)	Mohr-Coulomb	130	0	35
	Cover Soil (Drained)	Mohr-Coulomb	125	300	27
	Embankment (Drained)	Mohr-Coulomb	129	198	27.5
	Fly Ash (Drained)	Mohr-Coulomb	115	0	25
	Lean Clay with Sand (Drained)	Mohr-Coulomb	127	206	28
	Sandy Silt (Drained)	Mohr-Coulomb	125	0	30
	Silty Sand (Seismic Undrained)	Mohr-Coulomb	94	0	30



Factor of Safety: 1.93

50% PMF Pool, Downstream Slope

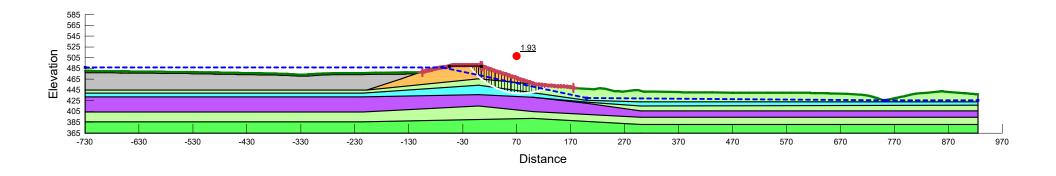
50% PMF Pool Elevation: 487 feet

**Method: Spencer** 

Slip Surface: Entry and Exit

Note: The results of the analysis shown here are based on available subsurface information, laboratory test results and approximate soil properties. The drawing depicts approximate subsurface conditions based on historical drawings or specific borings at the time of drilling.

Color	Name	Model	Unit Weight (pcf)	Effective Cohesion (psf)	Effective Friction Angle (°)
	Clayey Gravel with Sand (Drained)	Mohr-Coulomb	130	0	35
	Cover Soil (Drained)	Mohr-Coulomb	125	300	27
	Embankment (Drained)	Mohr-Coulomb	129	198	27.5
	Fly Ash (Drained)	Mohr-Coulomb	115	0	25
	Lean Clay with Sand (Drained)	Mohr-Coulomb	127	206	28
	Sandy Silt (Drained)	Mohr-Coulomb	125	0	30
	Silty Sand (Drained)	Mohr-Coulomb	94	0	30



Factor of Safety: 4.49

50% PMF Pool, Upstream Slope

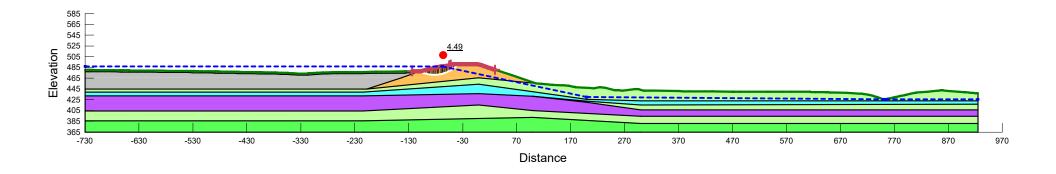
50% PMF Pool Elevation: 487 feet

**Method: Spencer** 

Slip Surface: Entry and Exit

Note: The results of the analysis shown here are based on available subsurface information, laboratory test results and approximate soil properties. The drawing depicts approximate subsurface conditions based on historical drawings or specific borings at the time of drilling.

Color	Name	Model	Unit Weight (pcf)	Effective Cohesion (psf)	Effective Friction Angle (°)
Clayey Gravel with Sand (Drained)		Mohr-Coulomb	130	0	35
	Cover Soil (Drained)	Mohr-Coulomb	125	300	27
	Embankment (Drained)	Mohr-Coulomb	129	198	27.5
	Fly Ash (Drained)	Mohr-Coulomb	115	0	25
	Lean Clay with Sand (Drained)	Mohr-Coulomb	127	206	28
	Sandy Silt (Drained)	Mohr-Coulomb	125	0	30
	Silty Sand (Drained)	Mohr-Coulomb	94	0	30



Madison, Indiana

Seismic (Pseudo-static), Downstream Slope

**Method: Spencer** 

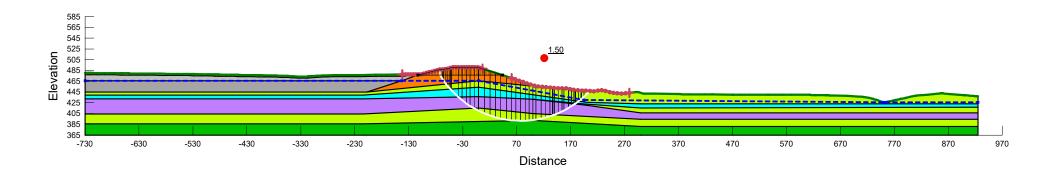
Slip Surface: Entry and Exit Horz Seismic Coef.: 0.045

Note: The results of the analysis shown here are based on available subsurface information, laboratory test results and approximate soil properties. The drawing depicts approximate subsurface conditions based on historical drawings or specific borings at the time of drilling.

No warranties can be made regarding the continuity of subsurface conditions.

Color	Name	Model	Unit Weight (pcf)	Cohesion (psf)	Effective Cohesion (psf)	Effective Friction Angle (°)
	Clayey Gravel with Sand (Seismic Undrained)	Mohr-Coulomb	130		0	35
	Cover Soil (Seismic Undrained)	Undrained (Phi=0)	125	1,000		
	Embankment (Seismic Undrained)	Undrained (Phi=0)	129	1,400		
	Fly Ash (Drained)	Mohr-Coulomb	115		0	25
	Fly Ash (Seismic Undrained)	Mohr-Coulomb	115		0	25
	Lean Clay with Sand (Seismic Undrained)	Undrained (Phi=0)	127	1,200		
	Sandy Silt (Seismic Undrained)	Mohr-Coulomb	125		0	30
	Silty Sand (Seismic Undrained)	Mohr-Coulomb	94		0	30

Factor of Safety: 1.50



Factor of Safety: 3.05

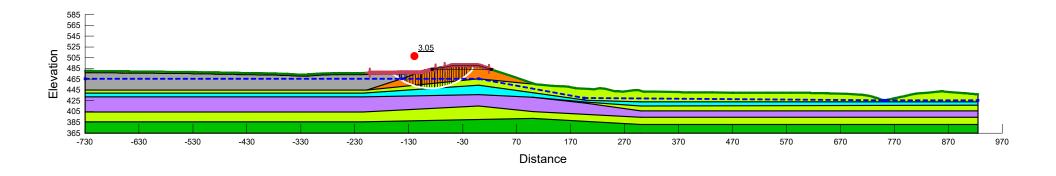
Seismic (Pseudo-static), Upstream Slope

**Method: Spencer** 

Slip Surface: Entry and Exit Horz Seismic Coef.: 0.045

Note: The results of the analysis shown here are based on available subsurface information, laboratory test results and approximate soil properties. The drawing depicts approximate subsurface conditions based on historical drawings or specific borings at the time of drilling.

Color	Name	Model	Unit Weight (pcf)	Cohesion (psf)	Effective Cohesion (psf)	Effective Friction Angle (°)
	Clayey Gravel with Sand (Seismic Undrained)	Mohr-Coulomb	130		0	35
	Cover Soil (Seismic Undrained)	Undrained (Phi=0)	125	1,000		
	Embankment (Seismic Undrained)	Undrained (Phi=0)	129	1,400		
	Fly Ash (Seismic Undrained)	Mohr-Coulomb	115		0	25
	Lean Clay with Sand (Seismic Undrained)	Undrained (Phi=0)	127	1,200		
	Sandy Silt (Seismic Undrained)	Mohr-Coulomb	125		0	30
	Silty Sand (Seismic Undrained)	Mohr-Coulomb	94		0	30



Madison, Indiana

Seismic (Post Earthquake), Downstream Slope

**Method: Spencer** 

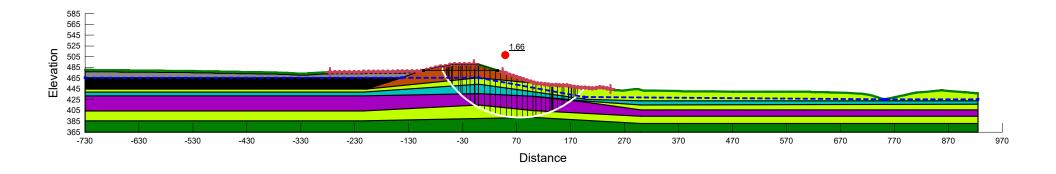
Slip Surface: Entry and Exit

Note: The results of the analysis shown here are based on available subsurface information, laboratory test results and approximate soil properties. The drawing depicts approximate subsurface conditions based on historical drawings or specific borings at the time of drilling.

No warranties can be made regarding the continuity of subsurface conditions.

Color	Name	Model	Unit Weight (pcf)	Tau/Sigma Ratio	Cohesion (psf)	Effective Cohesion (psf)	Effective Friction Angle (°)
	Clayey Gravel with Sand (Post Earthquake)	Mohr-Coulomb	130			0	35
	Cover Soil (Post Earthquake)	Undrained (Phi=0)	125		800		
	Embankment (Post Earthquake)	Mohr-Coulomb	129			1,120	27.5
	Fly Ash (Post Earthquake)	Mohr-Coulomb	115			0	25
	Fly Ash (Residual Strength)	SHANSEP	115	0.04			
	Lean Clay with Sand (Post Earthquake)	Undrained (Phi=0)	127		960		
	Sandy Silt (Post Earthquake)	Mohr-Coulomb	125			0	30
	Silty Sand (Post Earthquake)	Mohr-Coulomb	94			0	30

Factor of Safety: 1.66



Madison, Indiana

Seismic (Post Earthquake), Upstream Slope

**Method: Spencer** 

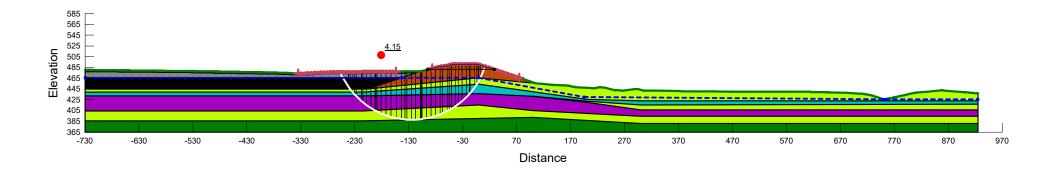
Slip Surface: Entry and Exit

Note: The results of the analysis shown here are based on available subsurface information, laboratory test results and approximate soil properties. The drawing depicts approximate subsurface conditions based on historical drawings or specific borings at the time of drilling.

No warranties can be made regarding the continuity of subsurface conditions.

Color	Name	Model	Unit Weight (pcf)	Tau/Sigma Ratio	Cohesion (psf)	Effective Cohesion (psf)	Effective Friction Angle (°)
	Clayey Gravel with Sand (Post Earthquake)	Mohr-Coulomb	130			0	35
	Cover Soil (Post Earthquake)	Undrained (Phi=0)	125		800		
	Embankment (Post Earthquake)	Mohr-Coulomb	129			1,120	27.5
	Fly Ash (Post Earthquake)	Mohr-Coulomb	115			0	25
	Fly Ash (Residual Strength)	SHANSEP	115	0.04			
	Lean Clay with Sand (Post Earthquake)	Undrained (Phi=0)	127		960		
	Sandy Silt (Post Earthquake)	Mohr-Coulomb	125			0	30
	Silty Sand (Post Earthquake)	Mohr-Coulomb	94			0	30

Factor of Safety: 4.15



#### **APPENDIX G**

LRCP Permit Drawings

# PERMIT DRAWINGS

# POND CLOSURE LANDFILL RUNOFF COLLECTION POND CLIFTY CREEK STATION JEFFERSON COUNTY, MADISON TOWNSHIP, INDIANA

PREPARED FOR



PIKETON, OHIO

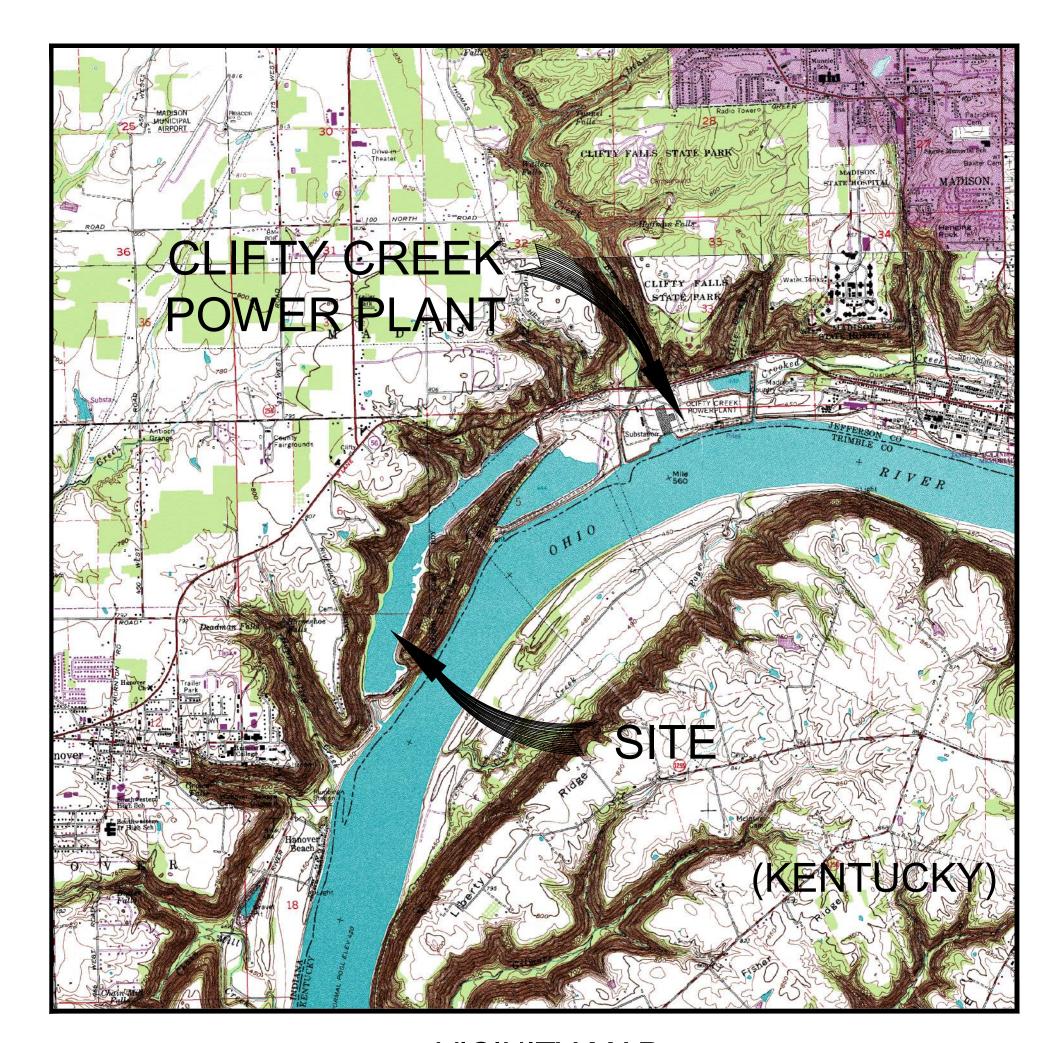
#### **INDEX OF SHEETS**

SHEET NO.	DRAWING NO.	DESCRIPTION	REVISION
01	P-LRCP-001-CVR	COVER SHEET	A
02	P-LRCP-101-OVR	OVERVIEW / SEQUENCING PLAN	A
03	P-LRCP-102-EC1	EXISTING CONDITIONS	A
04	P-LRCP-104-FG1	FINAL GRADING PLAN - PHASE 1 STORMWATER RUN-ON DIVERSION	A
05	P-LRCP-106-FG3	FINAL GRADING PLAN - PHASE 2 LRCP CAP CLOSURE	A
06	P-LRCP-201-EP	EROSION PREVENTION AND SEDIMENT CONTROL	A
07	P-LRCP-202-SMP	STORMWATER MANAGEMENT PLAN	A
08	P-LRCP-220-PR1	PROFILE - DIVERSION DITCH BASELINE	A
09	P-LRCP-221-PR2	PROFILE - LRCP CLOSURE MAIN CHANNEL BASELINE	A
10	P-LRCP-301-XS	CROSS SECTIONS - MAIN CHANNEL BASELINE	A
11	P-LRCP-501-DTL	CIVIL DETAILS	A
12	P-LRCP-502-DTL	CIVIL DETAILS	A

PREPARED BY



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Cincinnati, Ohio
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Tel. 513.842.8200
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VICINITY MAP



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Client/Project Logo



Client/Project
OHIO VALLEY ELECTRIC CORPORATION
INDIANA-KENTUCKY ELECTRIC CORPORATION
POND CLOSURE - LANDFILL RUNOFF
COLLECTION POND, CLIFTY CREEK STATION
MADISON TOWNSHIP, JEFFERSON COUNTY, INDIANA

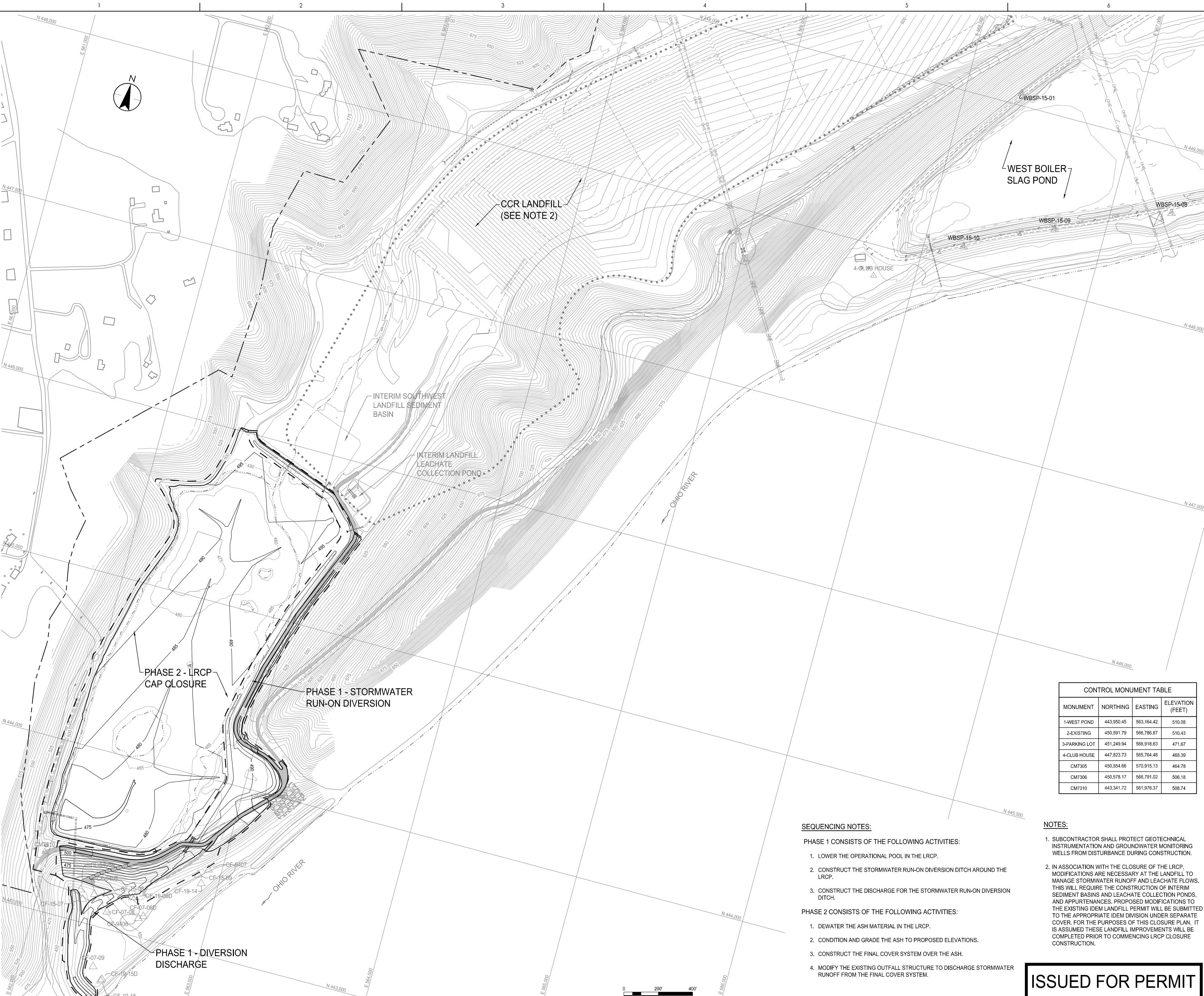
Title COVER SHEET

Revision Sheet

Project No. 175539026

Scale
AS SHOWN
Drawing No.

Drawing No.
P-LRCP-001-CVR





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#### LEGEN

**ELECTRIC TOWER ELECTRIC PULLBOX** TREE/SHRUB **ELECTRIC POLE** POWER POLE STORM CATCH BASIN OVERHEAD ELECTRIC — UGE——— UNDERGROUND ELECTRIC — − − PROPERTY LINE RAILROAD TRACKS  $\cdots$  —  $\cdots$  — EDGE OF WATER — 450 — EXISTING INDEX CONTOUR **EXISTING INTERMEDIATE CONTOUR** →→→→ DITCH / FLOW DIRECTION —— 450 ——— PROPOSED INDEX CONTOUR PROPOSED INTERMEDIATE CONTOUR • • • • • • • TYPE I LANDFILL WASTE BOUNDARY GROUNDWATER MONITORING WELL CONTROL MONUMENT

File Name: 02\_LRCP\_39



— PHASE BOUNDARY

Client/Project Logo



Client/Project
OHIO VALLEY ELECTRIC CORPORATION
INDIANA-KENTUCKY ELECTRIC CORPORATION

POND CLOSURE - LANDFILL RUNOFF COLLECTION POND, CLIFTY CREEK STATION

MADISON TOWNSHIP, JEFFERSON COUNTY, INDIANA

OVERVIEW / SEQUENCING PLAN

Project No. 175539026

Project No.

175539026

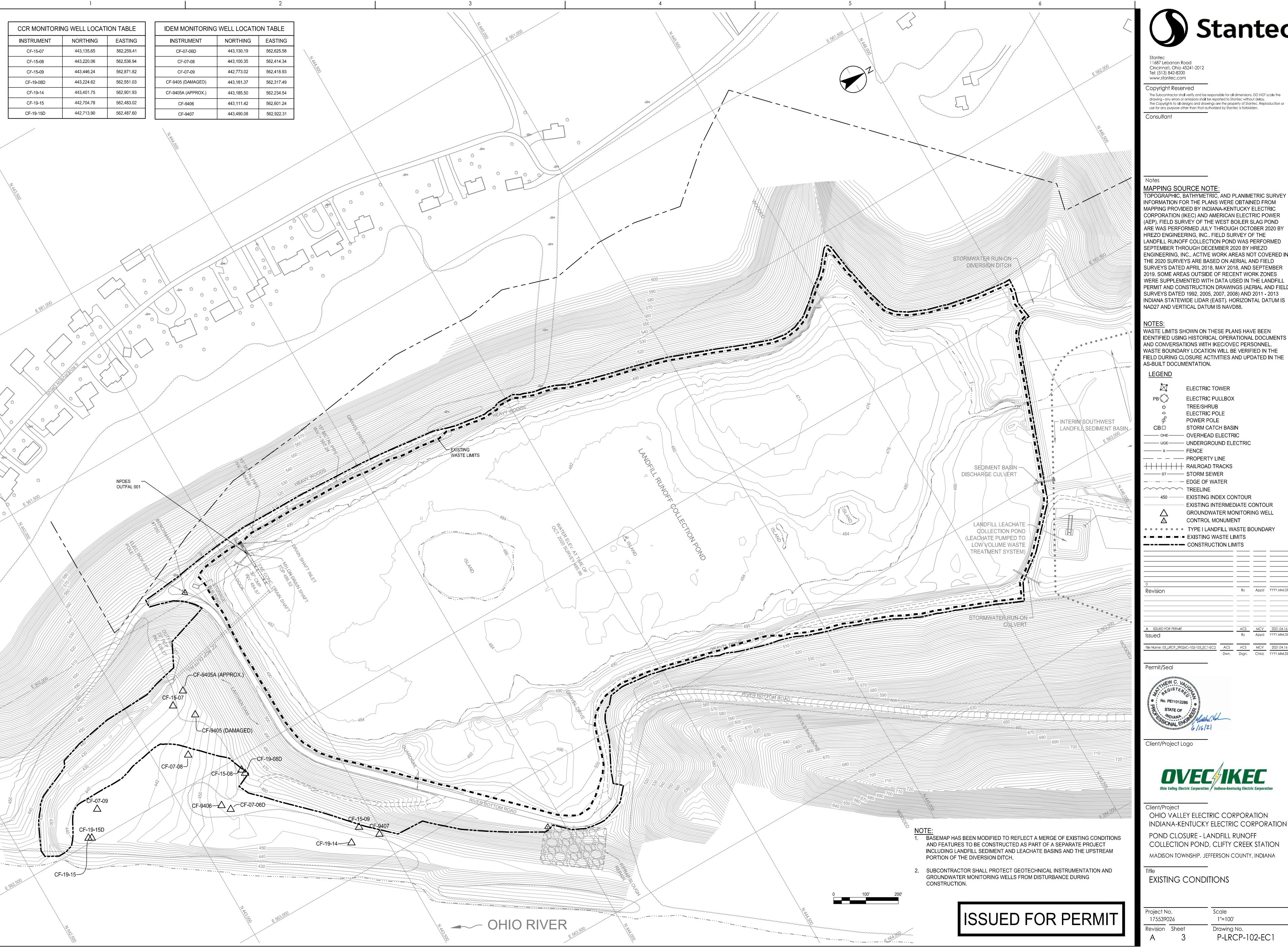
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WASTE LIMITS SHOWN ON THESE PLANS HAVE BEEN IDENTIFIED USING HISTORICAL OPERATIONAL DOCUMENTS AND CONVERSATIONS WITH IKEC/OVEC PERSONNEL. WASTE BOUNDARY LOCATION WILL BE VERIFIED IN THE FIELD DURING CLOSURE ACTIVITIES AND UPDATED IN THE

ELECTRIC PULLBOX

—— UGE——— UNDERGROUND ELECTRIC

450 — EXISTING INDEX CONTOUR **EXISTING INTERMEDIATE CONTOUR** GROUNDWATER MONITORING WELL

• • • • • • • TYPE I LANDFILL WASTE BOUNDARY

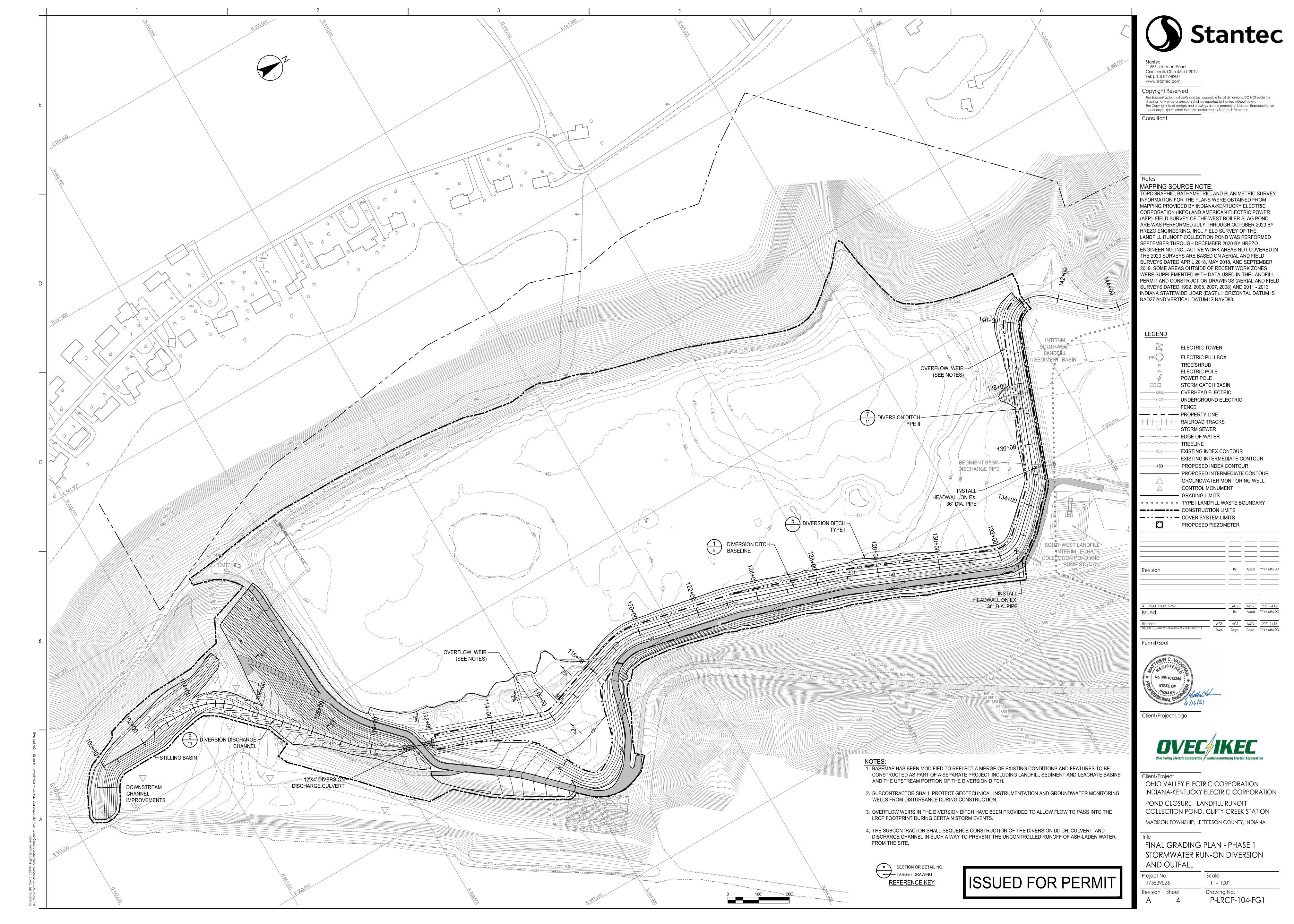


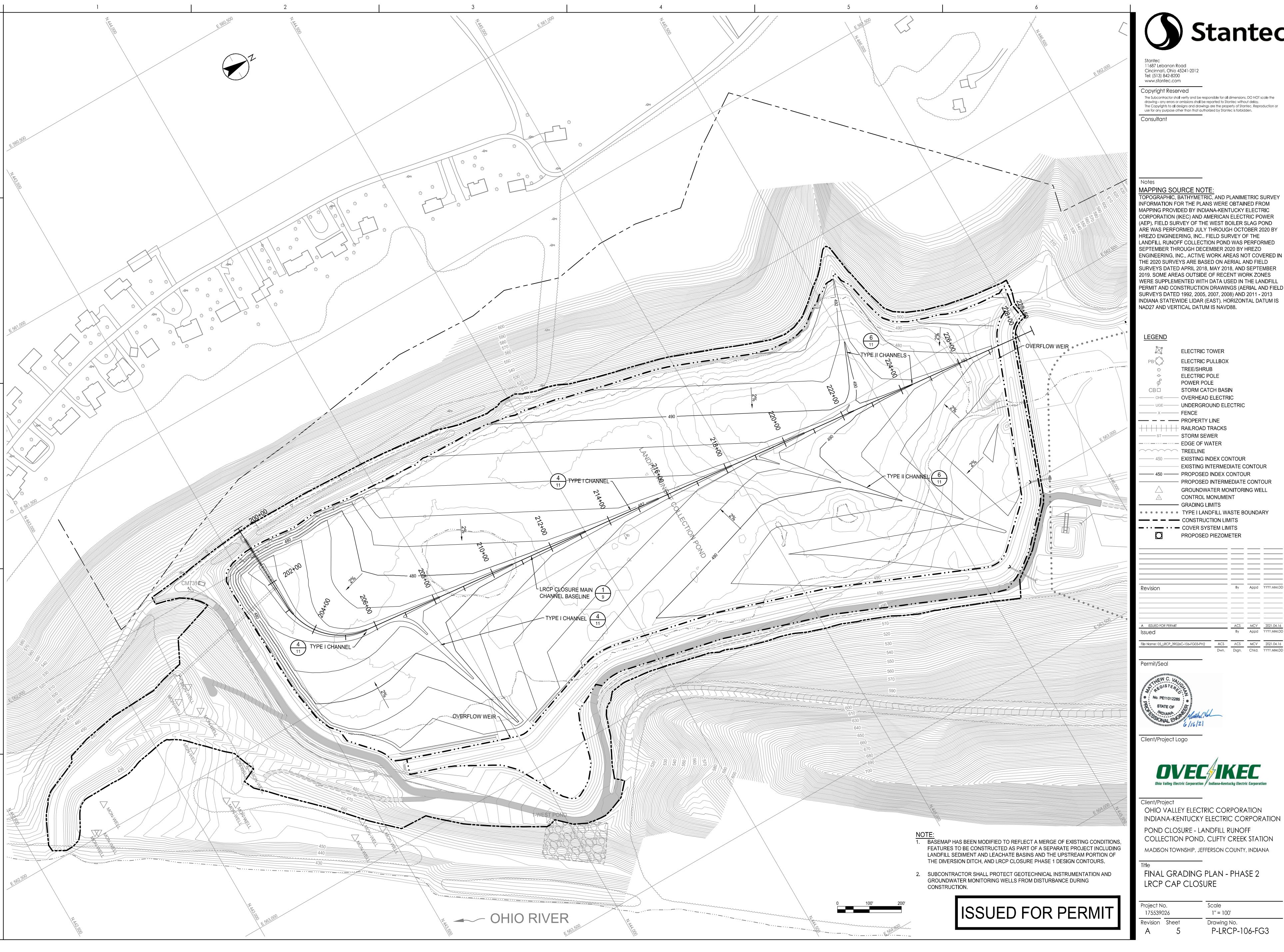
OHIO VALLEY ELECTRIC CORPORATION INDIANA-KENTUCKY ELECTRIC CORPORATION POND CLOSURE - LANDFILL RUNOFF

MADISON TOWNSHIP, JEFFERSON COUNTY, INDIANA

1''=100'

Drawing No.
P-LRCP-102-EC1





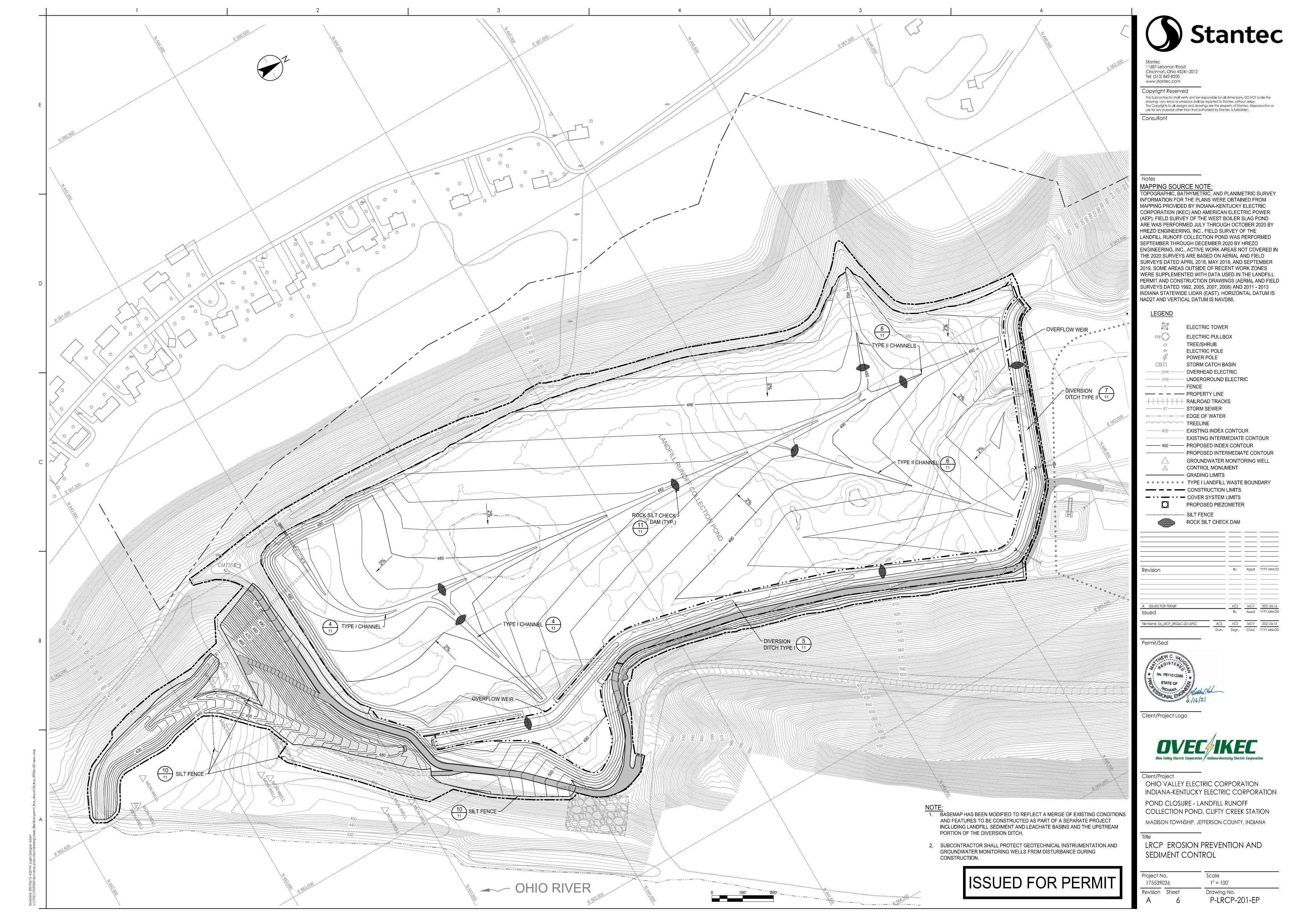
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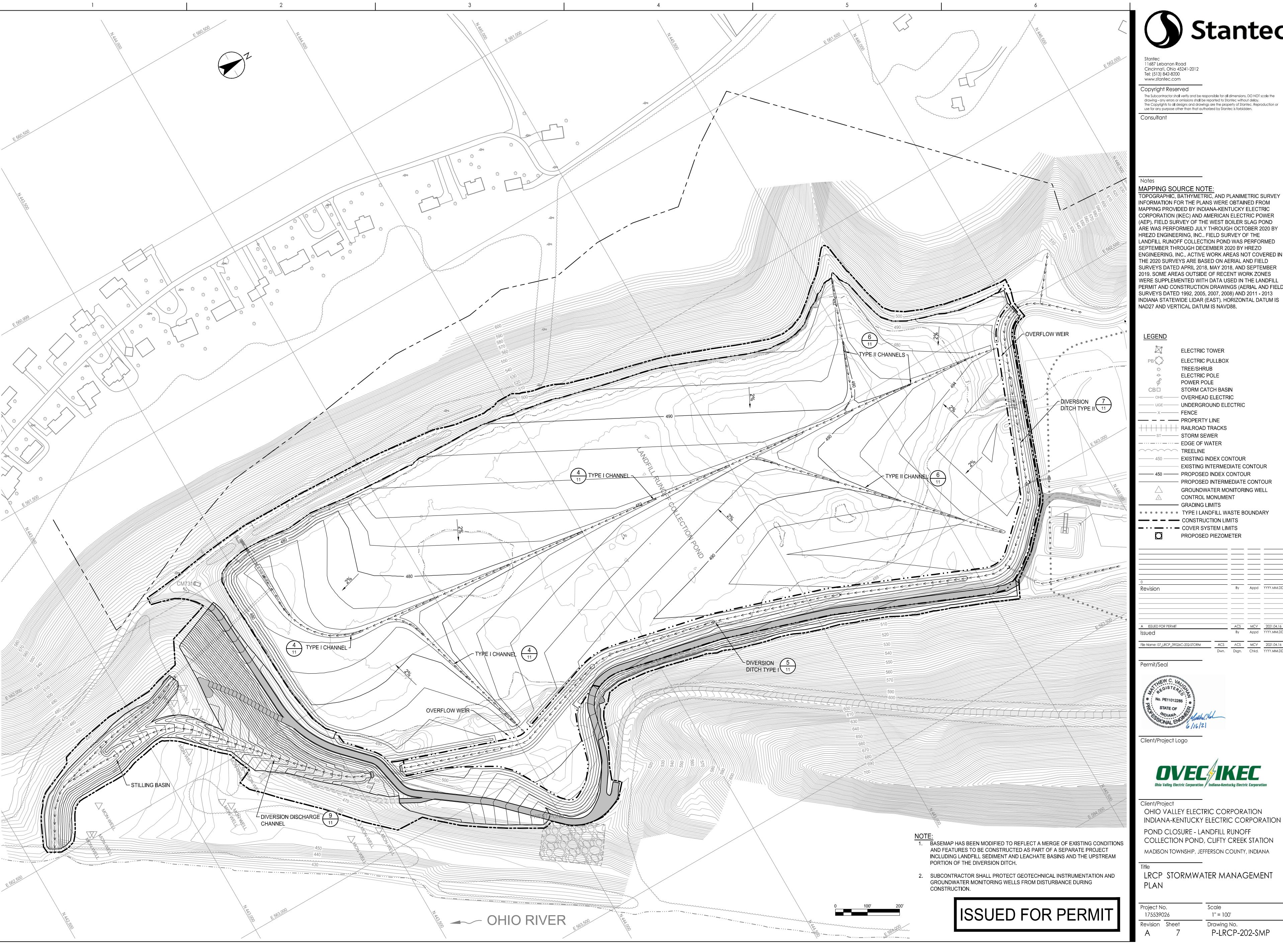
> **EXISTING INTERMEDIATE CONTOUR** GROUNDWATER MONITORING WELL



INDIANA-KENTUCKY ELECTRIC CORPORATION COLLECTION POND, CLIFTY CREEK STATION

FINAL GRADING PLAN - PHASE 2





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STORM CATCH BASIN

**EXISTING INTERMEDIATE CONTOUR** GROUNDWATER MONITORING WELL CONTROL MONUMENT

• • • • • • TYPE I LANDFILL WASTE BOUNDARY

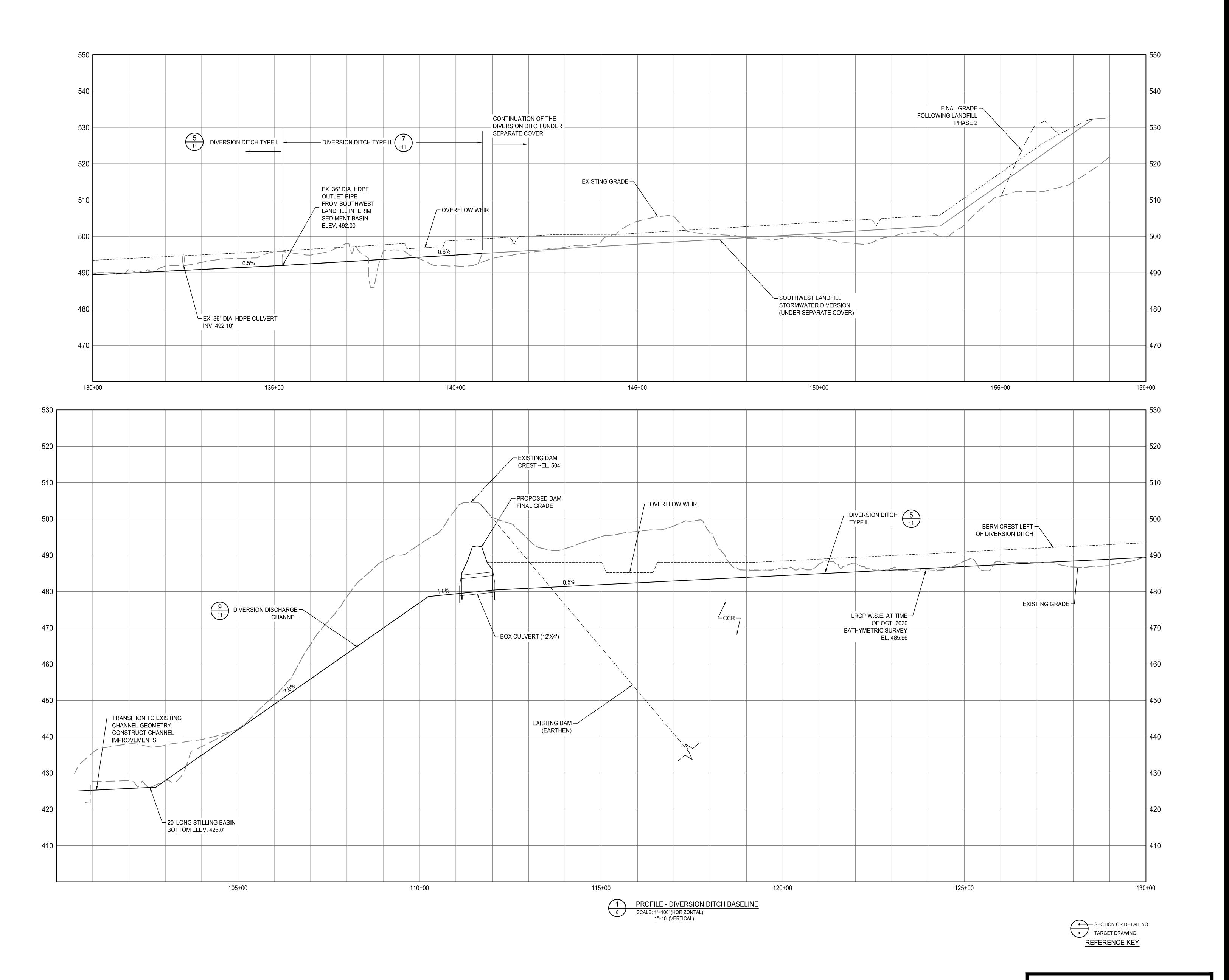


POND CLOSURE - LANDFILL RUNOFF COLLECTION POND, CLIFTY CREEK STATION MADISON TOWNSHIP, JEFFERSON COUNTY, INDIANA

LRCP STORMWATER MANAGEMENT

1" = 100'

Drawing No.
P-LRCP-202-SMP





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PROFILE - DIVERSION DITCH BASELINE

175539026

Revision Sheet

ISSUED FOR PERMIT

Scale as shown

Drawing No.
P-LRCP-220-PR1

PHASE 2 | PHASE 1 \_ DIVERSION BERM CREST (NOT IN SAME PROFILE) 4 TYPE I CHANNEL TYPE II CHANNEL 6 \_ INTERIM SOUTHWEST LANDFILL SEDIMENT OVERFLOW WEIR — LRCP W.S.E. AT TIME -BASIN (UNDER OF OCT. 2020 \_ \_ DIFFERENT COVER) BATHYMETRIC SURVEY PVI STA: 227+07.00 EL. 485.96 FINAL GRADE (CENTER -ELEV: 492.68 OF CHANNEL) 480 DIVERSION DITCH TYPE II PVI STA: 201+30.82 ELEV: 470.50 \ EXISTING GRADE -EX. 6'X3' OUTLET — SUBGRADE -STRUCTURE 205+00 210+00 215+00 220+00 225+00 200+00 230+00 231+00 PROFILE - LRCP CLOSURE MAIN CHANNEL BASELINE

SCALE: 1"=100' (HORIZONTAL) 1"=10' (VERTICAL) Stantec

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POND CLOSURE - LANDFILL RUNOFF
COLLECTION POND, CLIFTY CREEK STATION
MADISON TOWNSHIP, JEFFERSON COUNTY, INDIANA

PROFILES - LRCP CLOSURE MAIN
CHANNEL

Project No. Scale

Project No. Scale
175539026 AS SH
Revision Sheet Drawin

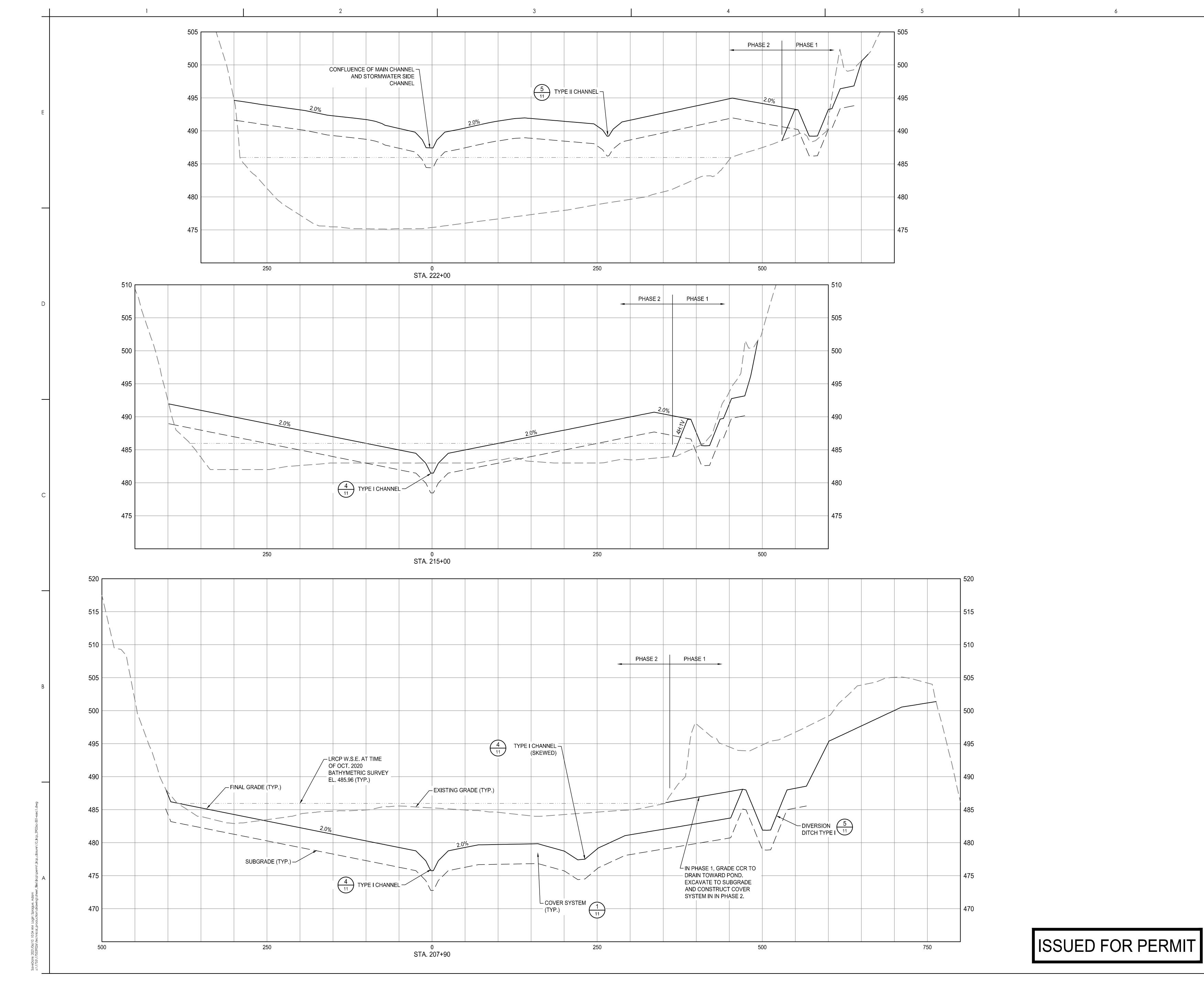
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Sheet Drawing No.
P-LRCP-221-PR2

SECTION OR DETAIL NO.

TARGET DRAWING

REFERENCE KEY

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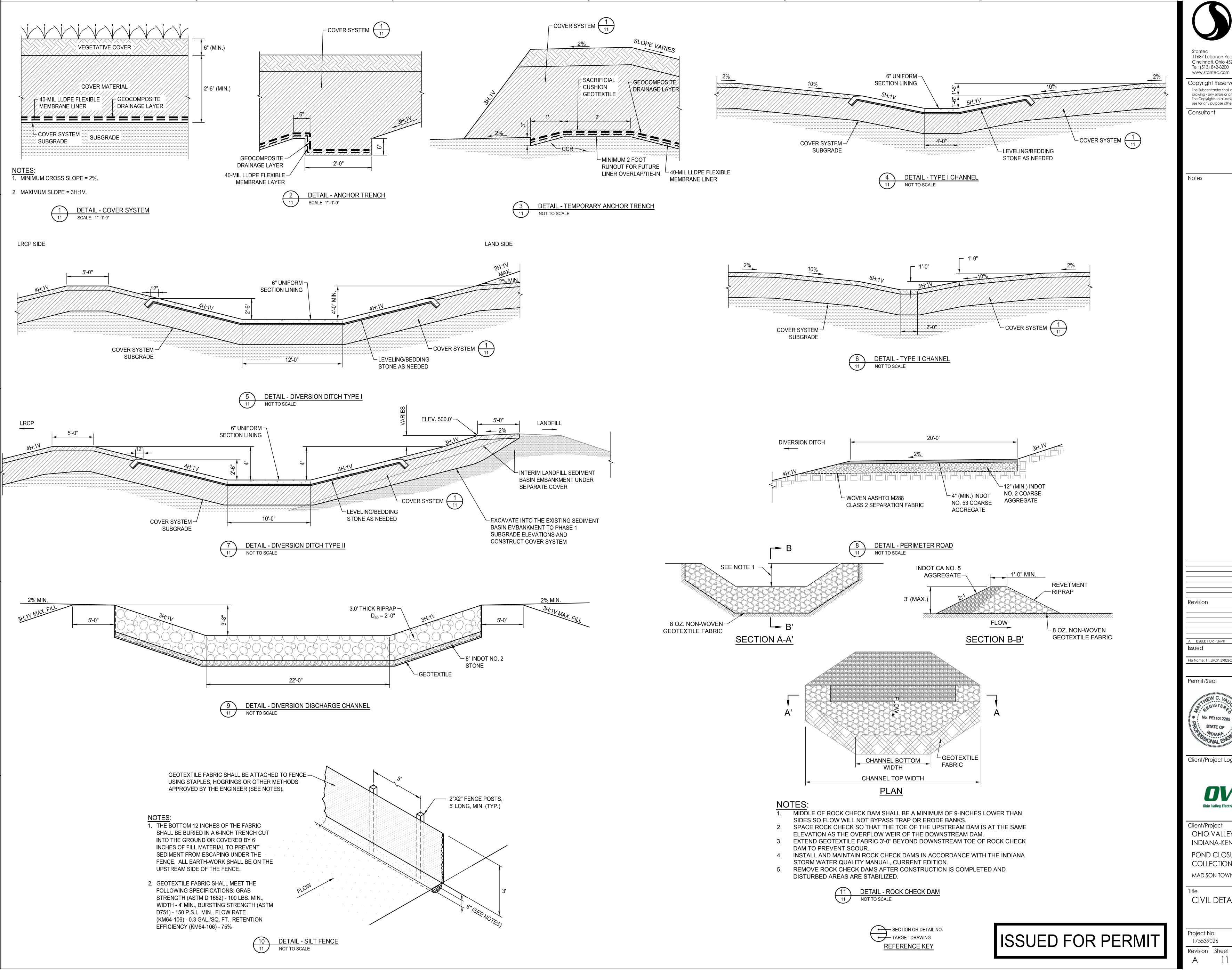
Client/Project OHIO VALLEY ELECTRIC CORPORATION INDIANA-KENTUCKY ELECTRIC CORPORATION

POND CLOSURE - LANDFILL RUNOFF COLLECTION POND, CLIFTY CREEK STATION MADISON TOWNSHIP, JEFFERSON COUNTY, INDIANA

CROSS SECTIONS - MAIN CHANNEL BASELINE

Project No. 175539026 Revision Sheet Scale as shown

Drawing No.
P-LRCP-301-XS





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File Name: 11\_LRCP\_39026C-501-DT1

Permit/Seal



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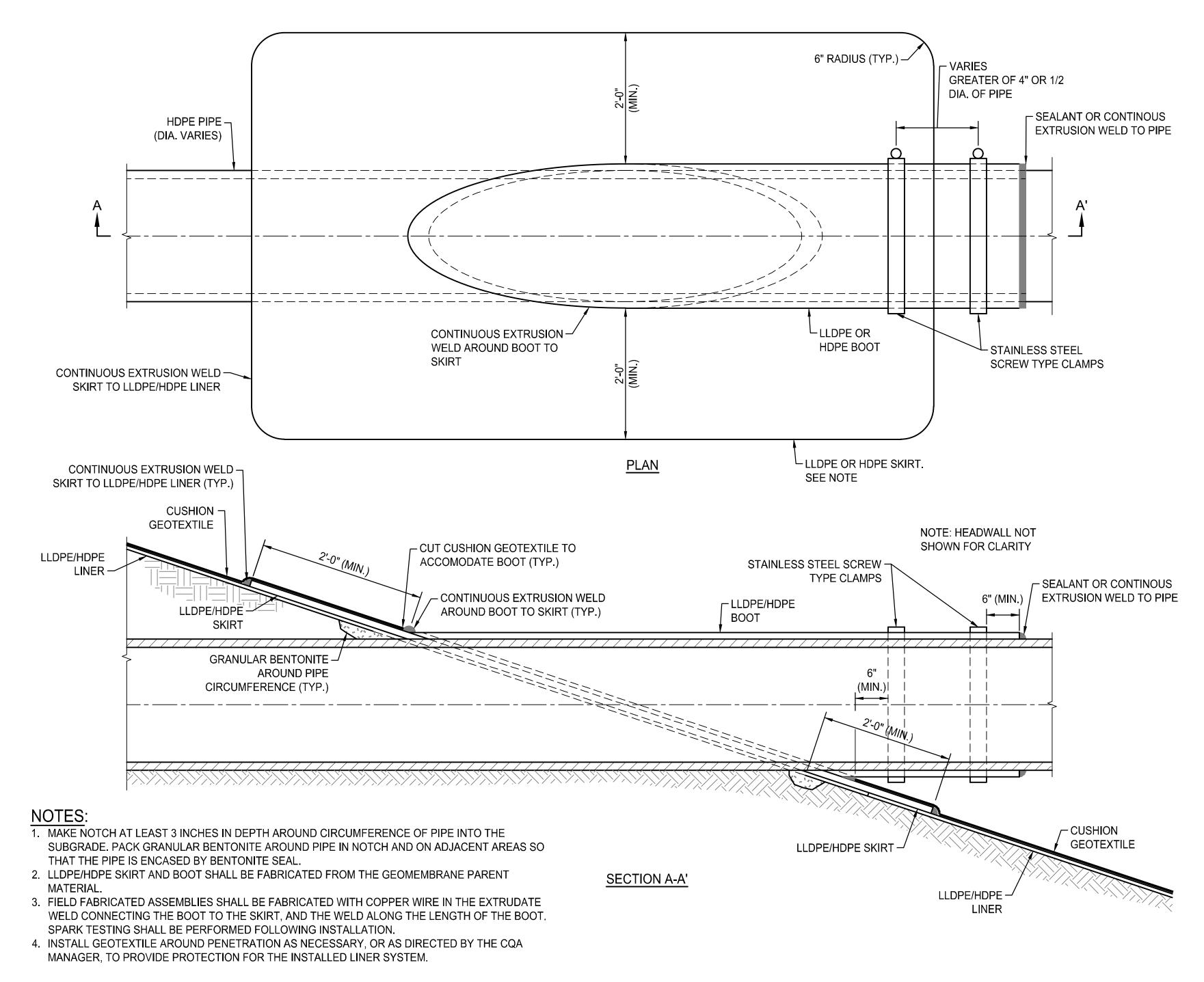
Client/Project OHIO VALLEY ELECTRIC CORPORATION INDIANA-KENTUCKY ELECTRIC CORPORATION POND CLOSURE - LANDFILL RUNOFF COLLECTION POND, CLIFTY CREEK STATION MADISON TOWNSHIP, JEFFERSON COUNTY, INDIANA

CIVIL DETAILS

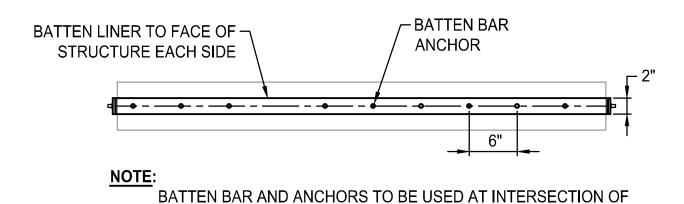
Project No. 175539026

Scale as shown

Drawing No. P-LRCP-501-DTL

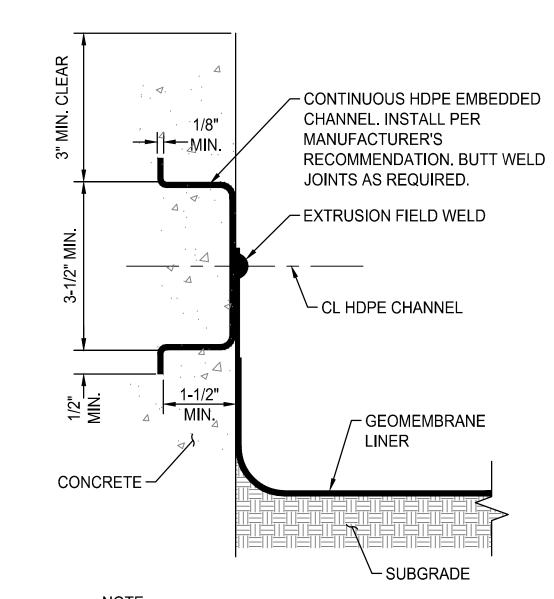


DETAIL - LINER BOOT/CLAMP ASSEMBLY

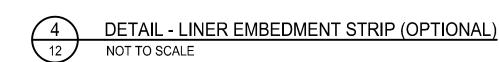


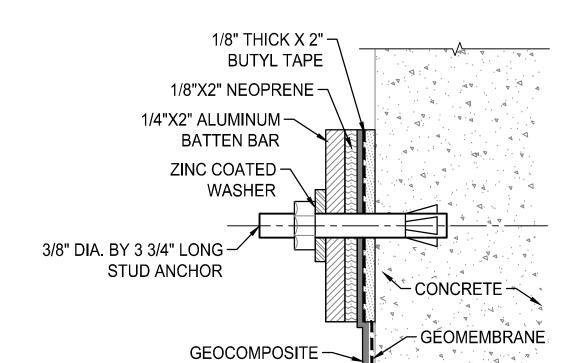
LINER SYSTEM AND CONCRETE STRUCTURES.

DETAIL - BATTEN BAR ATTACHMENT



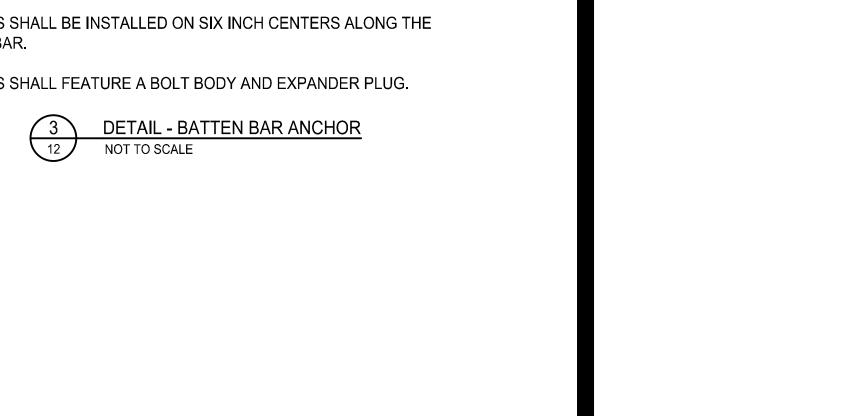
NOTE: LINER EMBEDMENT STRIP MAY BE USED AS AN ALTERNATIVE TO A BATTEN BAR ANCHOR.





1. ANCHORS SHALL BE INSTALLED ON SIX INCH CENTERS ALONG THE

2. ANCHORS SHALL FEATURE A BOLT BODY AND EXPANDER PLUG.



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Issued File Name: LRCP\_39026C-502-DT2 Permit/Seal



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CIVIL DETAILS

Project No. Revision Sheet

Scale

as shown Drawing No.
P-LRCP-502-DTL

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#### **APPENDIX H**

Ditch Sizing Calculations

Ohio Valley Electric Corporation /
Indiana-Kentucky Electric
Corporation
Madison Township, Indiana

Clifty Creek Station
Landfill Runoff Collection Pond



#### Hydrologic and Hydraulic Analysis Ditch Sizing Calculations

#### Purpose:

• Calculations to determine sizing of drainage ditches on the final grade closure plan

#### **Methods:**

PCSWMM was used to size and model the stormwater ditch network and simulate the peak discharges in the ditches. The model was simulated using the Curve Number Infiltration method and dynamic wave flow routing.

Table 1 - Design Standards and References

Parameter	Design Standard/Method/Source
Design Storm	25-Year, 24-Hour Recurrence Interval
Curve Number	TR-55, SCS CN methodology
Rainfall Temporal Distribution	Soil Conservation Service (SCS) Type II (USDA, 1986)
Rainfall Intensity	NOAA Atlas 14 Precipitation Frequency Data Server
Subbasin Area	Delineation based on Permit Drawings Dated
SUDDUSIT Ared	04.16.2021

#### Parameters:

#### Climatological Data:

The 25-year, 24-hour storm was used to determine ditch capacity. Peak rainfall depths for the design storm were taken from NOAA Atlas 14 Precipitation Frequency Data Server specific to the geographic location of the Clifty Creek Plant. The selected Point Precipitation depth is shown in Table 1. An SCS Type II temporal distribution was used to model the rainfall hyetograph over the 24-hour duration.

Table 1 - NOAA Atlas 14 PFDS Rainfall Depths

Storm Return	Rainfall
Interval and	depth
Duration	(inches)
25-year, 24-hour	5.30

#### Watershed Delineation:

Subcatchment delineations were completed in PCSWMM based on the permit design final grade surface. Table 2 provides a breakdown of the subcatchment areas. Subcatchment delineations are shown in Appendix A

#### **Curve Numbers**

The NRCS curve number method was used to estimate infiltration during the design storm event. A composite curve number was generated for the watershed using SCS hydrologic soil group data and land use data determined from aerial imagery. Curve number values for each land use and soil type combination were assigned based on the values published in Tables 2-2a through 2-2d in TR-55 (NRCS, 1986). To model the final closed conditions, land uses of "Meadow" (CN = 78) was assumed for the cap liner system and land use of "Open spaces – Fair" (CN = 84) was assumed for regraded areas around the cap. A summary of curve numbers used in this analysis is provided in Table 2.

Table 2 – Subbasin Drainage Areas and Curve Numbers

Subbasin	Area (acres)	Composite Curve Number
Ash Pond	46.46	98
R1_5	14.29	93
R1_6	22.17	79
R1_7	14.16	78
R5	3.44	92
R6	4.26	80
R7	3.54	94
R8	3.41	77
S2	0.32	94
<b>S3</b>	7.99	78
<b>S4</b>	1.69	83
<b>S</b> 5	0.99	67
SB10	37.16	76
SB11	35.73	75
SB16	5.91	94
SB18	6.21	94
SB6	31.55	76
SB7	84.24	80
SB8	71.00	79
SB9	19.75	75
SB-A	20.89	81
SB-F	12.14	77
SB-G_1	9.41	93
SB-G_2	6.37	89
SB-G_3	5.03	94
SB-H_1	6.66	92
SB-H_2	10.66	93

#### Calculations/Results:

Peak Discharges were calculated in the dynamic PCSWMM model. Peak discharges for each ditch type are shown in the table below

Table 3 – Peak Discharge Calculations

Ditch	Peak 25-year Q (cfs)
Diversion Ditch Upstream	227.1
Diversion Ditch Downstream	380.05

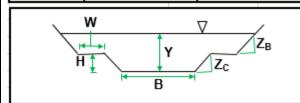
Peak discharges were used to calculate normal depth in the final cover ditches. Ditches were sized to convey the peak discharge accordingly.

#### **Methods:**

Hydraulic calculations were performed to determine minimum ditch size required to convey the peak discharges. Manning's equation was used to estimate flow depth in each ditch. The peak flow calculated was used to size all ditches.

#### Diversion Ditch Upstream Calculations – 25-year, 24-hour storm

#### Normal Depth - Benched Trapezoidal Channel



User I		
Discharge (cfs)	Q	227.1
Bottom Width (ft)	В	10
Side Slope 1 (ft/ft)	Z <sub>C1</sub>	4
Side Slope 2 (ft/ft)	Z <sub>C2</sub>	4
Bench Width Total (ft)	W <sub>1</sub> ,W <sub>2</sub>	0
Bench Stage (ft)	Н	10
Bench Side Slope 1 (ft/ft)	Z <sub>B1</sub>	4
Bench Side Slope 2 (ft/ft)	Z <sub>B2</sub>	4
Channel Slope (ft/ft)	S <sub>o</sub>	0.006
Manning's Roughness	n	0.017

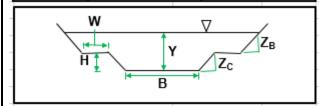
#### **Evaluate Normal Depth**

Normal Depth Output									
Normal Depth (ft)	Y	1.74							
Calculated Flow (cfs)	Qc	227.1							
Flow Area (ft²)	Α	29.4							
Wetted Perimeter (ft)	Pw	24.3							
Hydraulic Radius (ft)	R <sub>H</sub>	1.21							
Top Width (ft)	TW	24							
Average Velocity (ft/s)	V <sub>Avg</sub>	7.71							
Specific Energy (ft)	E	2.66							
Froude Number	FN	1.22							

The maximum estimated depth in the channel is 1.74 feet and is less than the design depth of 4 feet.

#### Diversion Ditch Downstream Calculations – 25-year, 24-hour storm

#### Normal Depth - Benched Trapezoidal Channel



User I	nput	
Discharge (cfs)	Q	380.05
Bottom Width (ft)	В	12
Side Slope 1 (ft/ft)	Z <sub>C1</sub>	4
Side Slope 2 (ft/ft)	Z <sub>C2</sub>	4
Bench Width Total (ft)	W <sub>1</sub> ,W <sub>2</sub>	0
Bench Stage (ft)	Н	10
Bench Side Slope 1 (ft/ft)	Z <sub>B1</sub>	4
Bench Side Slope 2 (ft/ft)	Z <sub>B2</sub>	4
Channel Slope (ft/ft)	S <sub>o</sub>	0.005
Manning's Roughness	n	0.017

#### **Evaluate Normal Depth**

Normal Depth Output								
Normal Depth (ft)	Y	2.22						
Calculated Flow (cfs)	Qc	380.1						
Flow Area (ft²)	Α	46.2						
Wetted Perimeter (ft)	Pw	30.3						
Hydraulic Radius (ft)	R <sub>H</sub>	1.53						
Top Width (ft)	TW	30						
Average Velocity (ft/s)	V <sub>Avg</sub>	8.22						
Specific Energy (ft)	E	3.27						
Froude Number	FN	1.16						

The maximum estimated depth in the channel is 2.22 feet and is less than the design depth of 4 feet.

#### References:

NOAA. (2013). NOAA Atlas 14 Point Precipitation Frequency Estimates. Retrieved July, 2015, from <a href="http://dipper.nws.noaa.gov/hdsc/pfds/">http://dipper.nws.noaa.gov/hdsc/pfds/</a>

USDA. (1986). Urban Hydrology for Small Watersheds, TR-55. United States Department of Agriculture.

Indiana Department of Transportation – 2013 Design Manual, Chapter 202 Hydrology, Revision Date Feb. 2014

#### Attachments:

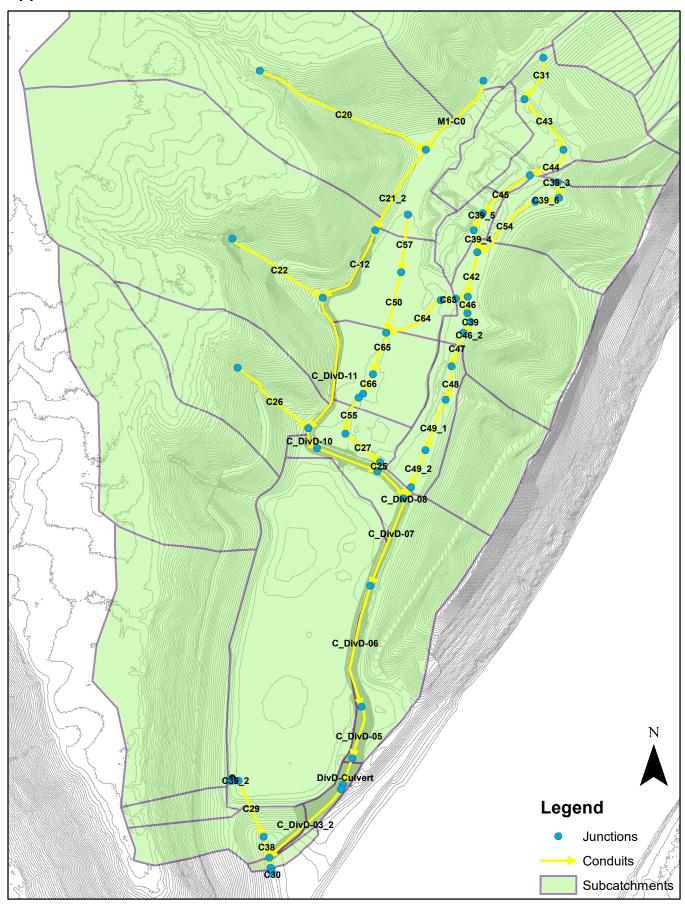
Attachment A: Final Grade Subbasin Boundaries

Calculation Performed by: Stantec Consulting Services Inc.

Prepared by: Brenton Newswanger Reviewed by: Nick Mueller

Revisions:R0

Appendix A - Subbasin Boundaries



#### **APPENDIX I**

Final Cover Soil Loss



Designed by: E Clare

Landfill Runoff Collection Pond Closure Plan

Final Cover Soil Loss Calculations

175539026

Clifty Creek Station

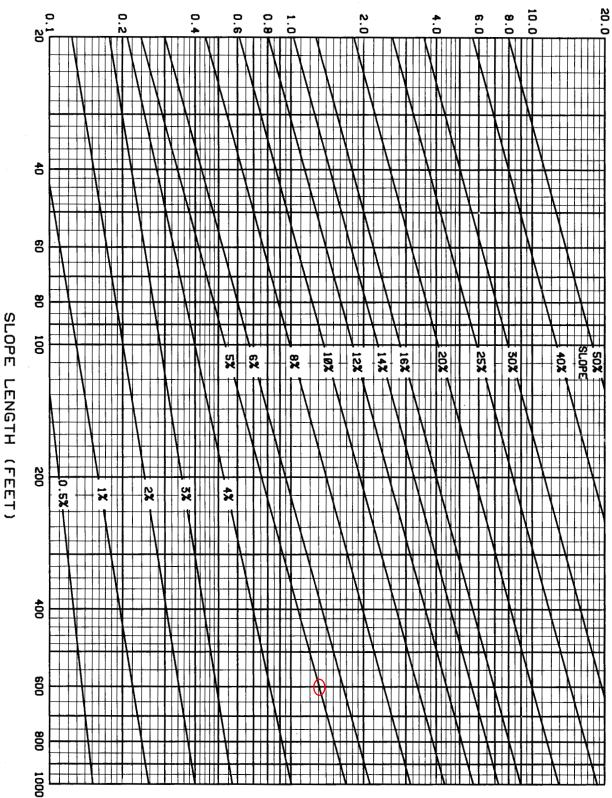
Madison, Jefferson County, Indiana

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	R = r	ainfa	ıll er	osion	ind	ex,															
	LS =									or,											
	P = 6							ctor													
	K = s					1 '															
	C = 1	/egei	tativ	e cov	er ta	acto	ſ.														
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of approxir																					
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Checked by:

N Mueller

TOPOGRAPHIC FACTOR -



(65.41  $\sin^2 heta + 4.56$  sin 0 + 0.065) where  $\lambda = heta$  slope length in feet; heta = heta angle of slope; and heta = 0.2 for gradients < 1 percent, 0.3 for 1 to 3 percent slopes, 0.4 for 3.5 to 4.5 percent slopes, and 0.5 for slopes of 5 percent or steeper.  $LS = (\lambda/72.6)^{III}$ FIGURE 4.—Slope-effect chart (topographic factor, LS).

3/2 Form 4336

#### STATE BOARD OF HEALTH

#### INDIANAPOLIS

OFFICE MEMORANDUM

DATE:

January 3, 1986

m:

James E. Traylor

THRU:

Bruce Palin

FROM:

Duane Leith

Engineering Section

Technical Support Branch

SUBJECT: Guideline for the Evaluation of the

Erosion Potential of Landfill Covers

Sanitary landfill covers should be designed for erosion control in order to avoid later exposure of the refuse and infiltration into the refuse. The design standards which are used can be controlling factors for the size and steepness of a landfill and are therefore often the basis for deciding when a landfill will have to close. It is therefore important to have reasonable and defensible standards. The most recent guidance available to staff in this regard is a memo by Mr. Larry Dunbar dated October 17, 1984, entitled "Evaluation of Design of Final Cover for Landfills." Certain publications recently available to staff and conversations with staff of the Soil Conservation Service have led to the conclusion that the guidance contained herein would be more appropriate than the previously-mentioned memo.

The Governor's Soil Resource Study Commission has proposed goals for erosion and sediment reduction as set forth in their report "Indiana's Erosion and Sedimentation Situation." Briefly, these goals are to reduce erosion on all land to an average annual rate denoted as "T" or the tolerable limit, which prevents depletion of the soil resource and to control all off-site sedimentation by application of best available technology. "T" is the rate at which the soil replaces itself. It is based on factors such as soil depth, texture, and permeability; its value is a matter of judgment rather than being quantifiable. The value of "T" is reported to range between two and five plus tons/acre/year. A value of five tons/acre/year is recommended as a standard under this guideline with an exception as noted.

The currently accepted method for determining the erosion potential of landfill covers is found in Agriculture Handbook 537 Predicting Rainfail Erosion Losses, which is available from SCS offices. Application of the Universal Soil Loss Equation (USLE) from Agriculture Handbook 537 is conducted as follows:

A = RKLSCP, where:

A is the average soil loss in tons per acre calculated on an annual basis for landfill design.

R is the rainfall and runoff factor as obtained from the map Figure 1, inserted between pages 6 and 7 of the handbook, copy is attached to this document. This value ranges from 130 to 225 depending on location.

K is the soil erodibility factor as obtained from Figure 3 on page 11 of the handbook (copy attached), for the composition, structure and permeability of the surface soil as replaced over the landfill. This generally ranges from .30 to .50. If the soil source or characteristics are unknown, it has been recommended that a value of 0.43 be used for landfills.

LS is the combined topographic factor for the length and slope which can be found from Table 3 on page 12, or the chart on page 13 of the handbook, copies of which are attached. Most landfills will have irregular convex slopes. The LS value for irregular slopes is determined by the procedures specified on page 16 of the handbook. The slope is divided into successive equal length segments of uniform slope. The LS factor is obtained for each segment at its respective slope and at a length equal to the total slope length. This LS factor is then adjusted by the "fraction of soil loss" figures shown under the column M = 0.5 in Table 4 on page 15 of the handbook, copies attached. Alternatively, this figure can be obtained from the equation

Soil loss fraction = 1m+1 - (1-1)m+1

#### Nm+1

where: i = segment sequence number, m = slope length exponent of 0.5 for slopes greater than or equal to five percent.

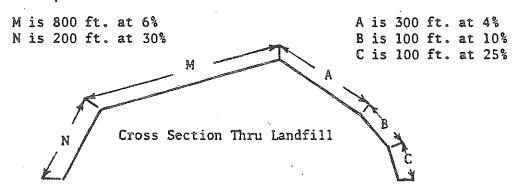
N = the number of equal length segments into which the slope was divided. The sum of the adjusted LS values is the LS value to be used for the entire slope.

C is the cover and management factor which can be found from Table 10 on page 32 of the handbook, or the copy attached. For properly prepared and seeded landfill covers which will be maintained in sod, use a value of 0.01. If a different vegetative cover is planned, adjust the value to reflect the anticipated conditions using Tables 5 through 12 of the handbook. A value of C lower than 0.01 should be allowed only with very intensive specifications regarding cover preparation, fertilization, seeding, and management. In order to establish high productivity, staff of the Soil Conservation Service discussed fertilization rates of 1,000 lbs/acre and contractor prices for fertilization, mulching, and seeding of \$700 to \$1,000 per acre.

<sup>&</sup>lt;sup>1</sup> Verbal communication with Mr. Raymond Sinclair of the Soil Conservation Service on October 7, 1985.

P is the supporting practices factor. For landfills, the value of P is 1.00, unless the site is to be used as cropland. Use the figures from Table 13 on page 35 and Table 14 on page 36 of the handbook as required for landfills used for cropland.

#### Sample calculation:



To determine A values for slopes A, B, C, and M, N:

## For slopes A, B, and C

- R = 175, for Marion County location on the rainfall and runoff map.
- K = 0.43, assumed typical value since actual soil samples have not been tested.
- 3. LS determination:

Total Slope Length	Segment	Slope	LS Value from Figure 4	Adjustment Factor from Table 4	Revised LS Value
500 ft (5	A1 A2	42	.76 .76	.09	.07 .12
segments	A3	42	.76	16 21	.16
of 100 ft	B4	102	3.06	.25	.77
each)	C5	25%	13.20	.28	3.70
					otal 4.82

- 4. C = 0.01, from grass sod, well maintained.
- 5. P = 1.0, since it is not tilled cropland.

Following the USLE: A = RKLSCP

 $A = 175 \times .43 \times 4.82 \times .01 \times 1.00$ , for slope A, B, C

A = 3.63 tons per acre

Since A is less than or equal to five tons per acre, this slope is acceptable.

## For slopes M and N

1000 ft	M	62	2.13	。09	.19
(5	<b>M2</b>	6%	2.13	.16	.34
segments	M3	6%	2.13	. 21	.45
of 200 ft	MA	6%	2.13	. 25	.53
each)	<b>H</b> 5	30%	25.57	. 28	7.16
					Total 9 67

 $A = 175 \times .43 \times 8.67 \times .01 \times 1.00$ , for slope MN

A = 6.52 tons per acre

Since A is greater than five tons per acre, this slope is not acceptable.

It has been suggested that a possible way for a facility to have a cover with an A value higher than five would be to increase the cover depth on the lower slopes. The increased depth can allow for the formation of gullies which can then be stone-lined or similarly stabilized. Whether the increased A value should be allowed and the calculation of the necessary depth increase is not within the scope of this guidance. Erosion control is not the only factor to be considered in cover design. Other factors, such as prevention of ponding, slope stability, drainage, and feasibility of maintenance, will need to be considered.

In conclusion, it is recommended that sanitary landfill covers be designed for an A value not greater than five tons/acre/year, as determined by the Universal Soil Loss Equation from Agriculture Handbook 537. It is further recommended that a K value of 0.43 be used in calculating the A value for typical landfill soils.

References and documents.

- 1. Predicting Rainfall Erosion Losses, 1978, Agricultural Handbook 537, U.S.D.A.
- Indiana's Erosion and Sediment Situation, 1984, Governor's Soil Resources Study Commission.
- 3. Dunbar, Larry, Office Memo to Engineering Staff, October 17, 1984.
- Design and Construction of Covers for Solid Waste Landfills, 1979, EPA-600/2-79-165, U.S. EPA.

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## **APPENDIX J**

LRCP Quality Management Plan (QMP)



# Construction Quality Management Plan (QMP)

Landfill Runoff Collection Pond Closure

Clifty Creek Plant Jefferson County, Madison, Indiana

Issued for Bid - 60% Design

Prepared for:

Indiana-Kentucky Electric Corporation

Prepared by:

Stantec Consulting Services Inc.

May 21, 2021

## **Abbreviations**

CCR Coal Combustion Residuals

CM Construction Manager

CQA Construction Quality Assurance

CQC Construction Quality Control

EDC Engineering during Construction

EOR Engineer of Record

EM Engineering Manager

FR Field Representative

FTP File Transfer Protocol

IKEC Indiana-Kentucky Electric Corporation

LRCP Landfill Runoff Collection Pond

LVWTS Low Volume Waste Treatment System

NCR Nonconformance Reports

OD Observed Deficiency

POD Plan of the Day

QMP Quality Management Plan

RFI Request for Information

WBSP West Boiler Slag Pond

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Introduction

## 1.0 INTRODUCTION

#### 1.1 PURPOSE

The purpose of the Construction Quality Management Plan (QMP) is to promote quality of the constructed work. It consists of three main components, namely, Construction Quality Control (CQC), Construction Quality Assurance (CQA), and Engineering during Construction (EDC). The CQC activities are the Subcontractor's responsibility. The CQA activities are an audit process, performed by the CQA Team, to make sure that the Subcontractor's CQC plan is implemented and on track. EDC activities consist primarily of reviewing and responding to Subcontractor submittals and requests for information (RFIs), and general design support throughout construction.

This Construction QMP provides guidance to the project team and establishes assessment, reporting, and documentation procedures to be implemented throughout the project. Where conflict arises between the requirements of this QMP and the contract documents, the most stringent requirements shall govern.

This QMP describes the CQC and CQA management structure, personnel requirements, and minimum project requirements. This QMP also serves as an outline to develop site-specific protocols based on conditions encountered during the work.

#### 1.2 SCOPE OF WORK

This Construction QMP has been prepared for the Landfill Runoff Collection Pond (LRCP) Pond Closure project.

This project generally consists of the following activities:

- Lowering the pond water surface.
- Constructing a diversion ditch around the perimeter of the pond.
- Grading coal combustion residuals (CCR) to proposed subgrade elevations.
- Installing final cover system over the CCR material.
- Modifying the existing spillway structure.

## 1.3 CONSTRUCTION QUALITY CONTROL AND QUALITY ASSURANCE

The QMP establishes the requirements for CQC and CQA. It outlines roles, responsibilities, CQC and CQA activities, and establishes project processes and procedures. The CQC/CQA Program structure is shown below as Figure 1-1.



Introduction

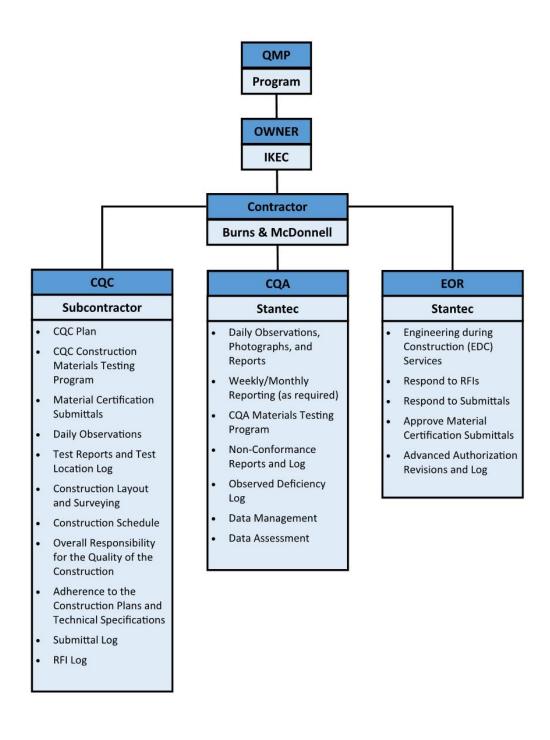


Figure 1-1. General Program Structure for CQC/CQA Program



Introduction

#### 1.4 SURVEY REQUIREMENTS

The Subcontractor shall provide construction layout services to execute the work according to the contract documents. Final as-built surveys and CQC surveys will be conducted by the Subcontractor and may be supplemented with surveys conducted by the Owner, Contractor, CQA Manager, or identified representative. As-built surveys shall be performed under the supervision of a professional surveyor licensed in the state of Indiana.

#### 1.5 LIMITATIONS

The QMP does not include any facility elements outside the limits of construction designated on the Plans for Construction.

#### 1.6 WORKING ON ASH

The Subcontractor shall be aware that there are inherent risks associated with working on CCR, including but not limited to, soft bearing conditions and unstable slopes. failures and instabilities may occur on apparently firm surfaces when loaded, or during active drawdown and dewatering. It is the Subcontractor's responsibility to actively manage their equipment and personnel to safely execute the work, the Subcontractor shall provide an access and excavation work plan that outlines their means and methods for working on CCR surfaces. The plan shall include sequencing of work and note the equipment and materials used.

Subcontractor is responsible for maintaining the stability of the CCR surfaces and perimeter dikes during construction. Subcontractor shall develop a monitoring plan detailing the type of monitoring to be performed (visual, installation of geotechnical instrumentation, or other methods). The plan shall also include, at a minimum, the potential for sloughing or instability of the perimeter dikes due to groundwater levels, how the risks will be mitigated, and how progressive failures along the perimeter dikes will be reduced and mitigated. the monitoring plan shall be submitted to the owner and their representative for review and approval.

The Subcontractor is responsible for all site safety and near surface ash stability at the site and shall follow all OSHA, Contractor, and owner safety requirements including but not limited to man on the ground exclusion zones. Furthermore, the Subcontractor shall provide to the owner a slope stability analysis prepared by a Registered Engineer in the State of Indiana for all excavations 10-feet or greater located outside of the exclusion zones or as directed by the Contractor. All evaluated excavations will require a minimum factor of safety of 1.3. The Subcontractor shall also prepare and provide to the owner for review a plan outlining all exclusion zones to include, but not limited to, barriers, setbacks, signage, and guidelines for entry.



Organization and Responsibilities

## 2.0 ORGANIZATION AND RESPONSIBILITIES

#### 2.1 PROGRAM ROLES AND RESPONSIBILITIES

The major participants in the project are listed below along with a description of their roles. An organizational chart is provided in Figure 1-1.

#### 2.1.1 Owner

The plant and its ancillary functions are owned and operated by the Indiana-Kentucky Electric Corporation (IKEC). The Owner has contracted with the Contractor to serve as the Construction Manager (CM) who shall approve any design and/or QMP revisions. The Owner will administer communication with any regulatory agencies, including any related permit modifications.

#### 2.1.1.1 Construction Manager (CM)

The Construction Manager (CM) is responsible for coordinating with the Subcontractor regarding contractual issues, including scope, budget, and schedule. In addition, the CM will provide daily oversight of the construction activities. The CM has the authority to stop work that is not in conformance with the Plans for Construction and Technical Specifications. The CM is responsible for tracking submittals and RFIs. The CM will then distribute the submittals and RFIs, as needed, to the appropriate party.

In addition, the CM or qualified representative shall perform at least monthly inspections of the LRCP facility. These inspections shall include observations of all outslopes for indications of slope instability including tension cracks, sloughs, and excessive seepage. These inspections shall be documented and retained within the project records. Any suspect site conditions shall be promptly reported to the Owner.

#### 2.1.2 Engineer of Record (EOR)

The Engineer of Record (EOR) is responsible for development of the technical components of the project. During field implementation, the Engineer will provide support to the CQA Team for construction observation and CQA services.

Specific examples of Engineer responsibilities include responding to technical RFIs and submittals, supporting the CQA team, and review of field-testing data.

#### 2.1.3 CQA Manager

The Construction Quality Assurance (CQA) Manager shall be responsible for the execution of the CQA program and related documentation as outlined in the QMP for all work performed. The CQA Manager shall be a Professional Engineer licensed in the state of Indiana.

#### 2.1.4 Contractor

The Prime Contractor (Contractor) holds the Engineering, Procurement, and Construction contract directly with the Owner. The Contractor is responsible for coordinating the design, procurement, and construction work to complete the project. The Subcontractor (CQC Team), CQA Team and Engineer are contracted with the Prime Contractor for execution of the project.



**Quality Assurance** 

#### 2.1.5 Subcontractor

The Subcontractor is responsible for execution of the work in accordance with the contract documents. The Subcontractor is solely responsible for field implementation activities, including instrumentation and monitoring. The Subcontractor will collaborate with the CM to propose adjustments (if needed) to the scope of work depending on encountered conditions. Review and approval of scope of work adjustments shall be made by the Engineer.

Specific examples of Subcontractor responsibilities include, but are not limited to, data collection and providing that data to the Owner; development and implementation of the CQC Plan, including development of applicable procedures, processes, and work plans; reporting in accordance with the Technical Specifications; CQC materials testing; quantity tracking; conformance to the Technical Specifications; and structure and ground movement monitoring data collection and reporting.

#### 2.1.5.1 Construction Quality Control Manager

The Subcontractor's CQC Manager shall have a minimum of five years of construction experience and be a full-time on-site employee of the Subcontractor. The Subcontractor CQC Manager is responsible for the Subcontractor's tests, inspections, processes, and related actions during and after construction execution to evaluate that both the actual products used, and the completed construction comply with the requirements of the Plans for Construction and Technical Specifications. The CQC Manager shall report to the Subcontractor's principal officers and the CM.

#### 2.2 STOP WORK AUTHORITY

The EOR, CQA Manager, CM, and CQC Manager may exercise stop work authority when concerns related to quality are identified. In situations where personnel safety is concerned, any project personnel may stop work at any time.

## 3.0 QUALITY ASSURANCE

Construction Quality Assurance (CQA) is the responsibility of the Contractor, Engineer, and CQA Team. CQA includes assessments, observations, and reporting to document that the implementation of the work performed by the Subcontractor meets the requirements of the Plans for Construction and Technical Specifications. The CQA Team functions as the field representative for the Engineer through performance of assessments, verifications, and observations. Specific CQA responsibilities are listed as follows:

- Daily field report of construction activities including photographs.
- Noting observed deficiencies (ODs) during construction that require correction.
- Reviewing applicable Subcontractor submittals and Requests for Information (RFIs) related to quality for adherence to project requirements.
- Reviewing test data for compliance with project requirements and specifications.
- Performing assessments of the Subcontractor CQC Plan to ensure adherence to the QMP.
- Reviewing the Subcontractor documentation deliverables for conformance with project requirements.



**Quality Assurance** 

- Periodic material testing to audit CQC test results.
- Reporting of test results.
- · Special testing requested by the Owner.

#### 3.1 QUALITY ASSURANCE TEAM

#### 3.1.1 Roles and Responsibilities

The CQA Team serves as observers of field implementation of the construction documents and to provide CQA documentation. The CQA Team is responsible for the execution of the CQA Plan and related documentation as outlined herein. Individual roles and responsibilities for the CQA Team members are defined below. Personnel assignments are subject to change, if qualification requirements are met and approved by IKEC.

#### 3.1.1.1 Construction Quality Assurance (CQA) Manager

A professional engineer licensed in the State of Indiana shall be designated as the CQA Manager. The CQA Manager shall be responsible for administering the CQA program and advising the CQA Team.

The CQA Manager will be responsible for the following tasks:

- Observing conformance with the QMP by reviewing and documenting project records and activities;
- Managing overall implementation of the CQA program;
- Evaluating the testing results of the CQC program;
- Evaluating work for conformance with the project plans and specifications and notifying the CM if work is non-compliant with the contract documents;
- Managing the documentation of all CQA activities;
- Reviewing progress of the work and reports prepared by the Subcontractor as part of the CQC Plan:
- Verify the appropriate test standards are used for the methods to conduct assessments and field and laboratory testing for CQA testing;
- Evaluating and auditing the results of CQC and CQA assessments and testing; and
- Review daily field reports prior to submittal to the CM and EOR.

#### 3.1.1.2 Construction Quality Assurance (CQA) Supervisor

The Construction Quality Assurance (CQA) Supervisor provides oversight for the CQA Field Representative(s) on site. The CQA Supervisor is familiar with the materials to be used, the observations and testing to be done, and the functional intent of the QMP. The CQA Supervisor has responsibility for:



#### **Quality Assurance**

- Coordination of the periodic CQA construction testing in the field;
- Coordination of other testing with a commercial laboratory (as needed);
- Provides support to field CQA staff;
- Plans and directs the activities of CQA field representative(s); and
- Reviews Daily Field Reports.

In conjunction with their staff, the CQA Supervisor reviews daily field reports and directives, and reports to the CQA Manager any situation where the Plans for Construction and Technical Specifications do not appear to be appropriate for the conditions encountered.

The CQA Supervisor reviews Nonconformance Reports and has authority to stop work due to adverse quality conditions or potentially unsafe work practices. The CQA Supervisor reports to the CQA Manager. The CQA Supervisor will be on-site an average of one day per month for the duration of construction, or as necessary to observe key construction activities to support the CQA team.

#### 3.1.1.3 Construction Quality Assurance (CQA) Field Representative (FR)

The Construction Quality Assurance Field Representative (CQA FR) staff shall consist of qualified personnel working under the direct supervision of the CQA Manager and CQA Supervisor. The CQA Supervisor will be responsible for the day-to-day coordination and management of the CQA FRs.

The CQA FR is responsible for performing quality assurance in the field, and for performing observations of conformance with the Plans for Construction and Technical Specifications. The CQA FR will document the results of the required CQA observations and testing and inform responsible personnel about unsatisfactory items. The CQA FR is also responsible for documenting that corrective actions are taken to resolve the conditions. For defective work, the site-specific CQA FR will initiate a Nonconformance Report and submit the report to the CQA Supervisor.

The duties of the CQA FR are listed below:

- Daily observations of construction activities to verify conformance with project Plans for Construction and Technical Specifications;
- Observe on-site testing performed by CQC team members;
- Perform periodic on-site CQA testing to verify CQC procedures and test results;
- Coordinate required sampling with commercial laboratory for other quality control testing (as needed);
- Prepare and submit daily field report of observations, testing results, and photographs;
- Conduct periodic inspection of specific construction items;
- Instrumentation monitoring (as needed);
- Verification that testing is performed and that results meet the Technical Specifications;



**Quality Control** 

- Verification of Subcontractor's CQC surveying;
- Reporting of nonconformances; and
- · Reporting of observed deficiencies.

#### 3.1.2 Materials Testing

The CQA Team will perform selected CQA sampling and material testing to audit CQC procedures and test results. The material testing schedule is included in Attachment A. The schedule specifies the anticipated types and minimum number of tests for each material subject to testing and required frequency of testing. The CQA Manager or identified representative is responsible for reviewing the material testing and results and manufacturer's supplied information for conformance to the Technical Specifications.

#### 3.2 QUALITY ASSURANCE DAILY FIELD OBSERVATIONS

#### 3.2.1 General

The CQA Team shall review the Plans for Construction and Technical Specifications for each day's construction activities. After observation of the day's activities, they shall document whether the work that was observed has been done in accordance with the Plans for Construction and Technical Specifications. The CQA Team's observation of work serves as an audit function. It is not to be considered a verification that all work performed was in accordance with the Plans for Construction and Technical Specifications. That responsibility remains with the CQC Team.

Daily observations are to be documented in a CQA daily field report.

Any suspect conditions shall be promptly reported to the CM. Each observation shall be documented on the Daily Field Report form for inclusion with the project records.

#### 3.2.2 Conformance Verification

Conformance verification shall consist of observing and documenting testing performed by the Subcontractor to ensure that the required tests and evaluation of materials and construction products are performed. The CQA Team shall confirm that testing is performed at frequencies specified in this QMP. The CQA Team shall perform periodic testing to verify the results of the Subcontractor. Additional or supplementary conformance testing may be added at the discretion of the CQA Manager. Results shall be reviewed by the CQA Manager to assess conformance with project requirements. Copies of all conformance results shall be included on a CQC testing log and with the project records.

The NCR process shall be used, as needed, to immediately report deficiencies, remediation required, and resolution to the Contractor and Engineer. NCRs may be submitted to the CQA Manager by the CQA Team or Subcontractor. The CM will maintain a log as a record of the non-conformances encountered and the final resolution. A detailed description of the NCR process is provided in Subsection 5.2.5.

### 4.0 QUALITY CONTROL

Construction Quality Control (CQC) and overall construction/material quality is the responsibility of the Subcontractor. CQC includes establishing procedures and work plans, performing observations,



**Quality Control** 

documenting construction processes and performance, and performing materials testing to demonstrate the quality of the constructed elements. Specific CQC items are as follows:

- Ensuring that the Subcontractor's work complies in all respects to the Plans for Construction, Technical Specifications, other contract documents, and any approved changes to the contract documents.
- Developing the CQC Plan.
- Developing work plans, procedures, and submittals related to the work.
- Preparing a daily log of observations and activities.
- Providing data collected during this project to the Contractor.
- Demonstrating the means and methods for complying with the Plans for Construction and the Technical Specifications.
- Performing construction staking and layout.
- Performing construction materials quality control testing and reporting.
- Maintaining an updated construction schedule with CQC/CQA hold point milestones represented.
- Maintaining calibrations on measuring and testing equipment.
- Providing final as-built surveys and drawings.
- Testing logs, timely submittal of required deliverables, reporting nonconforming conditions to the Engineer, and data management.

#### 4.1 SURVEY REQUIREMENTS

The Subcontractor shall provide field layout services for the purposes of executing the work according to the contract documents. The work shall be laid out and constructed to the elevations shown in the Plans for Construction and in accordance with the Technical Specifications. Tolerances shall be as defined in the Technical Specifications.

Final as-built surveys and CQC surveys will be conducted by the Subcontractor and may be supplemented with surveys conducted by the Contractor or identified representative.

#### 4.2 CQC EXECUTION AND PLAN PREPARATION

The Subcontractor shall prepare a CQC Plan meeting the requirements of this QMP and the project specifications. The CQC Plan must be approved by the Contractor, CM, and Engineer prior to the start of construction. The Subcontractor shall execute CQC activities in accordance with the approved CQC Plan.



**Quality Control** 

#### 4.3 CQC PLAN REQUIREMENTS

The Subcontractor is responsible for establishing and maintaining a CQC Plan for the project to ensure that the project is executed, and items are installed in accordance with the Plans for Construction and Technical Specifications. The objective of the CQC Plan is to provide a framework where a quality product will be produced. The details of the CQC system will be described in the CQC Plan document, which will establish procedures to ensure uniformity and provide a standard by which comparisons can be made.

The CQC Plan shall include, at a minimum, the following to cover all operations, both on-site and off-site, including work by subcontractors and suppliers:

- Organizational Structure: Chart showing the CQC organizational structure, including line of authority.
- Personnel: Names and qualifications, in resume format, for each person in the QC organization.
- Duties, Responsibilities, and Authorities: Duties, responsibilities, and authorities of each person in the QC organization.
- Outside Organizations: List of outside organizations, such as consulting engineering firms, that will be employed by the Subcontractor and a description of services these firms will provide, including decision-making authority (if any).
- Scope of Work: Scope, including testing laboratory information and accreditations and materials testing schedule.
- Submittals: Procedures for reviewing, approving, and managing submittals. Include the name(s) of the person(s) in the QC organization authorized to prepare required submittals, and the initial submittal of the submittal register as specified in the section entitled "Submittal Procedures."
- Completing Rework Items: Procedures for addressing nonconformance, deficient, and rework items.
- Measuring and Testing Equipment: Copies of current certifications for monitoring and testing equipment.
- Documentation Procedures: Documentation procedures, including proposed report/forms formats.
- Training Requirements: Documentation of personnel trained in specifics of the CQC Plan.
- Work Plans and Quality Process Documents.

#### 4.4 TESTING SCHEDULE

It is the responsibility of the Subcontractor to perform tests specified in the Technical Specifications and verify that control measures are adequate to provide a product that conforms to the requirements of the project documents. A material testing schedule outlining minimum CQC and CQA testing frequencies is included in Attachment A. The schedule specifies the anticipated types and minimum number of tests for each material subject to testing. The CQC Manager or identified representative is responsible for verifying



**Quality Control** 

that material testing and results conform to the Technical Specifications. The Subcontractor shall maintain a log of all CQC material test results.

### 4.5 SUBMITTALS AND REQUEST FOR INFORMATION

#### 4.5.1 Submittals

Subcontractor submittals shall be provided to the CM, consistent with the requirements of the Technical Specifications. For material submittals, the Subcontractor shall review and certify that the material conforms to the Plans for Construction and Technical Specifications prior to submittal to the CM.

#### 4.5.2 Request for Information

The Subcontractor shall communicate issues such as constructability, discrepancies in the plans, and requests for Engineer support during field implementation, etc., using the RFI form. RFIs shall be submitted by the Subcontractor to the CM. The CM routes the RFI to the Engineer. The Engineer will prepare a response to the RFI and submit it to the CM. The CM then sends the completed RFI response to the Subcontractor.

The CM, or representative, shall document each RFI in an RFI log and in the project records. The RFI log will be maintained by the CM, or representative, and will be reviewed at the weekly project progress meetings. The Subcontractor is encouraged to engage the Engineer and the Engineering Team prior to RFI submittal in efforts to streamline the RFI process. The Engineer has seven calendar days upon receipt of the RFI to provide a response to the CM. If a quicker response is required by the Subcontractor, this should be noted in the RFI and in the correspondence to the CM.

Any changes to the project that result from the RFI process shall be documented and communicated to the CQA Team and the Subcontractor. Communications shall include discussion of the issue that led to the RFI, the intent of the RFI response, and any resulting changes to the project.

It should be noted that, because of the nature of the project, the RFI process may not be suitable for some of the day-to-day adjustments that will be necessary. Instead, daily collaboration between the CM, Engineer, and Subcontractor field staff will be essential to successfully meet the intent of the project. Such daily adjustments shall be documented in both the CQC daily field report and the CQA daily field report.

#### 4.6 MEETINGS

#### 4.6.1 Orientation Meeting

An orientation meeting (i.e., kickoff meeting) shall be held before field implementation of the QMP. At a minimum, those present will include the CM, the Contractor, the Subcontractor's PM, the CQC Manager, the CQA Manager, CQA FR, IDEM, and others as needed. This meeting will include a review of the project document objectives, quality management processes, hold points, and special project requirements. The Subcontractor will prepare meeting minutes and distribute them for review. The Owner shall notify the IDEM permit manager 10 working days prior to the meeting.

Other kickoff meetings may be required before the start of discrete phases of the work.



**Quality Management Documentation** 

#### 4.6.2 Daily Meetings

Daily plan-of-the-day (POD) meetings related to safety and construction activities for the day's work shall be conducted by the CM or designated representative. Attendees shall include the CQA FR and the CQC Manager. Other key participants from IKEC, the Engineer, and the Subcontractor will be included in these meetings as appropriate.

## 4.6.3 Weekly Meetings

The CM will hold weekly on-site meetings with the project team during active construction. Portions of these meeting will be allotted to discuss quality and engineering. Those present shall include the CM; CQA FR; and the CQC Manager and on-site representative. Other key participants from IKEC, the Engineer, Contractor, and the Subcontractor will be included in these meetings as appropriate.

The primary purpose of the weekly meetings shall be to confirm that all parties involved with field activities are familiar with the design, required procedures, and associated quality objectives, along with any issues (e.g., safety, environmental) related to field implementation. Topics to be addressed at this meeting shall include a review of the schedule, any outstanding RFIs, outstanding change orders, status of Subcontractor submittals, and quality issues. The CM, or representative, shall provide minutes of each meeting for inclusion in the project records.

#### 4.6.4 Additional Meetings

Other on-site meetings will be organized to address site-specific issues that need quick resolution but are not conducive to the weekly or other regularly scheduled meetings or that require specific personnel to be present and to work through specific issues as they arise. Such meetings will be documented on the CQC and CQA daily field reports.

## 5.0 QUALITY MANAGEMENT DOCUMENTATION

#### 5.1 PROJECT DOCUMENTATION

Project CQA documentation shall be obtained and maintained by the CQA Manager and copied to the Engineer during all phases of field implementation. Project CQC documentation shall be obtained and maintained by the CM or identified representative and copied to the Engineer. The Subcontractor is required to submit all data collected, both raw and processed, to the Contractor. Transfer of these data should be through Procore or similar file management platform.

Distribution of the project documentation shall be in accordance with Table 5-1.



**Quality Management Documentation** 

Table 5-1. Reporting Responsibility

Item	Originator	Primary Recipients	Secondary Recipients
Daily CQA Report	CQA FR	CQA Supervisor	CM, EOR, CQA Manager, Subcontractor
Daily CQC Report	CQC Team	СМ	CQA Manager, EOR
Request for Information (RFI): Submittal	Subcontractor	СМ	EOR CQA Manager
Request for Information (RFI): Approval and RFI Log	СМ	EOR	CQA Supervisor CQA Manager
Observed Deficiency Log	CQA FR, CQA Supervisor	CM, CQA Manager	Subcontractor
Nonconformance Reports and Log	CQA Supervisor, CQA Manager	CM, EOR	Subcontractor
Subcontractor Submittals	Subcontractor	СМ	EOR, CQA Supervisor
Subcontractor Submittal Approval and Log	Subcontractor	СМ	EOR, CQA Manager
CQC Testing Results and Log	CQC Team	СМ	CQA Manager, EOR
CQC Correspondence and Log	Subcontractor	СМ	CQA Manager, EOR
Design Revisions and Log	EOR	CQA Manager	Subcontractor, CM
Subcontractor Daily Production Report	Subcontractor	СМ	EOR
Weekly Meeting Minutes	СМ	All Present	N/A
Construction Certification Report	CQA Manager	EOR	СМ



**Quality Management Documentation** 

#### 5.2 CQA DOCUMENTATION

The CQA documentation shall include, but is not limited to, the following:

- · Daily field report;
- RFI responses;
- ODs;
- NCRs;
- CQA correspondence (i.e., memos, letters);
- · Photographic documentation; and
- Plans for Construction and Technical Specifications revisions and log.

#### 5.2.1 CQA Daily Field Report

The CQA Team shall maintain daily field reports to document daily observations, investigations, and analyses of the construction, as well as to document the progress of the work. These reports shall include photographic documentation, where applicable. The CM will provide daily tracking of quantities, consistent with the Subcontractor pay items (hard quantities, standby hours tracked separately by CM). This daily tracking will allow independent assessment of the Subcontractor quantities in the team's daily field report.

## 5.2.2 Photographs

The CQA Team shall maintain a photographic record of the field implementation, documenting the progress of project construction. For the purpose of construction, photographs will be taken as needed to document processes, procedures, and any deficiencies or nonconformance, and will provide a photographic record for inclusion in the project record. Each photograph shall have the date recorded, the name of the person taking the photo (if by someone other than the daily field report author), and the location of photographs. This information shall be noted in the daily field report or the photographic log.

#### **5.2.3** Material Testing Reports

CQA material testing reports will be compiled and distributed to the project team. Daily summaries and offsite test CQA results will be included in the CQA Daily Field Report. Reports shall include CQA testing results for both laboratory and field testing.

## 5.2.4 Observed Deficiencies (ODs)

Observed Deficiencies (ODs) shall identify and document deficiencies in quality, workmanship, materials, equipment, or supplies and unauthorized deviations from Plans for Construction or the Technical Specifications. The OD log is used to track and rectify deficient events that do not need to be escalated to the NCR level. The following procedures shall be used to document ODs:

 When the Subcontractor, Engineer, or CM notices an observed deficiency, the CQA FR records it in the corresponding CQA daily report.



**Quality Management Documentation** 

- The CQA Supervisor will enter the OD into the Observations tool in Procore along with the following information: referenced daily report number, observed date, deficiency description, reference to the corresponding requirement (if applicable), hold point (if applicable), and responsible party.
- The CQA Team, CM, and the Subcontractor will work to establish a resolution and corresponding timeframe for the OD.
- Once a corrective action for the OD has been agreed upon by all involved parties, it will be implemented by the responsible party (if necessary).
- After the OD is verified to be resolved, the CQA FR will record it in the corresponding CQA daily report.
- The CQA Supervisor will enter the following information into the Observations tool in Procore: referenced daily report number, the resolved date, and corrective action description.
- The Observations tool in Procore will be maintained by the CQA Supervisor for tracking the status of OD events.

#### **5.2.5** Nonconformance Reports

Nonconformance Reports (NCRs) shall identify, report, and document a nonconforming event in quality, workmanship, materials, equipment, or supplies and unauthorized deviations from Plans for Construction or the Technical Specifications. The following procedures shall be used to report nonconformance:

- When a nonconformance is observed, whether by the Engineer, Subcontractor, or CM, the CQA FR prepares an NCR that describes deficiencies noted (including time, actions, locations, etc.) and references to the corresponding requirements. Additionally, a list of corrective actions is identified for the Subcontractor to complete, or meet, to the satisfaction of the Engineer.
- The NCR form is then routed to the Engineer for assessment, with a disposition on the installed work/materials that need to be replaced, if any. The form is then routed to the CM for finalization.
- The CM then routes the NCR package to the Engineer for final review and signature and returns it to the CM, if acceptable. Once that process is complete, the CM issues the NCR to the Subcontractor.
- The Subcontractor provides a list of proposed corrective actions on the form and returns it to the CM for processing and issuance to the CQA Manager. If the proposed corrective action is acceptable, the CQA Manager signs the NCR and includes a list of required documentation to be provided by the Subcontractor to confirm completion of the corrective action. The NCR is then returned to the CM for processing and issued to the Engineer for final approval and signature. The form is then routed to the CM for issuance to the Subcontractor for implementation. If the proposed corrective action is not deemed acceptable, the CQA Manager returns the form to the CM with additional comments for issuance to the Subcontractor for review and revision.
- The process is repeated until the Subcontractor's proposed corrective actions are acceptable to the CQA Manager.
- On completion of the rework identified as deficient or nonconforming, the CQA Manager will
  conduct a reassessment of the items noted in the NCR. If the reworked items are found acceptable,



**Quality Management Documentation** 

it will be so noted on the NCR. If, however, the items are still not acceptable to the CQA Manager, the items will be rejected and must be reworked before it is resubmitted for further assessment.

- The CM and the CQA Manager will periodically review the status of NCRs and will work with the Subcontractor to establish a timetable for the final resolution of all deficiencies.
- A Nonconformance Log will be maintained by the CM for tracking the status of nonconforming items.

## 5.3 PLANS AND TECHNICAL SPECIFICATIONS REVISIONS

Periodically during construction, changes to the Plans for Construction and/or Technical Specifications may be required. These changes will be reviewed and drafted by the Engineer and approved by the Engineer prior to field implementation. Any deviations and changes to the Plans for Construction and Specifications require review and concurrence from the Contractor. The revisions will be included on a revision log and in the project records.

#### 5.4 CQC DOCUMENTATION

The CQC documentation shall include, but not be limited to, the following:

- CQC Daily Field Report;
- Field Observation Logs and Test Data Sheets;
- Subcontractor Submittals and Shop Drawings;
- Material Conformance Test Results;
- Construction Problem and Solution Reports;
- Photographic Documentation;
- Design and/or Specification Modifications; and
- Meeting Minutes.

#### 5.4.1 CQC Daily Report

The CQC report shall include a summary of work performed for the day; CQC tests performed; test results; and a "remarks" section that will contain pertinent information, including significant observations, problems encountered during field implementation, and delays encountered. The following CQC data is to be provided in the report:

- Date.
- Weather.
- Quantities of material received on-site with corresponding delivery tickets.



**Quality Management Documentation** 

- Quantities of materials used.
- List of CQC tests performed.
- Remarks section outlining any issues, delays, etc., that were encountered during the day.
- Updated drawings, mapping, and graphical representations of field work.
- Quantities of materials and/or debris to be hauled off-site and disposed of at locations not owned, operated, or maintained by the Owner, including proper chain of custody, required haul tickets, and scale tickets.
- Daily hold points.
- Photographs of daily construction activities.

In addition to this reporting, refer to the Technical Specifications for more detailed requirements.



## **ATTACHMENT A**

**Material Testing Schedule** 

# LRCP Closure Quality Management Plan

## **Materials Testing Schedule**

MATERIAL	PROPERTY	TEST	VALUE	QC FREQUENCY	QA FREQUENCY
Concrete					
Cast-in-Place Concrete	Compressive Strength	ASTM C39	4,500 psi	1 / 50 CY placed or 1 / day whichever is more frequent	1 / 200 CY placed. Also review QC laboratory reports
	Entrained Air Content	ASTM C231	2.5 - 6.0 %	1 / delivery	Observe Only
	Slump	ASTM C143	3 ± 1	1 / delivery	Observe Only
CCR Fill					
	Standard Proctor	ASTM D698	laboratory test	1 / change in material	Review laboratory reports
	Field Density and Moisture	ASTM D6938	Min. 85%, within 2% of optimum moisture content	5 / acre per lift	1 / acre per lift
Flexible Membrane Liner (FML)					
40 mil LLDPE Geomembrane	Large scale direct shear testing	ASTM D5321	laboratory test	1 test per material interface	review laboratory reports provided by CQC Manager
	Seam Properties - Shear Strength	ASTM D6392	Fusion - 60 lbs/in , Extrusion - 60 lbs/in	Cut 1 sample / 500 linear feet of weld	destructive test sample provided by CQC Manager
	Seam Properties - Peel Strength	ASTM D6392	Fusion - 50 lbs/in , Extrusion - 44 lbs/in	Cut 1 sample / 500 linear feet of weld	destructive test sample provided by CQC Manager
	Trial Welds	ASTM D6392	Fusion: 50 lbs/in peel; 60 lbs/in shear Extrusion: 44 lbs/in peel; 60 lbs/in shear	2 / operator / machine / day (morning and mid-shift, max 4 hr work intervals)	Observation Only
	Vacuum Testing	ASTM D5641	Standard Practice for Geomembrane Seam Evaluation by Vacuum Chamber.	1 / extrusion weld and repair location	Observation Only
	Air Pressure Testing	ASTM D5820	Standard Practice for Pressurized Air Channel Evaluation of Dual Seamed Geomembranes	1 / wedge weld and repair location	Observation Only

# LRCP Closure Quality Management Plan

## **Materials Testing Schedule**

MATERIAL	PROPERTY	TEST	VALUE	QC FREQUENCY	QA FREQUENCY
Soils			CL, CH, MH, or ML, CL-ML, SC, or SM-SC according to the Unified Soil Classification System, or a	1 / source / change in material	Review laboratory reports
Cover Soil	Soil Classification	ASTM D2487	combination of these groups		
	Standard Proctor	ASTM D698	laboratory test	1 / source / change in material	Review laboratory reports
	Field Density and Moisture	ASTM D6938	Min. 92%, within 2% of optimum moisture content	5 / acre / lift	1 / acre / lift
Vegetative Cover	Soil Classification	ASTM D5268	Soils used for vegetative cover shall conform to the requirements set forth in ASTM D5268, unless otherwise approved based on the soils ability to sustain vegetation	1 / source / change in material	Review laboratory reports
	Agronomic Testing	-	laboratory test	1 / 10 acres	Review laboratory reports
Clay Berm Soil	Soil Classification	ASTM D2487	CL, CH, MH, or ML, CL-ML, SC, or SM-SC according to the Unified Soil Classification System, or a combination of these groups	1 / source / change in material	Review laboratory reports
	Standard Proctor	ASTM D698	laboratory test	1 / source / change in material	Review laboratory reports
	Field Density and Moisture	ASTM D6938	Min. 95%, within 2% of optimum moisture content	5 / acre / lift	1 / acre / lift
Anchor Trench Backfill	Standard Proctor	ASTM D698	laboratory test	1 / source / change in material	Review laboratory reports
	Field Density and Moisture	ASTM D6938	Compact each layer of material with at least three passes of a vibratory plate compactor with in-place moisture within -2% to +2% of optimum moisture content		1 / 500 linear feet of trench

# LRCP Closure Quality Management Plan

### **Materials Testing Schedule**

MATERIAL	PROPERTY	TEST	VALUE	QC FREQUENCY	QA FREQUENCY
Utility/Pipe Trench Backfill	Standard Proctor	ASTM D698	laboratory test	1 / source / change in material	Review laboratory reports
	Field Density and Moisture	ASTM D6938	Min. 95%, within 2% of optimum moisture content	1 / 100 linear feet of trench	1 / 500 linear feet of trench
HDPE Pipe					
HDPE Gravity Pipe	Low Pressure Air Test	ASTM F1417	Standard Practice for Installation Acceptance of Plastic Non-pressure Sewer Lines Using Low-Pressure Air	1 / pipe run	Observation Only
Materials Delivered to Site	Defects	Visual Observation	no defects	1 / material delivery	1 / material delivery
	Conformance to Submittals	Visual Observation	conforms to plans	1 / material delivery	1 / material delivery

#### Notes:

<sup>(1)</sup> Testing frequency may be adjusted as directed by the CQA Manager

<sup>(2)</sup> This table does not include all required quality control testing. The Subontractor shall be solely responsible for the proper implementation of its Quality Control Program.

## **APPENDIX K**

Post Closure Plan

# Post-closure Plan CFR 257.104(d)

## **Landfill Runoff Collection Pond**

Clifty Creek Station Madison, Indiana

April 2021

Prepared by: Indiana-Kentucky Electric Corporation 3932 U.S. Route 23 Piketon, OH 45661



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Г О	ION OF THE CCR UNITION OF THE POST-CLOSURE PLAN 257.104(d)(1)(i)

#### 1.0 OBJECTIVE

This report has been prepared to fulfill the requirements of 40 CFR 257.102(b) of the Coal Combustion Residuals (CCR) Rule to develop a Closure Plan for the Clifty Creek Station's Landfill Runoff Collection Pond (LRCP).

#### 2.0 DESCRIPTION OF THE CCR UNIT

The Clifty Creek Station is located on the shore of the Ohio River near Madison, Indiana and consists of six coal-fired electric generating units; each nominally rated at 217 megawatts, that began producing electricity in 1955 to support the Department of Energy's (DOE's) Portsmouth Gaseous Diffusion Plant located near Piketon, Ohio.

The LRCP is an inactive surface impoundment located southwest of the station, adjacent to the CCR landfill. The LRCP is surrounded by steep hillsides to the east and west, the CCR landfill to the north, and a constructed dam to the south. The current water surface elevation is approximately 486 feet. The crest of the dam is at approximate elevation 505 feet. Currently, the facility functions as the stormwater and leachate collection pond for the CCR landfill, and discharges to the Ohio River through an NPDES-permitted outfall.

#### 3.0 DESCRIPTION OF THE POST-CLOSURE PLAN 257.102(b)(1)(i)

[A description of the monitoring and maintenance activities required in paragraph (b) of this section for the CCR unit, and the frequency at which these activities will be performed]

#### 3.1 Section 257.104(b)(1)

[Maintaining the integrity and effectiveness of the final cover system including making repairs to the final cover as necessary to correct the effects of the settlement, subsidence, erosion, or other events and preventing run-on and run-off from eroding or otherwise damaging the final cover.]

Inspections are performed for the items noted below. The inspection frequencies are scheduled to properly detect any issues so that repairs can be performed before significant harm occurs.

- <u>Embankment</u>: The waste embankment will be inspected for slides, settlement, subsidence, displacement, and cover condition (see below).
- <u>Final Cover Surface</u>: The Final Cover surface will be inspected for any ponding of water or flat areas. Due to the design contours required to achieve the final cap grade, special attention will be focused to ensure that no settlement, subsidence, erosion, depressions or flat areas exist and that no water is allowed to pond above the cap system. Condition of the vegetation will be observed for maintenance needs (i.e., gaps in vegetation, presence of undesirable trees or brush).

• <u>Stormwater Management System</u>: The stormwater management system, including channels, culverts, slope drains, etc., will be inspected for erosion, integrity of channel lining, ponding, and accumulated sediment.

Maintenance during the post-closure care period will be performed as discussed below following the facility inspections.

- <u>Embankment:</u> Embankments will be inspected for slides, settlement, subsidence, displacement, and cover condition. Any areas exhibiting any such conditions will be repaired by reworking, replacing and/or compacting the material to design grade/ specifications.
- <u>Erosion Damage Repair</u>: Any areas exhibiting erosion will be repaired by reworking, replacing and/or compacting the material to design grade/specifications, and reseeding the area. Applications of additional fertilizer, selective herbicides, rodent control measures, etc. will be implemented as necessary. The selection of fertilizers and herbicides will strive to minimize their impact on groundwater. Follow-up monitoring of the repaired area will be conducted.
- Settlement, Subsidence, Displacement: Any areas at the closed site exhibiting evidence of settlement, subsidence, or displacement will be examined to determine the cause of the movement. If backfilling or placing additional fill material is needed to maintain the integrity of the closed structure, it will be performed in accordance with the site/closure specifications, including seeding. If the condition reoccurs or persists, or if the severity of the condition initially is judged to warrant it, a detailed investigation of the cause will be performed and remedial action will be performed. Repairs will be made as necessary. Follow-up monitoring of the area will be performed.
- <u>Closure Cap Surface</u>: Any areas that show signs of ponding water or flat contours will be
  observed and addressed. Due to the design contours required to achieve the final cap
  grade, special attention will be focused on the cap surface to promote drainage, reseeded to support vegetative growth, and maintained to minimize the ponding of water.
- <u>Stormwater Drainage System</u>: The channel linings are specified for design velocities.
   Maintenance of the stormwater management system will consist of removing sediment build up and/or undesirable vegetation from the stormwater management system's channels, culverts, and sediment basins as required. Eroded areas will be repaired by back-filling and reseeding in accordance with the specifications. Damage to culverts will be repaired; structure replacement will be performed if needed.

## 3.2 SECTION 257.104(b)(3)

[Maintaining the groundwater monitoring system and monitoring the groundwater in accordance with the requirements of §§257.90 through 257.98.]

The groundwater monitoring system will be observed for the general integrity of the wells, well casings and well protective casings. Any damaged portions of the monitoring wells and/or their protective casings will be replaced in-kind.

Monitoring the groundwater will be in accordance with the groundwater monitoring plan for this facility and in accordance with the requirements of §§257.90 through 257.98.

#### 4.0 POST-CLOSURE CONTACT 257.104 (d)(1)(ii)

[The name, address, telephone number and email address of the person or office to contact about the facility during the post-closure care period.]

The name, address, telephone number, and email address of the person to contact about the facility during the post-closure period will be provided upon notification of closure.

#### 5.0 POST-CLOSURE PLANNED USE 257.104 (d)(1)(iii)

[A description of the planned uses of the property during the post-closure period. Post-closure use of the property shall not disturb the integrity of the final cover, liner(s), or any other component of the containment system, or the function of the monitoring systems unless necessary to comply with the requirements in this subpart...]

The post-closure use of the property will be undisturbed vacant land space. The only activities occurring on the closed CCR unit will be related to the Post-Closure care activities. All other activities will be prohibited.



#### **Stantec Consulting Services Inc.** 11687 Lebanon Road, Cincinnati OH 45241

October 11, 2016 File: 175534018 Revision 0

Indiana-Kentucky Electric Corporation 3932 U.S. Route 23 P.O. Box 468 Piketon, Ohio 45661

RE:

Closure and Post-Closure Plans
Landfill Runoff Collection Pond
EPA Final Coal Combustion Residuals (CCR) Rule
Clifty Creek Station
Madison, Jefferson County, Indiana

#### 1.0 PURPOSE

This letter documents Stantec's certification of the EPA Final CCR Rule closure and post-closure plans for the Indiana-Kentucky (IKEC) Clifty Creek Station's Landfill Runoff Collection Pond.

#### 2.0 CLOSURE AND POST-CLOSURE PLAN

The closure plans describe the steps necessary to close the CCR units at any time during the life of the unit and is subject to the requirements described in 40 CFR 257.102(b). The post-closure plans describe the monitoring and maintenance activities to be performed during the post-closure period of the unit and is subject to the requirements of 40 CFR 257.104(d).

#### 3.0 SUMMARY OF FINDINGS

The EPA Final CCR Rule closure and post-closure plans are conceptual and subject to the completion of all necessary environmental reviews. They are therefore subject to change at any time. The attached closure and post-closure plans demonstrate compliance with the requirements set forth in 40 CFR 257.102(b) and 257.104(d).

#### 4.0 QUALIFIED PROFESSIONAL ENGINEER CERTIFICATION

I, Stan A. Harris, being a Professional Engineer in good standing in the State of Indiana, do hereby certify, to the best of my knowledge, information, and belief:

- 1. that the information contained in this certification is prepared in accordance with the accepted practice of engineering;
- 2. that the information contained herein is accurate as of the date of my signature below;



October 11, 2016 Page 2 of 2

RE:

Closure and Post-Closure Plans **Landfill Runoff Collection Pond** EPA Final Coal Combustion Residuals (CCR) Rule **Clifty Creek Station** Madison, Jefferson County, Indiana

- 3. that the closure plan for the IKEC Clifty Creek Station's Landfill Runoff Collection Pond meets the requirements described in 40 CFR 257.102(b); and
- 4. that the post-closure plan for the IKEC Clifty Creek Station's Landfill Runoff Collection Pond meets the requirement of 40 CFR 257.104(d).

**SIGNATURE** 

ADDRESS:

Stantec Consulting Services Inc.

11687 Lebanon Road Cincinnati, OH 45241

TELEPHONE:

(513) 842-8200

ATTACHMENT: Clifty Creek Landfill Runoff Collection Pond Closure and Post-Closure Plans

DATE 20/11/16

## **APPENDIX L**

Closure and Post-Closure Cost Estimate Opinion of Closure Costs
Landfill Runoff Collection Pond
Clifty Creek Plant
Indiana-Kentucky Electric Corporation
Madison, Jefferson County, Indiana

Facility Name: Clifty Creek Landfill Runoff Collection Pond

Facility Location: Madison, Indiana

Facility County: Jefferson

Total Waste Fill Acreage: 57.0 Acres
Total Grading Acreage: 63.0 Acres

Closure Year: 2020-2025
Acreage for Closure 57.0 Acres

(Based on MSW Landfill Closure Plan State Form 50391, Sections III and VI.)		37.0	ACIES	•
III. LABOR, MATERIALS, & TESTING (Provide a listing of items necessary to close	the facility	. For items th	at will va	ary depending upon
the number of acres to be closed, the quantities should be indicated on a per-acre	basis.			
A. Item		B. Quantity	C.	Units (per acre)
Geosynthetic materials (geomembrane, geotextile, geocomposite drainage layer)		57.0	\$	88,761.60
Uncompacted 30-inch soil layer		57.0	\$	35,717.53
6-inch vegetative soil layer		57.0	\$	12,524.64
Vegetative cover		57.0	\$	3,288.19
Surveying		57.0	\$	850.00
Engineering certification		57.0	\$	3,310.34
Additional items		lump sump		#REF!
Deed notation		lump sump	\$	10,000.00
V. COST PER ACRE FOR FINAL COVER & VEGETATION				
A. What Percent of Final Cover and Topsoil is Available from Areas that are Con by the Permittee?	trolled, and	will be Contro	olled thre	ough Post-Closure,
1. % of final cover:		0%		
2. Describe location of sources:	Offsite borrow sources are being assessed.			eing assessed.
3. % of topsoil:		0%		
4. Describe the location of sources:	Offsite borrow sources are being assessed.			
B. Cost per Acre for Acquisition and Placement of 30-inch Soil Layer				
1. Acquisition				
a. Quantity of soil needed per acre (cubic yard (yd³)/acre)		4,033		
b. Excavation unit cost (\$/yd <sup>3</sup> ) (if obtained onsite)		N/A		
c. Purchase unit cost (\$/yd³) (if obtained offsite)	\$	0.50	)	
d. Delivery unit cost (\$/yd <sup>3</sup> ) (if obtained offsite)	\$	5.50	)	
e. Acquisition cost (\$/acre)	\$	24,198.00	)	[1a * (1c+1d)
2. Placement and Compaction				
a. Placement/spreading unit cost	\$	2.4	1	
b. Compaction unit cost (\$/yd³)	\$	-		
c. Placement and compaction cost (\$/acre)	\$	9,719.53	3	[1a * (2a+2b)
3. Testing				
a. Soil classification (if soil source is of variable quality) (\$/acre)	\$	500.00	)	
b. Survey control for cover thickness and proper slopes (\$/acre)	\$	1,300.00	)	
c. Density testing (if planned) (\$/acre)		N/A		
d. Testing cost (\$/acre)	\$	1,800.00	)	
4. TOTAL COST, SOIL COVER (\$/acre)	\$	35,717.53	3	[1e + 2c + 3d

C. Cost per Acre for Acquisition and Placement of Geosynthetic Materials			
1. Acquisition			
a. Quantity of material needed per acre (square yards, yd²)		4,840	
b. Purchase and install geomembrane (\$/yd²)	\$	5.67	
c. Purchase and install nonwoven geotextile (\$/yd²)	\$	2.19	
d. Purchase and install composite drainage layer (\$/yd²)	\$	7.38	
e. Delivery unit cost (\$/yd²) (if applicable)	\$	-	
f. Acquisition cost (\$/acre)	\$	73,761.60	[1a * (1b+1c+1d)]
2. Placement			
Placement cost (\$/acre) (if applicable and not included in purchasing unit cost)	\$	-	
3. Testing and QA/QC			
a. Fingerprinting, destructive (shear and peel tests) & nondestructive seam test (\$/acre)	\$	10,000.00	
b. Other testing (\$/acre)	\$	5,000.00	
c. Testing cost (\$/acre) (if applicable)	\$	15,000.00	[3a + 3b]
4. TOTAL COST, GEOSYNTHETIC LAYERS (\$/acre)	\$	88,761.60	[1f + 2 +3c]
D. Cost per Acre for Acquisition & Placement of Topsoil			
1. Acquisition		007	
a. Quantity of topsoil needed per acre (yd³/acre)		807	
b. Excavation unit cost (\$/yd³) (if obtained onsite)	_	N/A	
c. Purchase unit cost (\$/yd³) (if obtained offsite)	\$	7.61	
d. Delivery unit cost (\$/yd³) (if obtained offsite)	\$	5.50	
e. Acquisition cost (\$/yd³)	\$	10,579.77	[1a * (1c+1d)]
2. Placement			
a. Spreading unit cost (\$/yd³)	\$	2.41	
b. Placement cost (\$/acre)	\$	1,944.87	[1a * 2a]
3. Topsoil Cost (\$/acre)	\$	12,524.64	[1e + 2b]
F. Cost per Acre to Establish Vegetation			
1. Vegetation			
a. Seeding unit cost (\$/acre)			
b. Fertilization unit cost (\$/acre)			
c. Mulching unit cost (\$/acre)			
d. Vegetation Establishment Cost (\$/acre)	\$	3,288.19	[1a + 1b + 1c]
G. Cost per Acre to Certify Closure			
Registered Professional Engineer		00	
a. Initial review of closure plan (hours)		80	
b. Total number of inspections		30	
c. Inspection time required (hours/visit)		24	
d. Total inspection time (hours)		720	[1b * 1c
e. Prepare final documentation (hours)		240	
f. Total engineer time (hours)		1,040	[1a + 1d + 1e]
g. Engineer unit labor cost (\$/hour)	\$	120.00	
g. Engineer unit labor cost (ψ/nour)	\$	124,800.00	[1f * 1g
h. Professional engineer cost (\$)	т .		
		37.7	

\$	140,291.96	soil thickness survey
\$	140,291.96	per acre
\$	140,291.96	per acre
\$	10,000.00	
	Cost	
\$	2,000,000.00	
\$	2,010,000.00	[A + B]
\$	10,006,641.72	[(Acreage * VI) + VI.C]
\$	1,000,664.17	
\$	11,007,305.89	
AL ASSU		REMENTAL BASIS
	\$ \$ \$ \$	Cost \$ 2,000,000.00 \$ 2,010,000.00 \$ 10,006,641.72 \$ 1,000,664.17

Opinion of Post-Closure Costs **Landfill Runoff Collection Pond** Clifty Creek Plant Indiana-Kentucky Electric Corporation Madison, Jefferson County, Indiana

Facility Name: **Clifty Creek Landfill Runoff Collection Pond** Facility Location: Madison, Indiana Facility County: Jefferson

Total Waste Fill Acreage: 57.0 Acres Total Grading Acreage: 63.0 Acres

Closure Year: 2020-2025

Acreage for Closure		57.0	Acres
Based on MSW Landfill Closure Plan State Form 50391, Section VI.)			
. Cost for Semi-Annual Inspections and Reports			
1. Inspection			
a. Number of inspections during post-closure period (semi-annual inspections for 30			
years		60	
b. Inspector time required (hours/insp)		30	
c. Inspector time labor cost (\$/hour)	\$	90.00	
d. Inspection cost (\$)	\$	162,000.00	[1a * 1b * 1c
2. Report Preparation			
Number of reports during post-closure period		60	
b. Cost per report (\$)	\$	5,000.00	
c. Report cost	\$	300,000.00	[2a * 2b
3. TOTAL COST, INSPECTIONS AND REPORTS (\$)	\$	462,000.00	[1d + 2c
(4)			
3. Cost for Maintenance of Final Cover and Vegetation/Vegetation Control	20/ 411		
<ol> <li>Cost for Maintenance of Final Cover and Vegetation/Vegetation Control</li> <li>Final Cover Maintenance - The cost for cover maintenance and vegetation shall be 1</li> </ol>	0% of th	e cost per are cal	culated for final cover and
<ol> <li>Cost for Maintenance of Final Cover and Vegetation/Vegetation Control</li> <li>Final Cover Maintenance - The cost for cover maintenance and vegetation shall be 1 vegetation in the closure plan (329 IAC 10-23-3(c)(5)(A)).</li> </ol>		e cost per are cal	culated for final cover and
<ol> <li>Cost for Maintenance of Final Cover and Vegetation/Vegetation Control</li> <li>Final Cover Maintenance - The cost for cover maintenance and vegetation shall be 1</li> </ol>		•	culated for final cover and
3. Cost for Maintenance of Final Cover and Vegetation/Vegetation Control  1. Final Cover Maintenance - The cost for cover maintenance and vegetation shall be 1 vegetation in the closure plan (329 IAC 10-23-3(c)(5)(A)).  a. 10% of cost for placement of final cover and vegetation (0.10 * cost given in Item VII of the closure plan)	.l	e cost per are cald 3,571.75 57	culated for final cover and
<ul> <li>3. Cost for Maintenance of Final Cover and Vegetation/Vegetation Control</li> <li>1. Final Cover Maintenance - The cost for cover maintenance and vegetation shall be 1 vegetation in the closure plan (329 IAC 10-23-3(c)(5)(A)).</li> <li>a. 10% of cost for placement of final cover and vegetation (0.10 * cost given in Item VII</li> </ul>	.l	3,571.75	
3. Cost for Maintenance of Final Cover and Vegetation/Vegetation Control  1. Final Cover Maintenance - The cost for cover maintenance and vegetation shall be 1 vegetation in the closure plan (329 IAC 10-23-3(c)(5)(A)).  a. 10% of cost for placement of final cover and vegetation (0.10 * cost given in Item VII of the closure plan)  b. Total permitted fill acreage	.I \$	3,571.75 57	
3. Cost for Maintenance of Final Cover and Vegetation/Vegetation Control  1. Final Cover Maintenance - The cost for cover maintenance and vegetation shall be 1 vegetation in the closure plan (329 IAC 10-23-3(c)(5)(A)).  a. 10% of cost for placement of final cover and vegetation (0.10 * cost given in Item VII of the closure plan)  b. Total permitted fill acreage  c. Total Cost, Maintenance of Final Cover and Vegetation Cover  2. Vegetation Control Costs	.I \$	3,571.75 57	
3. Cost for Maintenance of Final Cover and Vegetation/Vegetation Control  1. Final Cover Maintenance - The cost for cover maintenance and vegetation shall be 1 vegetation in the closure plan (329 IAC 10-23-3(c)(5)(A)).  a. 10% of cost for placement of final cover and vegetation (0.10 * cost given in Item VII of the closure plan)  b. Total permitted fill acreage  c. Total Cost, Maintenance of Final Cover and Vegetation Cover  2. Vegetation Control Costs  a. Mowing frequency (visits/30 years)	.I \$	3,571.75 57 203,589.92	
3. Cost for Maintenance of Final Cover and Vegetation/Vegetation Control  1. Final Cover Maintenance - The cost for cover maintenance and vegetation shall be 1 vegetation in the closure plan (329 IAC 10-23-3(c)(5)(A)).  a. 10% of cost for placement of final cover and vegetation (0.10 * cost given in Item VII of the closure plan)  b. Total permitted fill acreage  c. Total Cost, Maintenance of Final Cover and Vegetation Cover  2. Vegetation Control Costs  a. Mowing frequency (visits/30 years)  b. Area to be mowed (acres/visit)	.I \$ \$	3,571.75 57 203,589.92 60	
3. Cost for Maintenance of Final Cover and Vegetation/Vegetation Control  1. Final Cover Maintenance - The cost for cover maintenance and vegetation shall be 1 vegetation in the closure plan (329 IAC 10-23-3(c)(5)(A)).  a. 10% of cost for placement of final cover and vegetation (0.10 * cost given in Item VII of the closure plan)  b. Total permitted fill acreage  c. Total Cost, Maintenance of Final Cover and Vegetation Cover  2. Vegetation Control Costs  a. Mowing frequency (visits/30 years)	.I \$	3,571.75 57 203,589.92 60 63 296.63	[1a * 1b
3. Cost for Maintenance of Final Cover and Vegetation/Vegetation Control  1. Final Cover Maintenance - The cost for cover maintenance and vegetation shall be 1 vegetation in the closure plan (329 IAC 10-23-3(c)(5)(A)).  a. 10% of cost for placement of final cover and vegetation (0.10 * cost given in Item VII of the closure plan)  b. Total permitted fill acreage  c. Total Cost, Maintenance of Final Cover and Vegetation Cover  2. Vegetation Control Costs  a. Mowing frequency (visits/30 years)  b. Area to be mowed (acres/visit)  c. Mowing unit cost (\$/acre)  d. Total mowing cost (\$)	.I \$ \$ \$	3,571.75 57 203,589.92 60 63	[1a * 1b
3. Cost for Maintenance of Final Cover and Vegetation/Vegetation Control  1. Final Cover Maintenance - The cost for cover maintenance and vegetation shall be 1 vegetation in the closure plan (329 IAC 10-23-3(c)(5)(A)).  a. 10% of cost for placement of final cover and vegetation (0.10 * cost given in Item VII of the closure plan)  b. Total permitted fill acreage  c. Total Cost, Maintenance of Final Cover and Vegetation Cover  2. Vegetation Control Costs  a. Mowing frequency (visits/30 years)  b. Area to be mowed (acres/visit)  c. Mowing unit cost (\$/acre)	.I \$ \$	3,571.75 57 203,589.92 60 63 296.63	[1a * 1b]
3. Cost for Maintenance of Final Cover and Vegetation/Vegetation Control  1. Final Cover Maintenance - The cost for cover maintenance and vegetation shall be 1 vegetation in the closure plan (329 IAC 10-23-3(c)(5)(A)).  a. 10% of cost for placement of final cover and vegetation (0.10 * cost given in Item VII of the closure plan)  b. Total permitted fill acreage  c. Total Cost, Maintenance of Final Cover and Vegetation Cover  2. Vegetation Control Costs  a. Mowing frequency (visits/30 years)  b. Area to be mowed (acres/visit)  c. Mowing unit cost (\$/acre)  d. Total mowing cost (\$)  e. Other (\$) - specify below (weed control for well access, etc.)  f. Vegetation Control Costs	\$ \$ \$ \$ \$	3,571.75 57 203,589.92 60 63 296.63 1,121,261.40	[1a * 1b] [2a * 2b * 2c] [2d + 2e]
3. Cost for Maintenance of Final Cover and Vegetation/Vegetation Control  1. Final Cover Maintenance - The cost for cover maintenance and vegetation shall be 1 vegetation in the closure plan (329 IAC 10-23-3(c)(5)(A)).  a. 10% of cost for placement of final cover and vegetation (0.10 * cost given in Item VII of the closure plan)  b. Total permitted fill acreage  c. Total Cost, Maintenance of Final Cover and Vegetation Cover  2. Vegetation Control Costs  a. Mowing frequency (visits/30 years)  b. Area to be mowed (acres/visit)  c. Mowing unit cost (\$/acre)  d. Total mowing cost (\$)  e. Other (\$) - specify below (weed control for well access, etc.)  f. Vegetation Control Costs	.I \$ \$ \$ \$ \$	3,571.75 57 203,589.92 60 63 296.63 1,121,261.40	[1a * 1b [2a * 2b * 2c [2d + 2e
3. Cost for Maintenance of Final Cover and Vegetation/Vegetation Control  1. Final Cover Maintenance - The cost for cover maintenance and vegetation shall be 1 vegetation in the closure plan (329 IAC 10-23-3(c)(5)(A)).  a. 10% of cost for placement of final cover and vegetation (0.10 * cost given in Item VII of the closure plan)  b. Total permitted fill acreage  c. Total Cost, Maintenance of Final Cover and Vegetation Cover  2. Vegetation Control Costs  a. Mowing frequency (visits/30 years)  b. Area to be mowed (acres/visit)  c. Mowing unit cost (\$/acre)  d. Total mowing cost (\$)  e. Other (\$) - specify below (weed control for well access, etc.)	.I \$ \$ \$ \$ \$	3,571.75 57 203,589.92 60 63 296.63 1,121,261.40	[2a * 2b * 2c] [2d + 2e]

E. Cost for Groundwater Water Monitoring and Well Maintenance			
1. Monitoring Well Maintenance Labor Cost			
a. Maintenance frequency (visits/30 years)		60	
b. Number of monitoring wells needing maintenance per visit		2	(estimated)
c. Maintenance time required (hours/well)		4	
d. Unit labor cost (\$/hour)	\$	81.00	
e. Monitoring well maintenance labor cost (\$)	\$	38,880.00	[1a * 1b * 1c * 1d]
2. Monitoring Well Parts and Sampling Equipment Replacement Cost			
a. Number of wells needing replacement during post-closure period		0	
b. Existing monitoring well abandonment unit cost (\$)	\$	-	
c. New monitoring well construction unit cost (\$)	\$	-	(drilling charged by foot)
d. Monitoring well replacement cost (\$)	\$	-	
e. Number of pumps/bailers needing replacement during post-closure period		5	
f. Pump/bailer unit cost (\$/pump)	\$	2,000.00	
g. Pump/bailer replacement cost (\$)	\$	10,000.00	[2e * 2f]
h. Monitoring Maintenance and Pump/bailer Replacement Cost (\$)	\$	48,880.00	[1e + 2d + 2g]
3. Cost for Groundwater Monitoring			
a. Number of required monitoring wells		10	
b. Monitoring frequency (semi-annual sampling for 30 years)		60	
c. Sampling cost (\$/well)	\$	1,100.00	
d. Laboratory testing cost (\$/well)	\$	400.00	
e. Statistical Analyses and Report (\$/well)	\$	300.00	
d. Groundwater Monitoring Cost (\$)	\$	1,080,000.00	[3a * 3b * (3c+3d+3e)]
4. TOTAL, GROUNDWATER MONITORING AND WELL MAINTENANCE COST	\$	1,128,880.00	[2h + 3d]
F. Cost for Methane Monitoring and Maintenance		N/A	
F. Cost for Methane Monitoring and Maintenance		N/A	
G. Cost for Drainage and Erosion Control Maintenance			
G. Cost for Drainage and Erosion Control Maintenance  1. Drainage and erosion control maintenance frequency (visits/30 years)		60	
G. Cost for Drainage and Erosion Control Maintenance  1. Drainage and erosion control maintenance frequency (visits/30 years)  2. Cost for materials to repair per visit	\$	60 500.00	
G. Cost for Drainage and Erosion Control Maintenance  1. Drainage and erosion control maintenance frequency (visits/30 years)  2. Cost for materials to repair per visit  3. Total material cost (\$)	\$ \$	60	[1 * 2]
G. Cost for Drainage and Erosion Control Maintenance  1. Drainage and erosion control maintenance frequency (visits/30 years)  2. Cost for materials to repair per visit  3. Total material cost (\$)  4. Maintenance time required per visit (hours)		60 500.00	[1 * 2]
G. Cost for Drainage and Erosion Control Maintenance  1. Drainage and erosion control maintenance frequency (visits/30 years)  2. Cost for materials to repair per visit  3. Total material cost (\$)		60 500.00 30,000.00	
G. Cost for Drainage and Erosion Control Maintenance  1. Drainage and erosion control maintenance frequency (visits/30 years)  2. Cost for materials to repair per visit  3. Total material cost (\$)  4. Maintenance time required per visit (hours)	\$	60 500.00 30,000.00 10	[1 * 2]
G. Cost for Drainage and Erosion Control Maintenance  1. Drainage and erosion control maintenance frequency (visits/30 years)  2. Cost for materials to repair per visit  3. Total material cost (\$)  4. Maintenance time required per visit (hours)  5. Unit labor cost	\$ \$	60 500.00 30,000.00 10 140.00	
G. Cost for Drainage and Erosion Control Maintenance  1. Drainage and erosion control maintenance frequency (visits/30 years)  2. Cost for materials to repair per visit  3. Total material cost (\$)  4. Maintenance time required per visit (hours)  5. Unit labor cost  6. Total labor costs (\$)  7. TOTAL, DRAINAGE AND EROSION CONTROL MAINTENANCE COST	\$ \$ \$	60 500.00 30,000.00 10 140.00 84,000.00	[1 * 4 * 5]
G. Cost for Drainage and Erosion Control Maintenance  1. Drainage and erosion control maintenance frequency (visits/30 years)  2. Cost for materials to repair per visit  3. Total material cost (\$)  4. Maintenance time required per visit (hours)  5. Unit labor cost  6. Total labor costs (\$)  7. TOTAL, DRAINAGE AND EROSION CONTROL MAINTENANCE COST  H. Cost for Access Control and Benchmark Maintenance	\$ \$ \$ <b>\$</b>	60 500.00 30,000.00 10 140.00 84,000.00	[1 * 4 * 5] [3 + 6]
G. Cost for Drainage and Erosion Control Maintenance  1. Drainage and erosion control maintenance frequency (visits/30 years)  2. Cost for materials to repair per visit  3. Total material cost (\$)  4. Maintenance time required per visit (hours)  5. Unit labor cost  6. Total labor costs (\$)  7. TOTAL, DRAINAGE AND EROSION CONTROL MAINTENANCE COST  H. Cost for Access Control and Benchmark Maintenance  4. Fencing material cost (\$)	\$ \$ \$ <b>\$</b>	60 500.00 30,000.00 10 140.00 84,000.00	[1 * 4 * 5]
G. Cost for Drainage and Erosion Control Maintenance  1. Drainage and erosion control maintenance frequency (visits/30 years)  2. Cost for materials to repair per visit  3. Total material cost (\$)  4. Maintenance time required per visit (hours)  5. Unit labor cost  6. Total labor costs (\$)  7. TOTAL, DRAINAGE AND EROSION CONTROL MAINTENANCE COST  H. Cost for Access Control and Benchmark Maintenance  4. Fencing material cost (\$)  7. Total labor costs (\$)	\$ \$ <b>\$</b> \$	60 500.00 30,000.00 10 140.00 84,000.00 114,000.00	[1 * 4 * 5] [3 + 6]
G. Cost for Drainage and Erosion Control Maintenance  1. Drainage and erosion control maintenance frequency (visits/30 years)  2. Cost for materials to repair per visit  3. Total material cost (\$)  4. Maintenance time required per visit (hours)  5. Unit labor cost  6. Total labor costs (\$)  7. TOTAL, DRAINAGE AND EROSION CONTROL MAINTENANCE COST  H. Cost for Access Control and Benchmark Maintenance  4. Fencing material cost (\$)  7. Total labor costs (\$)  8. Benchmark maintenance cost (if applicable (\$)	\$ \$ <b>\$</b> \$ \$	60 500.00 30,000.00 10 140.00 84,000.00	[1 * 4 * 5] [3 + 6]
G. Cost for Drainage and Erosion Control Maintenance  1. Drainage and erosion control maintenance frequency (visits/30 years)  2. Cost for materials to repair per visit  3. Total material cost (\$)  4. Maintenance time required per visit (hours)  5. Unit labor cost  6. Total labor costs (\$)  7. TOTAL, DRAINAGE AND EROSION CONTROL MAINTENANCE COST  H. Cost for Access Control and Benchmark Maintenance  4. Fencing material cost (\$)  7. Total labor costs (\$)  8. Benchmark maintenance cost (if applicable (\$)  9. Other (\$)	\$ \$ <b>\$</b> \$	60 500.00 30,000.00 10 140.00 84,000.00 114,000.00	[1 * 4 * 5] [3 + 6]
G. Cost for Drainage and Erosion Control Maintenance  1. Drainage and erosion control maintenance frequency (visits/30 years)  2. Cost for materials to repair per visit  3. Total material cost (\$)  4. Maintenance time required per visit (hours)  5. Unit labor cost  6. Total labor costs (\$)  7. TOTAL, DRAINAGE AND EROSION CONTROL MAINTENANCE COST  H. Cost for Access Control and Benchmark Maintenance  4. Fencing material cost (\$)  7. Total labor costs (\$)  8. Benchmark maintenance cost (if applicable (\$)	\$ \$ <b>\$</b> \$ \$ \$	60 500.00 30,000.00 10 140.00 84,000.00 114,000.00	[1 * 4 * 5] [3 + 6] Facility is fenced.
G. Cost for Drainage and Erosion Control Maintenance  1. Drainage and erosion control maintenance frequency (visits/30 years)  2. Cost for materials to repair per visit  3. Total material cost (\$)  4. Maintenance time required per visit (hours)  5. Unit labor cost  6. Total labor costs (\$)  7. TOTAL, DRAINAGE AND EROSION CONTROL MAINTENANCE COST  H. Cost for Access Control and Benchmark Maintenance  4. Fencing material cost (\$)  7. Total labor costs (\$)  8. Benchmark maintenance cost (if applicable (\$)  9. Other (\$)	\$ \$ \$ \$ \$ \$	60 500.00 30,000.00 10 140.00 84,000.00 114,000.00	[1 * 4 * 5] [3 + 6] Facility is fenced.
G. Cost for Drainage and Erosion Control Maintenance  1. Drainage and erosion control maintenance frequency (visits/30 years)  2. Cost for materials to repair per visit  3. Total material cost (\$)  4. Maintenance time required per visit (hours)  5. Unit labor cost  6. Total labor costs (\$)  7. TOTAL, DRAINAGE AND EROSION CONTROL MAINTENANCE COST  H. Cost for Access Control and Benchmark Maintenance  4. Fencing material cost (\$)  7. Total labor costs (\$)  8. Benchmark maintenance cost (if applicable (\$)  9. Other (\$)  10. TOTAL, ACCESS CONTROL/BENCHMARK MAINTENANCE COST	\$ \$ \$ \$ \$ \$ \$	60 500.00 30,000.00 10 140.00 84,000.00 114,000.00	[1 * 4 * 5] [3 + 6] Facility is fenced.
G. Cost for Drainage and Erosion Control Maintenance  1. Drainage and erosion control maintenance frequency (visits/30 years)  2. Cost for materials to repair per visit  3. Total material cost (\$)  4. Maintenance time required per visit (hours)  5. Unit labor cost  6. Total labor costs (\$)  7. TOTAL, DRAINAGE AND EROSION CONTROL MAINTENANCE COST  H. Cost for Access Control and Benchmark Maintenance  4. Fencing material cost (\$)  7. Total labor costs (\$)  8. Benchmark maintenance cost (if applicable (\$)  9. Other (\$)  10. TOTAL, ACCESS CONTROL/BENCHMARK MAINTENANCE COST	\$ \$ \$ \$ \$ \$ \$	60 500.00 30,000.00 10 140.00 84,000.00 114,000.00	[1 * 4 * 5] [3 + 6] Facility is fenced.
G. Cost for Drainage and Erosion Control Maintenance  1. Drainage and erosion control maintenance frequency (visits/30 years)  2. Cost for materials to repair per visit  3. Total material cost (\$)  4. Maintenance time required per visit (hours)  5. Unit labor cost  6. Total labor costs (\$)  7. TOTAL, DRAINAGE AND EROSION CONTROL MAINTENANCE COST  H. Cost for Access Control and Benchmark Maintenance  4. Fencing material cost (\$)  7. Total labor costs (\$)  8. Benchmark maintenance cost (if applicable (\$)  9. Other (\$)  10. TOTAL, ACCESS CONTROL/BENCHMARK MAINTENANCE COST  I. Optional - Maintenance of dike(s) required for facilities constructed in floodplain/flo	\$ \$ \$ \$ \$ \$ \$	60 500.00 30,000.00 10 140.00 84,000.00 114,000.00	[1 * 4 * 5] [3 + 6] Facility is fenced.
G. Cost for Drainage and Erosion Control Maintenance  1. Drainage and erosion control maintenance frequency (visits/30 years)  2. Cost for materials to repair per visit  3. Total material cost (\$)  4. Maintenance time required per visit (hours)  5. Unit labor cost  6. Total labor costs (\$)  7. TOTAL, DRAINAGE AND EROSION CONTROL MAINTENANCE COST  H. Cost for Access Control and Benchmark Maintenance  4. Fencing material cost (\$)  7. Total labor costs (\$)  8. Benchmark maintenance cost (if applicable (\$)  9. Other (\$)  10. TOTAL, ACCESS CONTROL/BENCHMARK MAINTENANCE COST  I. Optional - Maintenance of dike(s) required for facilities constructed in floodplain/flo	\$     \$    \$     \$	60 500.00 30,000.00 10 140.00 84,000.00 114,000.00	[1 * 4 * 5] [3 + 6] Facility is fenced.
G. Cost for Drainage and Erosion Control Maintenance  1. Drainage and erosion control maintenance frequency (visits/30 years)  2. Cost for materials to repair per visit  3. Total material cost (\$)  4. Maintenance time required per visit (hours)  5. Unit labor cost  6. Total labor costs (\$)  7. TOTAL, DRAINAGE AND EROSION CONTROL MAINTENANCE COST  H. Cost for Access Control and Benchmark Maintenance  4. Fencing material cost (\$)  7. Total labor costs (\$)  8. Benchmark maintenance cost (if applicable (\$)  9. Other (\$)  10. TOTAL, ACCESS CONTROL/BENCHMARK MAINTENANCE COST  I. Optional - Maintenance of dike(s) required for facilities constructed in floodplain/flo	\$     \$    \$     \$	60 500.00 30,000.00 10 140.00 84,000.00 114,000.00	[3 + 6] Facility is fenced.  [4 + 7 + 8 + 9]
G. Cost for Drainage and Erosion Control Maintenance  1. Drainage and erosion control maintenance frequency (visits/30 years)  2. Cost for materials to repair per visit  3. Total material cost (\$)  4. Maintenance time required per visit (hours)  5. Unit labor cost  6. Total labor costs (\$)  7. TOTAL, DRAINAGE AND EROSION CONTROL MAINTENANCE COST  H. Cost for Access Control and Benchmark Maintenance  4. Fencing material cost (\$)  7. Total labor costs (\$)  8. Benchmark maintenance cost (if applicable (\$)  9. Other (\$)  10. TOTAL, ACCESS CONTROL/BENCHMARK MAINTENANCE COST  I. Optional - Maintenance of dike(s) required for facilities constructed in floodplain/flo	\$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$	60 500.00 30,000.00 10 140.00 84,000.00 114,000.00 - 5,000.00 - 5,000.00	[1 * 4 * 5] [3 + 6] Facility is fenced. [4 + 7 + 8 + 9]
G. Cost for Drainage and Erosion Control Maintenance  1. Drainage and erosion control maintenance frequency (visits/30 years)  2. Cost for materials to repair per visit  3. Total material cost (\$)  4. Maintenance time required per visit (hours)  5. Unit labor cost  6. Total labor costs (\$)  7. TOTAL, DRAINAGE AND EROSION CONTROL MAINTENANCE COST  H. Cost for Access Control and Benchmark Maintenance  4. Fencing material cost (\$)  7. Total labor costs (\$)  8. Benchmark maintenance cost (if applicable (\$)  9. Other (\$)  10. TOTAL, ACCESS CONTROL/BENCHMARK MAINTENANCE COST  I. Optional - Maintenance of dike(s) required for facilities constructed in floodplain/flo	\$     \$    \$     \$     \$     \$     \$     \$     \$     \$     \$     \$     \$    \$     \$     \$     \$     \$     \$     \$     \$     \$     \$     \$    \$    \$     \$     \$     \$     \$     \$     \$     \$     \$     \$     \$	60 500.00 30,000.00 10 140.00 84,000.00 114,000.00 - 5,000.00	[1 * 4 * 5] [3 + 6]  Facility is fenced  [4 + 7 + 8 + 9]